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## Article

# VARs and HDMR Sensitivity Analysis of Groundwater Flow Modeling through an Alluvial Aquifer Subject to Tidal Effects

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**Abstract:** Groundwater flow and transport models are essential tools for assessing and quantifying the migration of organic contaminants at polluted sites. Uncertainties in the hydrodynamic and transport parameters of the aquifer have a significant effect on model predictions. Uncertainties can be quantified with advanced sensitivity methods such as Sobol's High Dimensional Model Reduction (HDMR) and Variogram Analysis of Response Surfaces (VARS). Here we present the application of VARS and HDMR to assess the global sensitivities of the outputs of a transient groundwater flow model of the Gállego alluvial aquifer which is located downstream the Sardas landfill in Huesca (Spain). The aquifer is subject to the tidal effects caused by the daily oscillations of the water level in the Sabiñánigo reservoir. Global sensitivities are analysed for hydraulic heads, aquifer/reservoir fluxes, groundwater Darcy velocity and hydraulic head calibration metrics. Input parameters include aquifer hydraulic conductivities and specific storage, aquitard vertical hydraulic conductivities, and boundary inflows and conductances. VARS, HDMR and graphical methods agree to identify the most influential parameters which for most of the outputs are the hydraulic conductivities of the zones closest to the landfill, the vertical hydraulic conductivity of the most permeable zones of the aquitard, and the boundary inflow coming from the landfill. The sensitivity of heads and aquifer/reservoir fluxes with respect to specific storage change with time. The aquifer/reservoir flux when the reservoir level is high shows interactions between specific storage and aquitard conductivity. VARS and HDMR parameter rankings are similar for the most influential parameters. However, there are discrepancies for the less relevant parameters. The efficiency of VARS was demonstrated by achieving stable results with a relatively small number of simulations.

**Keywords:** VARS; HDMR; global sensitivity analysis; groundwater flow model; tidal effect; Sardas

## 1. Introduction

Groundwater flow and transport numerical models are used to simulate the migration of persistent pollutants in contaminated sites. The relationship between input and output variables of these models over the entire range of input values is non-linear. Numerical models are subject to numerous uncertainties which hinder the groundwater and contaminant transport modeling. Quantifying the impact of these uncertainties is crucial to improve the accuracy of model predictions [1,2]. Uncertainty in groundwater models may arise from different sources, including: (1) The lack of hydrogeological and hydrodynamic data in the study area; (2) Experimental and data measurement errors; (3) Conceptual or mathematical model oversimplification; (4) Heterogeneity of the hydrodynamic parameters; (5) Boundary conditions; (6) Sparse estimations of aquifer properties derived from tests; and (7) Scale effects [3–9].



Sensitivity analysis provides a powerful tool to evaluate the impact of uncertainties in input parameters on model outputs and identify the most influential parameters. Measures of sensitivity can be local or global. Local sensitivity methods quantify the sensitivity of the model to one-at-a-time changes in parameters around a reference set of parameters [10–14]. On the other hand, Global Sensitivity Analysis (GSA) evaluate the model sensitivity for wide ranges of parameter and quantify also the interactions among input parameters [15–17].

Graphical methods are often used in the first steps of the sensitivity analysis. Scatterplots or two-variable plots of an output versus an input parameter can be useful sometimes for identifying parameter interactions. Some graphical methods are based on the analysis of the cumulative sum of the normalized reordered model output (CUSUNORO) curve plots [18]. CUSUNORO curves provide a compact and fast way to rank the input parameters, identify the sign of the parameter sensitivity, assess the monotonicity of the dependence of the input parameter and the output, and identify nonlinear relationships.

The method of Morris “elementary effects” is a derivative-based method which consists on perturbing each parameter independently and averaging either the partial finite differences [19] or their absolute values [20]. The method of Morris and its variants are often used to identify and discard the least influential parameters. Morris method is less reliable in determining the most relevant parameters [21].

Variance-based methods such as the Sobol method also known as High Dimensional Model Representation, HDMR, are useful to rank the relevance of input parameters [22], quantify their importance and identify parameters having linear additive effects or nonlinear interactive effects [4,23]. The Sobol method has been found useful to study the uncertainties of groundwater flow and solute transport models [3,4]. However, several studies have highlighted some challenges related to variance-based GSA implementation for complex numerical models [4]. Morris and Campolongo methods [19,20] do not address the dependencies between various sources of uncertainty [4]. Furthermore, sampling size and extreme results can hinder the identification of uncertainty by using the Sobol indexes [24]. Variance-based methods require many model simulations with high computational cost to evaluate the variance of the outputs [4,24]. This is especially challenging when considering high-dimensional spatially-distributed inputs such as hydraulic conductivity, areal recharge and boundary fluxes [4]. In addition, variance-based methods assume that the uncertainty of the model outputs is fully characterized by its variance [25]. The ranking of the influential parameters based only on Sobol indexes may exclude important information [26].

Morris and HDMR methods require a sufficiently large number of model simulations to achieve reliable results. Running thousands of simulations of complex numerical models can be challenging. Alternative solutions have been proposed such as the Variogram Analysis of Response Surfaces (VARS). VARS is a GSA method based on the properties of variograms [27,28]. It combines local sensitivities such as those coming from derivative-based Morris methods and variance-based approaches such as HDMR [29]. Therefore, Morris and Sobol methods are particular cases of VARS. The spatial structure of model outputs and the sensitivity analysis across a wide range of scales are characterized from the directional variograms of the model outputs [27]. According to Razavi and Gupta (2016), VARS is between 1 and 2 orders of magnitude more efficient compared to the Sobol method, while still providing consistent results [28]. This efficiency derives from reducing the number of elementary effects used to compute the total-order index in a given number of simulations [30]. The reduction to explore the input space is compensated when the model is dominated by main effects (as most physical models are) by using a star-based sampling design [30]. In addition, VARS provides metrics that accurately estimate Sobol total-order effects [28] and, for high-dimensional complex models, VARS estimators achieve a high performance even for a low number of runs [30].

GSA methods have been used to quantify the output uncertainties caused by parameter uncertainties for hydrogeological models. Malaguerra *et al.* [31] applied the Morris method to rank the influence of parameters in a tracer test. Zou *et al.* [32] proposed a surrogate model to improve the computational efficiency of the analysis and to approximate the results of a numerical contaminant transport model. The most influential parameters identified with VARS were the hydraulic

conductivity, the recharge and the porosity. The Sobol analysis of a synthetic heterogeneous phreatic aquifer contaminated by chlorobenzene [33] concluded that the contaminant distribution in the aquifer depends mainly on: (1) Transverse coordinate of the contamination source; (2) Porosity; (3) Hydraulic conductivity; (4) Hydraulic gradient; and (5) Longitudinal and transverse dispersivities. Wang *et al.* [34] presented a GSA of a groundwater flow model of a colluvial landslide. The results highlighted the importance of selecting an appropriate range of input parameters [34].

Using more than a single GSA method is advisable to increase the confidence in the ranking of input parameters [21,35]. Mishra *et al.* [35] presented the application of two GSA methods to a synthetic groundwater flow model and a groundwater flow and transport model of a nuclear testing site. The two GSA methods provided consistent results and supplemented one another for both models. VARS and HDMR methods were applied recently to a reactive transport model of a high-level radioactive waste repository in a granitic host rock [36]. The study concluded that parameter rankings of both methods are nearly identical for the 5 input parameters.

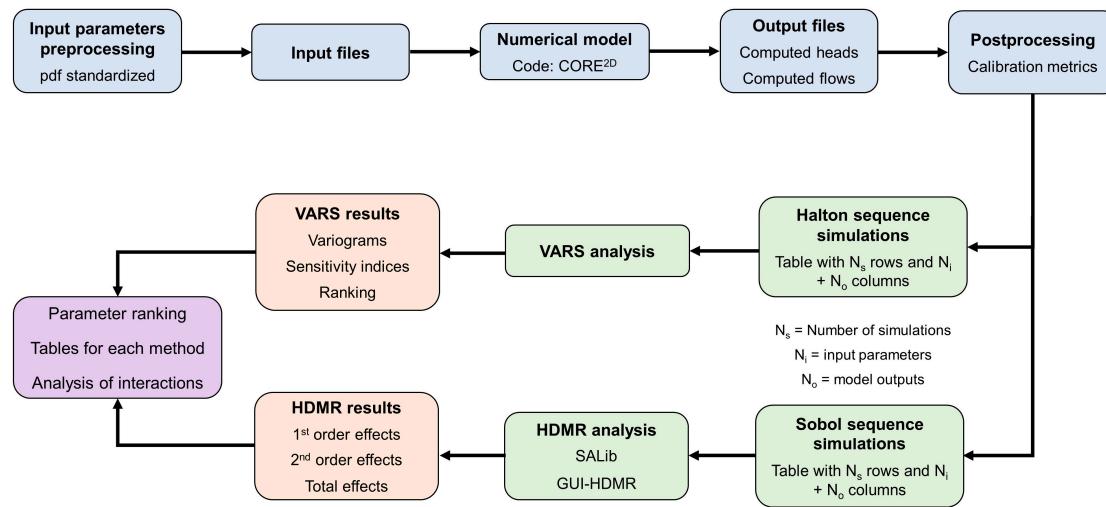
The Sardas site is near the Sabiñánigo (Huesca, Spain) reservoir and is heavily affected by lindane and its degradation products released from the Sardas landfill which contains solid lindane production wastes and chlorinated organic contaminants forming a dense non-aqueous phase liquid (DNAPL) [37]. The Sabiñánigo reservoir fluctuations produce a tidal effect on the piezometric heads of the alluvial aquifer [38]. Understanding the dynamics of the tidal effect and the aquifer/reservoir interactions is crucial for quantifying groundwater and contaminant fluxes and proposing remediation techniques. Sobral *et al.* [38] presented a two-dimensional horizontal groundwater flow model through the Gállego alluvial aquifer. The model reproduced the oscillations of the piezometric head in the aquifer caused by the Sabiñánigo reservoir. The groundwater flow model of the Gállego alluvial is subject to uncertainties in aquifer and aquitard parameters, boundary fluxes and recharge rates. The local sensitivity analysis presented by Sobral *et al.* [38] was useful to identify the most influential parameters for hydraulic heads in the aquifer. However, their sensitivity analysis was limited to the combination of parameter values corresponding to the calibrated conditions and did not account for the interactions among parameters. The limitations of the local sensitivity analysis are overcome here by performing global sensitivity analyses by using VARS and HDMR methods. Input uncertain parameters include (1) Aquifer parameters; (2) Aquitard vertical conductivities; (3) Boundary inflows; 4) Conductances or leakage coefficients for aquifer/river and aquifer/dam interactions and 5) Areal recharge. The outputs include the computed piezometric heads in 3 monitoring wells, calibration metrics, aquifer/reservoir fluxes, and the average groundwater Darcy velocity modulus. HDMR and VARS methods are used to: (1) Identify the most influential input parameters on the model outputs; and (2) Quantify parameter interactions. The paper starts by describing the study area and the groundwater flow model. Then, the global sensitivity methods (VARS and HDMR) are presented, and the input and output variables are listed. GSA results and discussion are presented afterwards. The paper ends with the main conclusions.

## 2. Materials and Methods

### 2.1. Methodological Framework

The methodology used in this study is outlined in Figure 1. A conceptual model was developed based on the analysis of the study area, available hydrogeological and hydrodynamic data, the initial and boundary conditions, and material zones as defined in Sobral *et al.* [38]. A finite element mesh of triangular elements was used to solve the partial differential equations of groundwater flow [38]. Uncertainties in model input aquifer and boundary parameters lead to uncertainties in model outputs. Thus, global sensitivity analysis methods (VARS and HDMR) are employed here to rank the most important input parameters, quantify the contribution of each parameter to the variance of the results, and quantify uncertainties in model outputs. The input parameters were selected first together with their ranges and probability distribution functions.  $N_s$  simulations of groundwater flow were performed with CORE<sup>2D</sup> for the selected combinations of input parameters. The outputs of the flow model were postprocessed to generate the tables containing the  $N_i$  input parameters and  $N_o$  outputs. A Halton sequence was selected to generate the input parameters for the VARS analysis and

a Sobol sequence was adopted for the HDMR analysis. VARS and HDMR results were compared in terms of parameter rankings (Table 1).



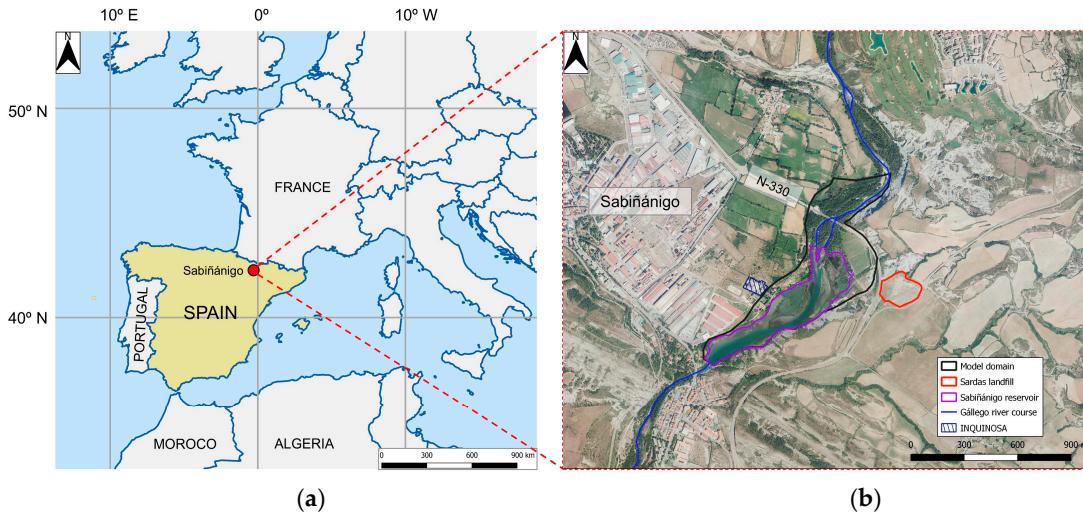
**Figure 1.** Flowchart of the methodology used in this study.

**Table 1.** Ranges and statistical distributions of the input parameters.

Parameter	Minimum	Maximum	Unit	Distribution
Aquifer conductivity K1	10	$10^3$	m/d	Log-uniform
Aquifer conductivity K2	10	$10^3$	m/d	Log-uniform
Aquifer conductivity K3	10	$10^3$	m/d	Log-uniform
Aquifer conductivity K4	10	$10^3$	m/d	Log-uniform
Storage coefficient $S_s$	$10^{-5}$	$10^{-3}$	1/m	Log-uniform
Aquitard conductivity $K_{Vs1}$	$10^{-3}$	1	m/d	Log-uniform
Aquitard conductivity $K_{Vs2}$	$10^{-3}$	1	m/d	Log-uniform
Aquitard conductivity $K_{Vs3}$	$10^{-4}$	$10^{-1}$	m/d	Log-uniform
Leakage coefficient $\alpha_r$	10	$10^3$	$m^2/d$	Log-uniform
Conductance $\alpha_d$	1	100	$m^2/d$	Log-uniform
Boundary inflow $Q_6$	$3 \cdot 10^{-3}$	0.05	$m^3/d/m$	Uniform
Boundary inflow $Q_7$	$2 \cdot 10^{-3}$	0.20	$m^3/d/m$	Uniform
Boundary inflow $Q_9$	0.25	1.00	$m^3/d/m$	Uniform
Boundary inflow $Q_2$	$1.70 \cdot 10^{-2}$	1.70	$m^3/d/m$	Uniform
Boundary inflow $Q_1$	$2 \cdot 10^{-3}$	$10^{-1}$	$m^3/d/m$	Uniform
Recharge $r_c$	5	200	mm/year	Uniform
Recharge $r_u$	20	401.5	mm/year	Uniform

## 2.2. Site Description

The study area is located in Sabiñánigo (Huesca, Spain) where a lindane-producing factory (INQUINOSA) operated from 1975 to 1992 on the right bank of the Sabiñánigo reservoir [39] (Figure 2). The Sabiñánigo dam was built in the Gállego River course in 1963 to provide enough hydroelectric power to chemical factories [38]. INQUINOSA deposited solid and liquid hexachlorocyclohexane (HCH) wastes in an uncontrolled manner in the Sardas landfill until approximately 1984 [39]. The landfill is located in the left bank of the Gállego river at 500 m from the Sabiñánigo reservoir.



**Figure 2.** (a) Location of the study area; (b) enlargement showing the model domain, the Sabiñánigo reservoir, the Sardas landfill, the Gállego river course and the INQUINOSA former production site.

The floodplain of the Gállego River downstream the Sardas landfill is heavily affected by HCH wastes. By the 1980s, the Sardas landfill was completely filled with urban, construction, and industrial solid wastes including lindane production wastes (between  $3 \cdot 10^4$  and  $8 \cdot 10^4$  tons of solid HCH wastes) [39]. Since the landfill lacks a bottom liner system [39], DNAPL and leachates flowed freely from the landfill into the alluvial of the Gállego river until 1995. In addition, during the construction of the road N-330 in the early 1990s, 50 000 m<sup>3</sup> of landfill wastes were deposited on the ground surface of the alluvial plain [40]. In 1995, the landfill was sealed superficially with a PEAD sheet and laterally with a front slurry wall. However, the slurry wall does not prevent leachates from flowing into the alluvial aquifer [38,41].

The alluvial of the Gállego river consists of quaternary silts overlying a layer of sands and gravels. The Larrés marls underlie the quaternary sediments of the Gállego river alluvial [40]. A geological profile across the Sabiñánigo reservoir and the Gállego river floodplain is presented by Sobral *et al.* [38]. The sands and gravels are much more permeable than the silts and the marls [42].

Liquid HCH wastes were detected downstream the Sardas landfill in 2009 [37], prompting hydrogeological and chemical studies aimed at identifying groundwater contaminant sources and proposing treatment options [39]. DNAPL has migrated by gravity through the alluvial due to its high density [43], and is mainly located on top of the marl layer [42] and inside its fractures [44]. This poses a significant risk since the aquifer and the reservoir are partially connected [38].

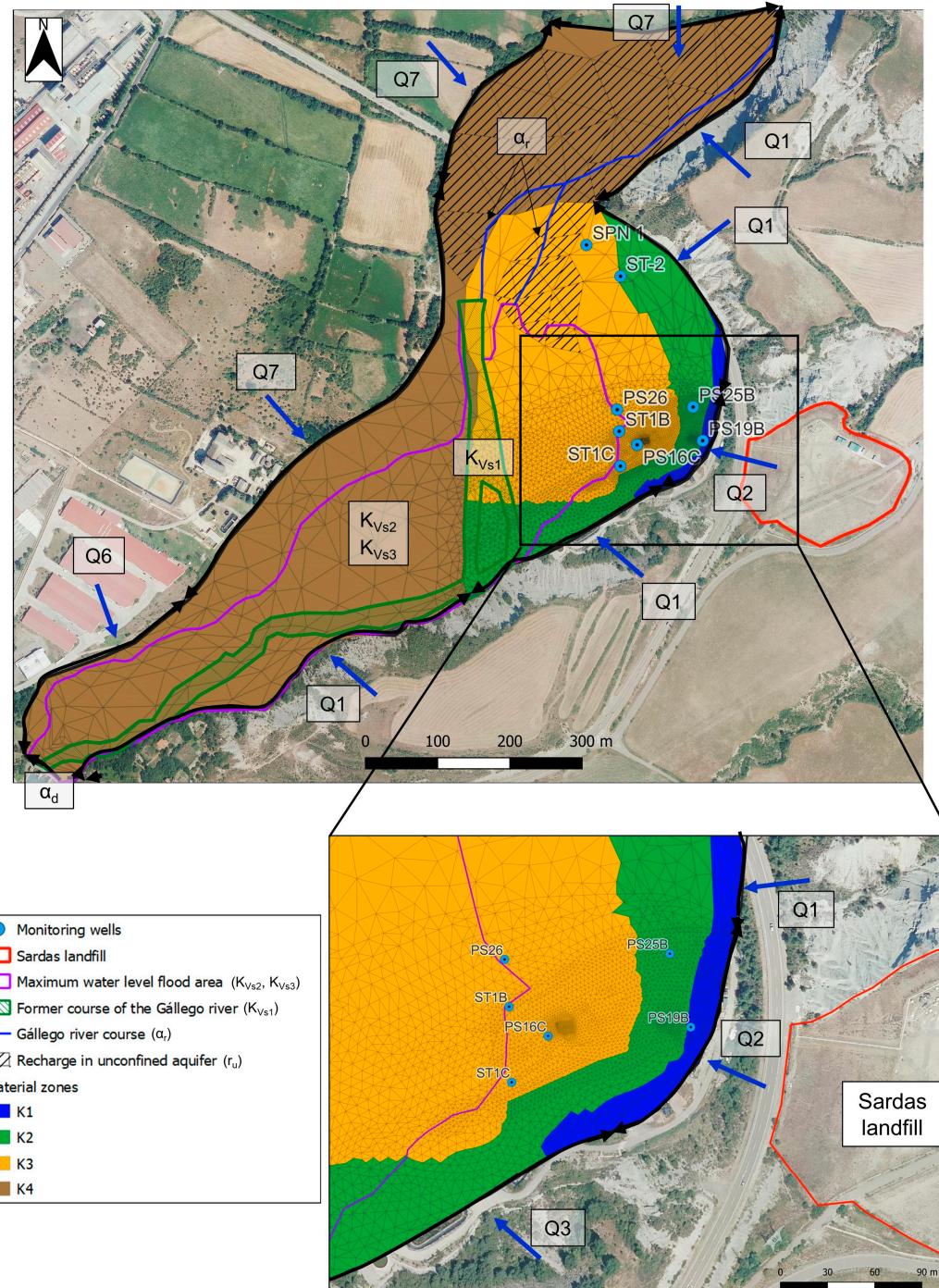
The layer of alluvial sands and gravels is confined by quaternary alluvial silts. Since its construction, the reservoir has undergone a siltation process [45] which deposited silting sediments and reducing greatly the reservoir capacity [38]. The alluvial silts and silting sediments act as an aquitard and play the role of a barrier for pollutants by retaining and slowing the arrival of contaminants to the reservoir [38]. However, the presence of DNAPL and HCH sorbed in the soil [42] constitutes a persistent source of organic pollutants.

### 2.3. Conceptual Model

The groundwater flow model presented here focuses on the alluvial aquifer downstream the Sardas landfill with a large presence of solid and liquid HCH wastes. According to the monthly chemical analyses by the Ebro River Authority, the Sabiñánigo reservoir is the main receptor of chlorinated organic contaminants in the Sardas site.

The model assumes that the aquifer recharges through rainfall infiltration, from the surrounding fluvioglacial terraces on the right bank and from the Larrés marls on the left bank (Figure 3). The hydraulic conductivity of the sands and gravels layer is extremely large compared to that of the silts, silting sediments and marl formations. Therefore, groundwater flow takes place mostly through the

sands and gravels of the alluvial and discharges into the Gállego river, the Sabiñánigo reservoir and underneath the Sabiñánigo dam in the downstream part of the study area. The tidal effect caused by the reservoir water level fluctuations has a significant effect on water transfer between the alluvial aquifer and the reservoir [38,46]. Aquifer groundwater flows from the aquifer into the reservoir for normal and low reservoir water levels. However, the flow reverses when the reservoir level rises above the piezometric head in the aquifer [38]. The sands and gravels located underneath the Sabiñánigo reservoir are assumed to be confined by the reservoir silting sediments and the alluvial silts.



**Figure 3.** 2D finite element mesh, monitoring wells, material zones, boundary conditions, and GSA input parameters (top plot) and enlargement showing the area downstream the Sardas landfill

(bottom plot). The confined storage coefficient ( $S_s$ ) is the same in the four material zones. The sands and gravels are assumed to be confined in the alluvial ( $r_c$ ), except in the wooded areas ( $r_u$ ). Unconfined areas are shown with a back hashed polygon.

#### 2.4. Numerical Groundwater Flow Model

The numerical model presented here is an updated version of the groundwater flow model through the Gállego alluvial aquifer reported in Sobral *et al.* [38]. Contour maps of thickness of the geological formations were slightly improved by incorporating information from recently drilled wells. Leakage coefficients for the aquifer/reservoir interactions were also updated.

The limits of the model domain include the right bank of the Gállego river along the west, the left bank of the river where the Sardas landfill is located, and the Sabiñánigo dam. The northern limit is located about 500 m north of the tail of the Sabiñánigo reservoir (Figure 3). The model was performed with a non-uniform finite element mesh of triangular elements made of 4399 nodes and 8655 elements (Figure 3). Groundwater flow simulation spans 109 days, starting July 15, 2020 and ending on October 31, 2020. Time increments are all equal to 30 minutes. This time increment is equal to the frequency of the automatic reservoir level measurements.

Aquifer inflows include recharge from rainfall infiltration which was estimated with a hydrological water balance model [47], and inflows along the boundaries in both left and right banks. These inflows were simulated with a Neuman condition [38]. River/aquifer, reservoir/aquifer and aquifer/dam interactions were simulated with Cauchy conditions as described by Sobral *et al.* [38].

#### 2.5. Global Sensitivity Methods

##### 2.5.1. Graphical Methods

Graphical methods provide a simple, compact and informative tool to study global sensitivities. They are especially adequate for communicating results. They provide a good compromise between the complexity of other methods and compactness [48]. Two-variable interaction plots are helpful to identify visually parameter interactions.

The method of cumulative sum of the normalized reordered model output (CUSUNORO) curve plots condenses in a compact form the sensitivity of an output,  $y$ , to a set of input parameters [18]. The CUSUNORO curve function,  $z(i)$ , is the scaled cumulative sum of the ordered residuals of the output,  $y$ , [18] and is given by:

$$z(i) = \frac{1}{\sqrt{n \cdot S_{yy}}} \cdot \sum_{j=1}^i (\tilde{y}_{\pi(j)} - \bar{y}) \quad (1)$$

where  $\bar{y}$  and  $S_{yy}$  are the sample mean and standard deviation of the outputs, respectively,  $n$  is sample size or the number of simulations, and  $(\tilde{y}_{\pi(j)} - \bar{y})$  is the  $j$ -th ordered residual. The function  $z(i)$  can be interpreted based on the following terms: (1) A divisor term or scaling factor,  $\sqrt{n}$ , which guarantees that the cumulative sum is scaled relative to the sample size  $n$ ; (2) A divisor term  $S_{yy}$  (standard deviation) which ensures that  $z(i)$  is dimensionless and standardized; and (3) A summation term representing the cumulative sum of the deviations of the ordered output values from the mean  $\bar{y}$ .

##### 2.5.2. Sobol Method

The High Dimensional Model Representation (HDMR) method evaluates the input-output relationships of complex models with many input parameters [22]. The Sobol method for global sensitivity analysis requires to compute complex quadratures in a high dimensional space. Random and quasi-random rules are usually applied, which involve a set of sample points where model outputs are analyzed. The input parameters are normalized to range from 0 to 1.

The variance of the output is split as the sum of non-negative variance terms, each associated with combinations of input parameters [22]. Sobol indexes quantify the combined contributions of the input parameters to the variance of the output. The first-order sensitivity index or "main effect"

index,  $S_i$ , measures the relative individual contribution of the  $i$ -th input variable on the variance of the output. The second order sensitivity indexes,  $S_{ij}$ , measure the interactions between the  $i$ -th and the  $j$ -th inputs on the output variable [22]. Both terms,  $S_i$  and  $S_{ij}$ , are usually provided by HDMR software packages. The total sensitivity index, or total-effect index or total-order index,  $S_{Ti}$ , is a measure of the total contribution of the  $i$ -th input to the output variance [51–53], including all the interactions with the rest of the inputs of order  $k$  ranging from 2 to  $n$ . Its expression is given by:

$$S_{Ti} = S_i + \sum_j S_{ij} + \sum_{j,k} S_{ijk} + \dots + \sum_{j,\dots,n} S_{ij,\dots,n} \quad (2)$$

### 2.5.3. Variogram Analysis of Response Surfaces

The variogram analysis of response surfaces (VARS) is a method of characterizing the structure and variability of model outputs within the input parameter space [27]. VARS computes several global sensitivity metrics, including Morris and total sensitivity Sobol indexes by using a tailored sampling strategy [27]. VARS star-based sampling strategy requires all input parameters to be normalized to range between 0 and 1. Then, the star centers are selected by using a random sampling strategy. A cross section of equally spaced points with a selected resolution  $\Delta h$  is performed for each star center and for each input variable. Each set of points constitutes a “star”. There are different methods to sample the star centers [49]. The Halton sequence was selected for this study [50]. The output variables are calculated at all points for all stars. Directional variograms and covariograms are calculated along the lines of the input parameters.

Like other GSA methods, VARS requires performing a sufficiently large number of model simulations to obtain estimates of the directional variograms for each input variable. The variograms along the directions of the parameters provide the sensitivity indexes of the GSA. The limit of the directional variogram divided by  $h^2$  as  $h$  approaches 0, corresponds to Campolongo's version of the Morris effects [28]. On the other hand, the limit of the variogram for large  $h$  corresponds to the total variance linked to the Sobol total effect or total sensitivity index [30].

Morris and Sobol methods are limiting cases of VARS. The directional variograms of VARS provide metrics that accurately estimate Sobol and Morris indexes [27]. In addition, VARS calculates the Integrated Variogram Across a Range of Scales index (IVARS<sub>50</sub>). IVARS<sub>50</sub> provides a sensitivity index corresponding to intermediate scales and is calculated from the directional variograms according to:

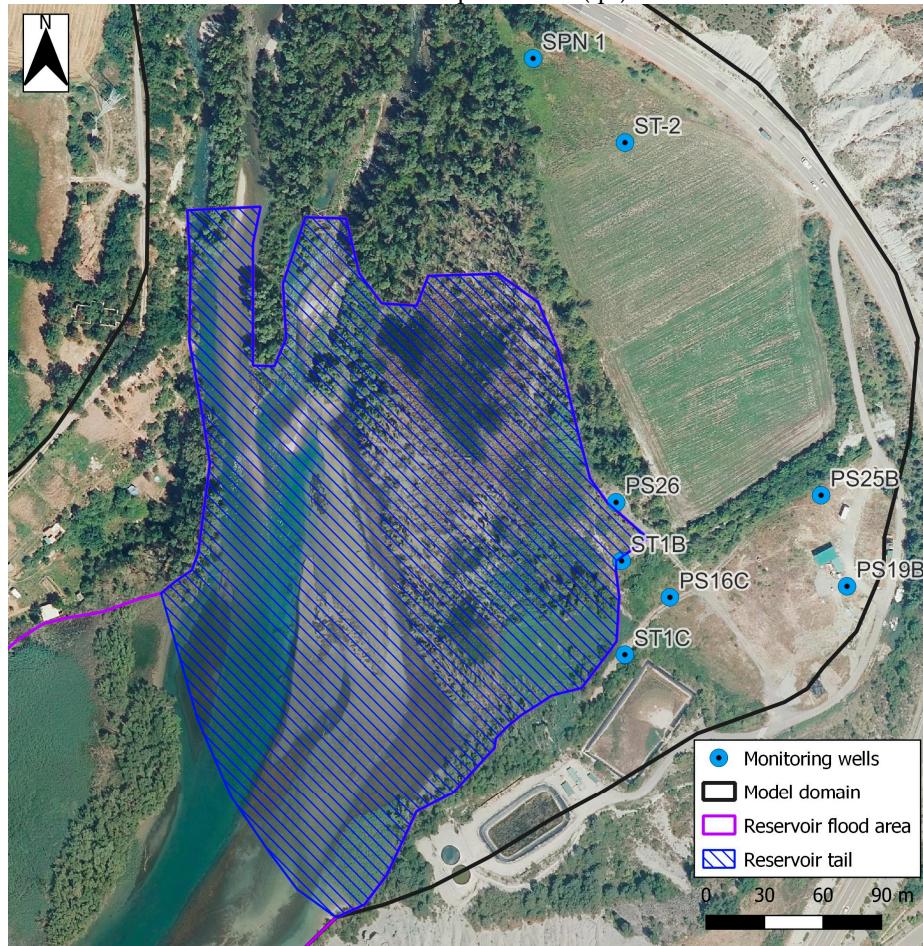
$$IVARS_{50} = \int_0^{0.5} \gamma(h) dh \quad (3)$$

where  $\gamma(h)$  is the directional variogram and  $h$  is the normalized input parameter. It should be recalled that the VARS method requires that all the input parameters are normalized to range from 0 to 1.

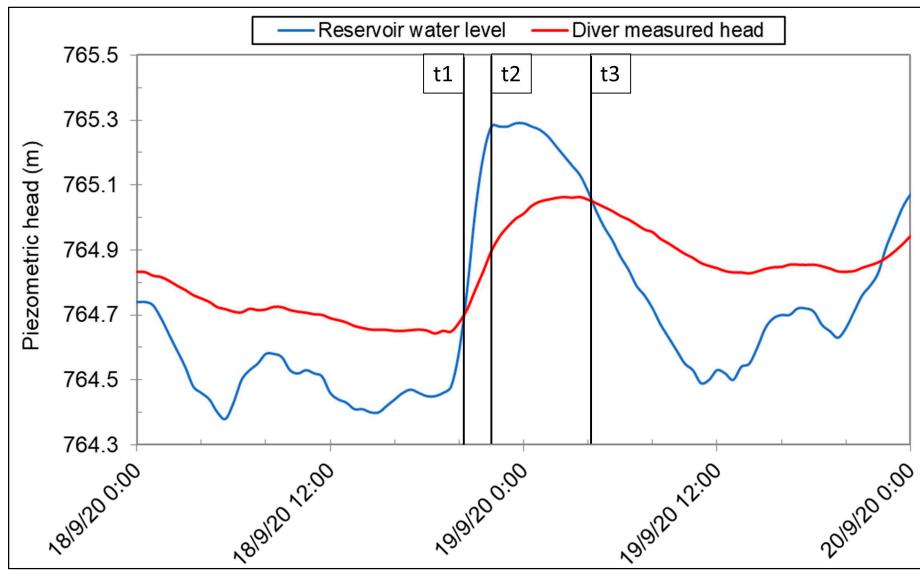
### 2.6. Inputs and Outputs

The input parameters considered in the GSA include (Figure 3): (1) The hydraulic conductivities of the aquifer in four material zones which were defined to account for the spatial heterogeneity of the alluvial aquifer ( $K_1, K_2, K_3$  and  $K_4$ ); (2) The aquifer specific storage ( $S_s$ ); (3) The vertical hydraulic conductivities of the aquitard interposed between the aquifer and the reservoir which includes the silting sediments settled along the former course of the Gállego river ( $K_{Vs1}$ ), the silting sediments in the rest of the reservoir ( $K_{Vs2}$ ) and the alluvial silts ( $K_{Vs3}$ ); (4) The leakage coefficients of the aquifer/river interactions ( $\alpha_r$ ) and the conductance for the flow underneath the dam ( $\alpha_d$ ); (5) Aquifer recharge rates from precipitation in confined ( $r_c$ ) and unconfined ( $r_u$ ) parts of the aquifer; and (6) Boundary inflows ( $Q_6, Q_7, Q_9, Q_2$  and  $Q_1$ ) (Figure 3). The ranges and statistical distributions of the input parameters are listed in Table 1. Log-uniform probability density functions were used for the hydrodynamic parameters and leakage coefficients, and uniform distributions were adopted for recharge rates and boundary inflows (Table 1).

The output variables considered in the analysis include the global mean absolute error in hydraulic heads (MAE<sub>g</sub>), the global root mean square error in hydraulic heads normalized by the standard deviation of the measured head data (NRMSE<sub>g</sub>) and the global Nash-Sutcliffe index [54] (NSE<sub>g</sub>) of the head residuals (measured minus computed heads) at 8 monitoring wells (Figure 4). Other output variables include the computed piezometric heads in wells ST1C, PS19 and SPN1 at 3 selected times, t<sub>1</sub>, t<sub>2</sub> and t<sub>3</sub>, which correspond to an event of sudden rise of the reservoir water level which took place from September 18, 2020 at 20:30 to September 19, 2020, at 04:30 (Figure 5). The heads in these 3 wells at the 3 selected times are denoted as ST1C<sub>t1</sub>, ST1C<sub>t2</sub>, ST1C<sub>t3</sub>, PS19B<sub>t1</sub>, PS19B<sub>t2</sub>, PS19B<sub>t3</sub>, SPN1<sub>t1</sub>, SPN1<sub>t2</sub> and SPN1<sub>t3</sub>. Other output variables include the aquifer/reservoir fluxes at times t<sub>1</sub>, t<sub>2</sub> and t<sub>3</sub>, which are denoted as Q<sub>t1</sub>, Q<sub>t2</sub> and Q<sub>t3</sub>, and the average modulus of the Darcy velocity near well PS16C where a tracer test was performed ( $q_{av}$ ).



**Figure 4.** Map showing the reservoir tail area (hatched blue polygon) where aquifer/reservoir fluxes were calculated at times t<sub>1</sub>, t<sub>2</sub> and t<sub>3</sub>, the monitoring wells whose piezometric data were used to calculate the calibration metrics, monitoring wells ST1C, PS19B, SPN1 and PS16C (where the average Darcy velocity is computed).



**Figure 5.** Measured reservoir hydrograph and piezometric heads in well ST1C from September 18, 2020, to September 20, 2020. The computed piezometric heads in monitoring wells ST1C, PS19B and SPN1 and the aquifer/reservoir fluxes are analyzed at the following times: 1) t1, September 18, 2020, 20:00 (low reservoir water level), 2) t2, September 18, 2020, 22:30 (peak reservoir water level) and 3) t3, September 19, 2020, 04:30 (descending reservoir water level).

The model head residuals were calculated as the differences between measured and computed heads using recorded piezometric head data from ST1B, ST1C, ST2, SPN1, PS16C, PS19B, PS25B, and PS26 wells. The mean absolute error (MAE), the root mean squared error normalized by the standard deviation of the measured data (NRMSE), and the Nash–Sutcliffe index (NSE) were computed for each well (j). The metrics were globalized to evaluate goodness of fit in the eight wells equipped with divers:

$$MAE_g = \frac{\sum_j MAE_j}{N_w} \quad (4)$$

$$NRMSE_g = \frac{\sqrt{\frac{1}{N_T} \sum_j N_j RMSE_j^2}}{\sigma_T} \quad (5)$$

$$NSE_g = 1 + \frac{\sum_j N_j \sigma_j^2 (NSE_j - 1)}{N_T \cdot \sigma_T^2} \quad (6)$$

where:

- $N_w$  is the number of monitoring wells
- $N_j$  is the number of measured piezometric heads in the j-th well
- $N_T$  is the total number of measured piezometric heads in all the wells
- $\sigma_j$  is the standard deviation of the measured piezometric heads in the j-th well
- $MAE_j$ ,  $RMSE_j$  and  $NSE_j$  are the mean absolute error, the root mean squared error and the Nash–Sutcliffe index for the j-th well,
- $\sigma_T$  is the standard deviation of the measured piezometric heads in all wells

## 2.7. Global Sensitivity Simulation Runs

Global sensitivity simulation runs were generated by using two different sequences. A first set of 30 800 runs were prepared for VARS by using the Halton sequence to generate the sequence of star centers [50]. The second sequence of 16 384 runs was obtained by using the Sobol method [55]. The runs were performed at the Galician Supercomputing Center (CESGA).

## 2.8. Software

Model simulations were performed with CORE<sup>2D</sup> V5, a finite element code for transient saturated and unsaturated water flow and heat transport in heterogeneous and anisotropic porous and fractured media [56]. The code solves for groundwater flow and solute transport by using Galerkin triangular finite elements and an Euler time discretization scheme. CORE<sup>2D</sup> V5 has undergone extensive verification against analytical solutions and has been benchmarked against other reactive transport codes [57,58]. CORE<sup>2D</sup>V5 has been used extensively for modeling groundwater flow and solute transport in aquifers and lab and in situ experiments [59,60].

The single processor code CORE<sup>2D</sup> V5 was compiled at the CESGA by using the FinisTerra-III supercomputer [61,62]. This advanced computing infrastructure supports the concurrent execution of hundreds of simultaneous and parallel model runs. The simulation of the VARS-Halton and Sobol sequences runs took 6 days of computing wall time. The same task would have taken more than three years running on a personal computer.

VARS version 2.1 was run under MATLAB® version 2022a in an UBUNTU 20.04.6 LTS system. HDMR computations were carried out by using both SALib [63] and GUI-HDMR V1.1 [64].

## 3. Results and Discussion

### 3.1. Groundwater Flow Model Results

The computed hydraulic gradient at the Sardas site downstream the landfill is very small due to the large hydraulic conductivity of the layer of sands and gravels, and the small groundwater inflows [38]. The daily fluctuations of the reservoir water level induce a tidal effect on the piezometric heads of the aquifer. The fluctuations in the aquifer are damped and delayed because the aquitard acts as a dumping barrier between the reservoir and the aquifer [38]. The amplitude of the oscillations of the piezometric heads,  $A_h$  and the time lag,  $t_R$ , are very sensitive to the specific storage coefficient of the layer of sands and gravels ( $S_s$ ), and the vertical hydraulic conductivities of the aquitard ( $K_{Vs1}$ ,  $K_{Vs2}$  and  $K_{Vs3}$ ) [38].

The reservoir water level oscillates between 764.31 m and 765.43 m daily in the modelling period. The average water level in the reservoir (764.80 m) is slightly smaller than the piezometric head in the aquifer (764.90 m). Groundwater generally flows from the aquifer to the silting sediments and the alluvial silts of the reservoir. However, when the reservoir water level rises above the piezometric head in the aquifer, the Sabiñánigo reservoir recharges the aquifer, resulting in a rise in piezometric heads. The modulus of the Darcy velocity in the aquifer decreases during these high-reservoir water level events. If the duration of the high water level is long enough, the direction of groundwater flow may reverse near the reservoir [38,41].

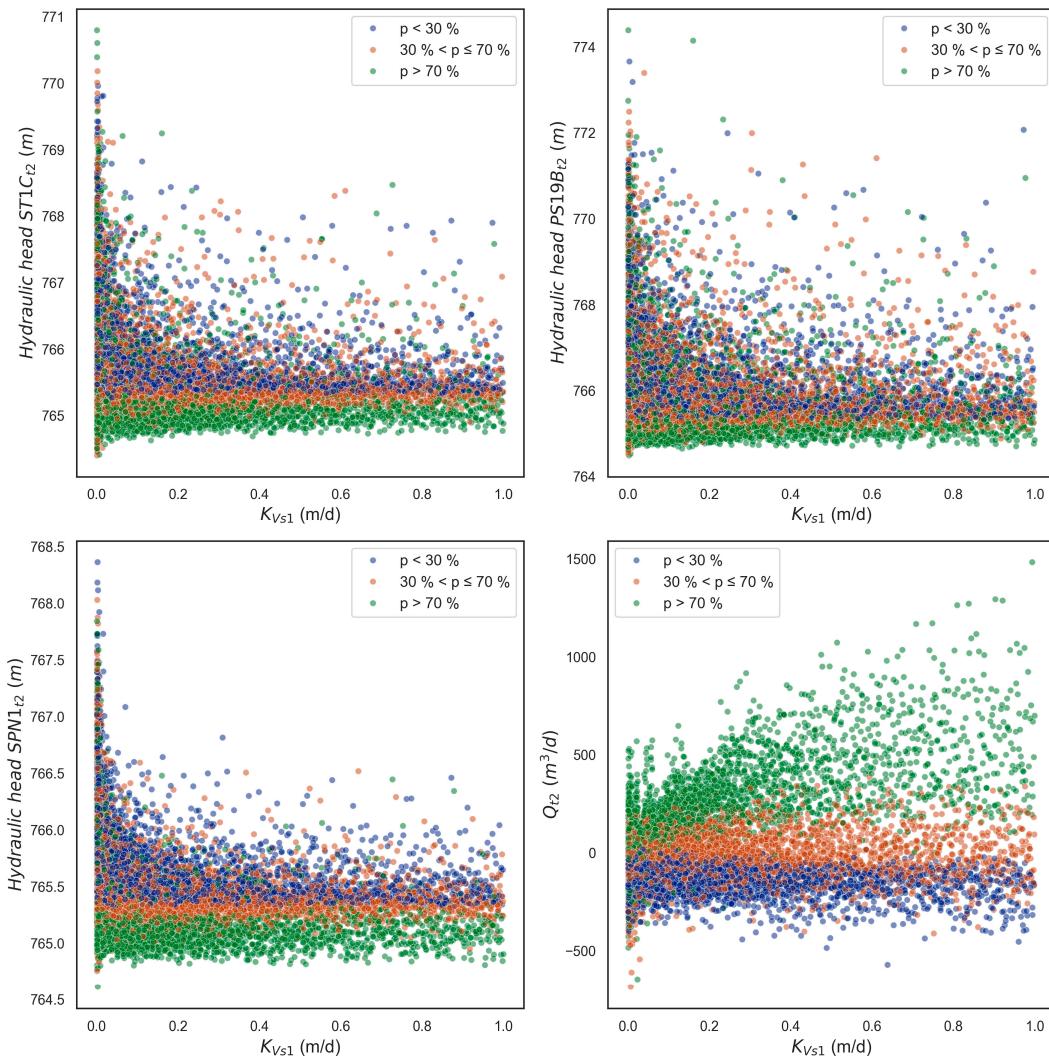
### 3.2. GSA Results for the Groundwater Flow Model of the Gállego Alluvial Aquifer

#### 3.2.1. Graphical Methods

Two-variable scatterplots are useful to identify and illustrate the interactions of two input parameters. Figure 6 shows two-variable scatterplots of the computed heads in 3 monitoring wells s (outputs ST1C<sub>t2</sub>, PS19B<sub>t2</sub> and SPN1<sub>t2</sub>) and the aquifer/reservoir flow ( $Q_{t2}$ ) at time  $t_2$  corresponding the peak reservoir water level. The outputs are plotted versus the vertical conductivity of the aquitard ( $K_{Vs1}$ ). These plots correspond to the 16 384 runs of the Sobol sequence. The clouds of plots are shown for the following three ranges of percentiles,  $p$ , of the specific storage coefficient ( $S_s$ ): 1)  $p < 30\%$ ; 2)  $30\% < p < 70\%$  and 3)  $p > 70\%$ .

The scatterplots show that the computed heads and the aquifer/reservoir flow at time  $t_2$  depend on  $S_s$ . Generally, the computed piezometric heads in wells ST1C and SPN1 at  $t_2$  are low for large  $S_s$  ( $p > 70\%$ ). However, simulations corresponding to extreme values of other input parameters result in high piezometric heads even for high values of  $S_s$ . On the other hand, the computed heads are higher when  $S_s$  is in the lower percentile ( $p < 30\%$ ). It should be noticed that the well PS19B is located

further away from the reservoir, and thus, its piezometric head is less affected by the interaction between  $K_{Vs1}$  and  $Ss$ .

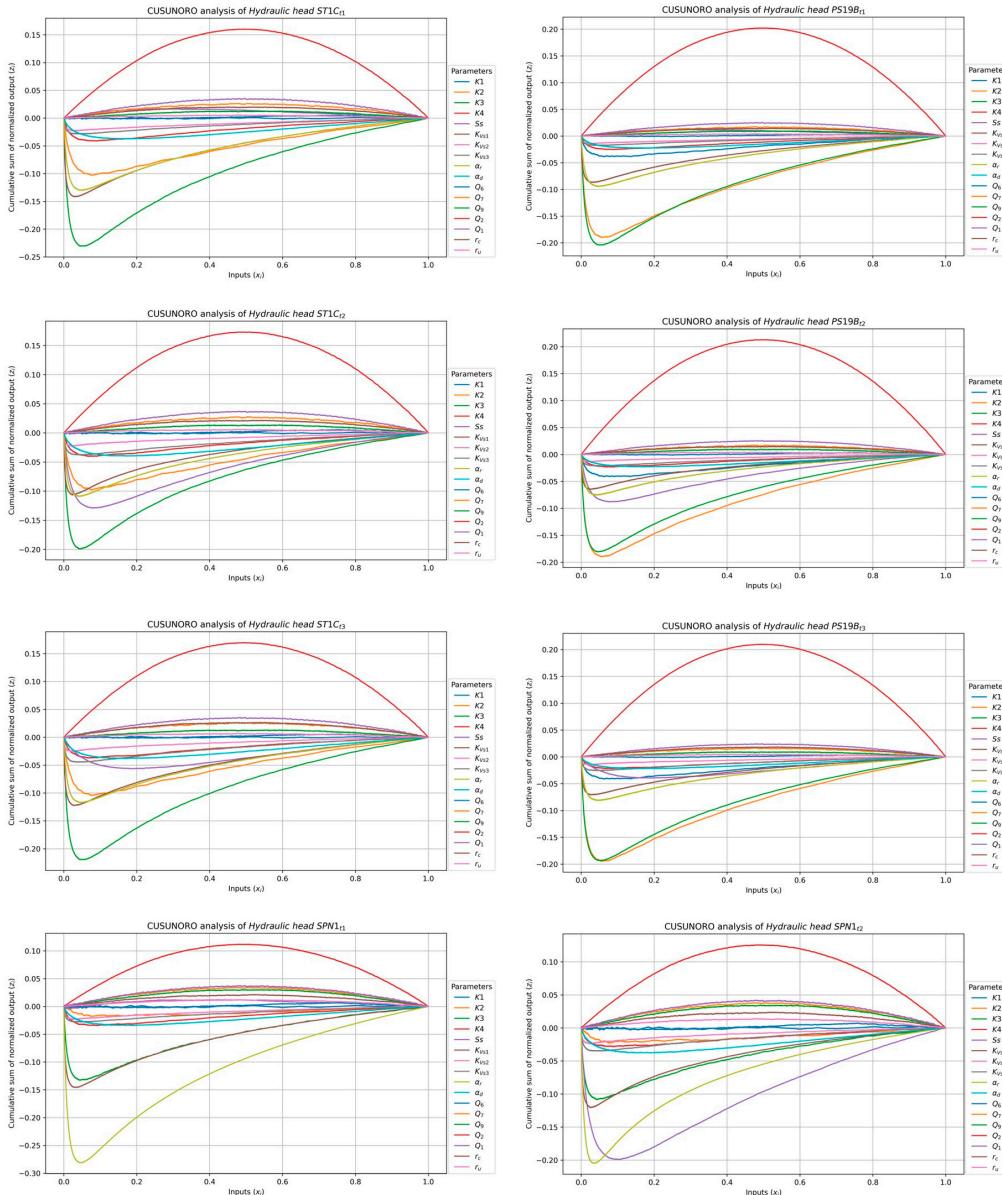


**Figure 6.** Scatterplots of the computed piezometric heads in wells ST1Ct2 (upper left plot), PS19Bt2 (upper right plot), SPN1t2 (lower left plot), and  $Q_{t2}$  (lower right plot) versus the vertical hydraulic conductivity of the silting sediments in the former river course ( $K_{Vs1}$ ). The sample of 16384 points was generated with a Sobol sequence. The clouds of plots are shown for the following three ranges of percentiles,  $p$ , of the specific storage coefficient ( $Ss$ ): 1)  $p < 30\%$ ; 2)  $30\% < p \leq 70\%$  and 3)  $p > 70\%$ .

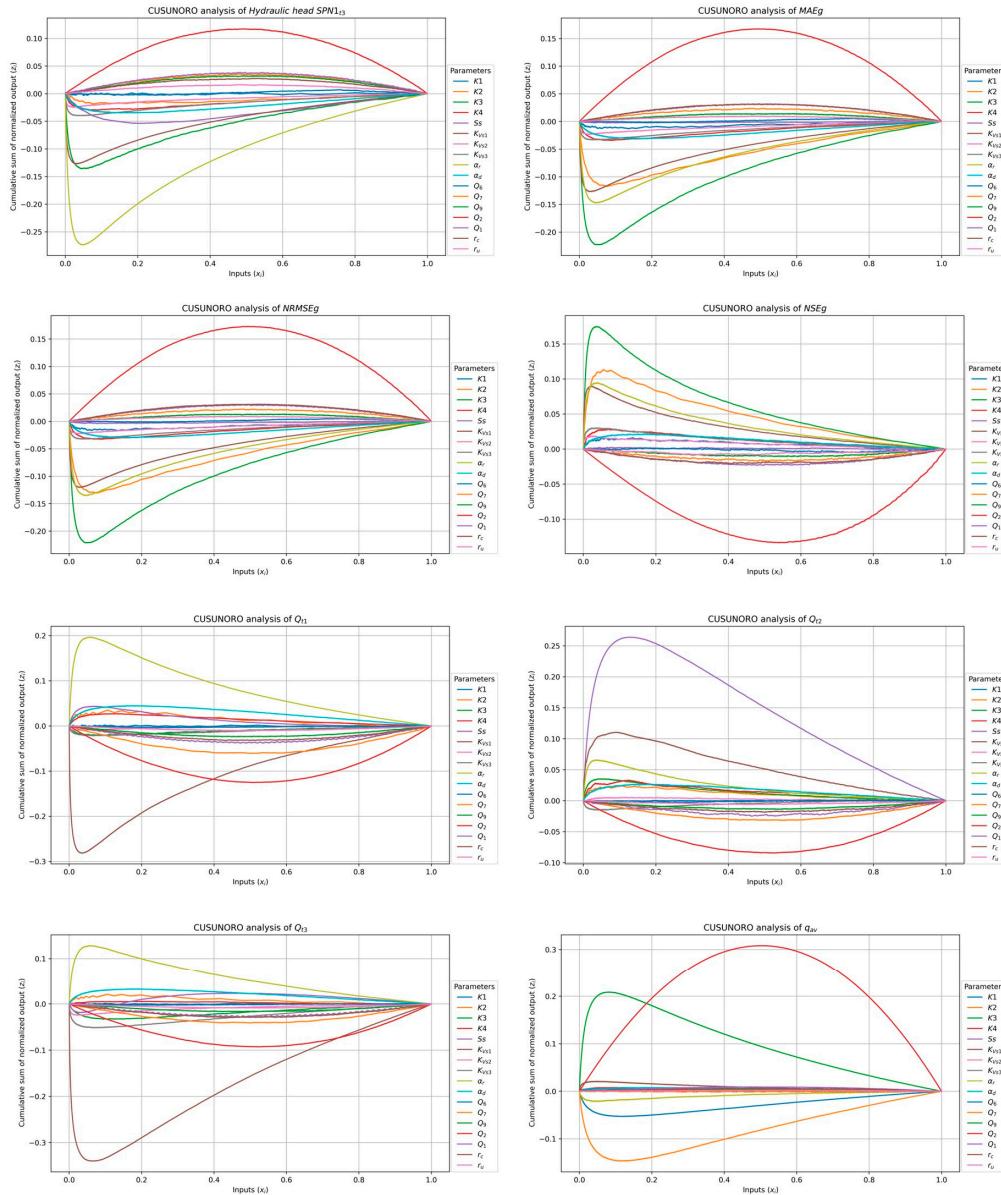
The scatterplot of  $Q_{t2}$  shows three branches. The most negative flows, which correspond to aquifer discharge into the reservoir, at time  $t2$  are associated with low values of  $Ss$  ( $p < 30\%$ ) while the largest positive water flows (flow from the reservoir into the aquifer) occur for high  $Ss$  values ( $p > 70\%$ ). The water flow oscillates from positive (aquifer recharge) to negative (aquifer discharge) for intermediate values of  $Ss$  ( $30\% < p < 70\%$ ). The amplitude of the oscillations of the piezometric heads in the aquifer,  $A_h$ , is inversely proportional to the value of the specific storage of the aquifer. When  $Ss$  is low,  $A_h$  is large and the piezometric heads in the alluvial increase quickly and may rise above the reservoir water level, thus leading to groundwater discharge to the reservoir. On the other hand, when  $Ss$  is large,  $A_h$  is small and the piezometric heads increase slowly. Here, piezometric heads can hardly rise above the reservoir water level during the events of rise of the reservoir water level, Then, the reservoir recharges the aquifer.

Figures 7 and 8 show CUSUNORO curves for computed heads in wells ST1C, PS19B and SPN1 at times t1, t2 and t3 and MAEg, NRMSEg, NSEg,  $Q_{t1}$ ,  $Q_{t2}$ ,  $Q_{t3}$  and  $q_{av}$ .

From the CUSUNORO curves above, parameters K3 (green), Q<sub>2</sub> (red), and K2 (orange) consistently exhibit the highest maximum absolute values across all hydraulic head plots, except for the hydraulic head at well SPN1, where parameters  $\alpha_r$  (yellow),  $K_{v1}$  (brown) and S<sub>s</sub> (purple) are more influential. Q<sub>6</sub> (blue) and K1 (blue) typically show the lowest maximum absolute values (not shown here).



Most of the CUSUNOTO curves do no shot crossings of the x-axis. This attests the means the monotonicity of the outputs versus the input parameters. Some crossings of the x-axis are found in the curves of the least influential parameters, especially for  $Q_6$  and  $K1$  (not shown here).

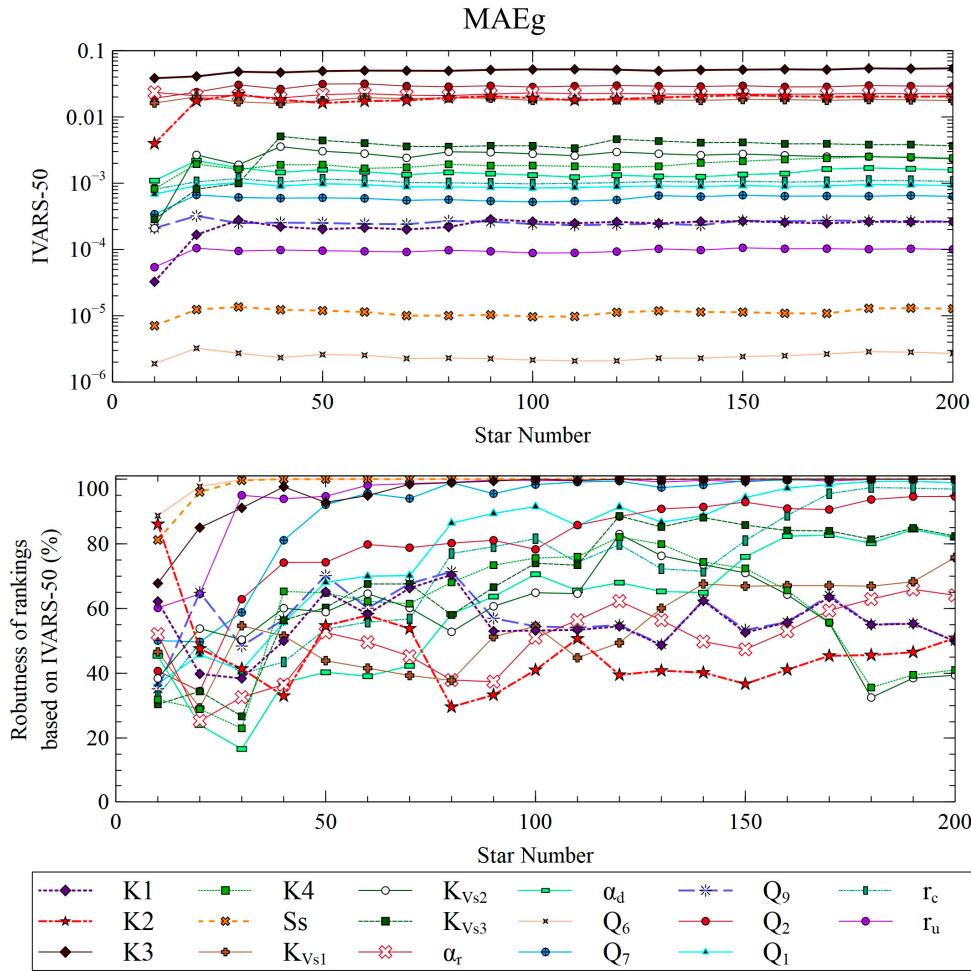


**Figure 8.** CUSUNORO curves of the computed head in well SPN1 at time t3, MAEg, NRMSEg, NSEg,  $Q_{H1}$ ,  $Q_{I2}$ ,  $Q_{I3}$  and  $q_{av}$ .

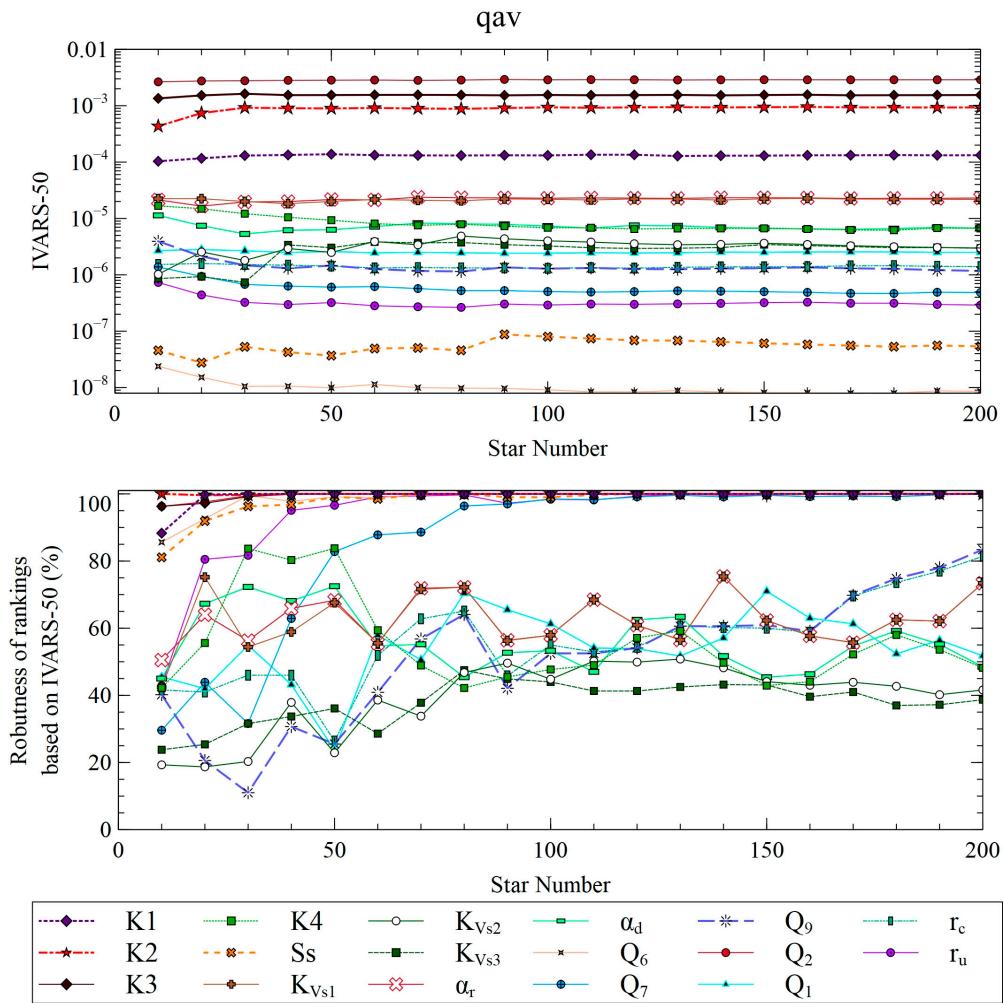
### 3.2.2. VARS Results

Figures 9 and 10 show the IVARS<sub>50</sub> indexes for the global mean absolute error (MAE<sub>g</sub>), and the average groundwater Darcy velocity modulus ( $q_{av}$ ) as a function of the number of star centers. They also show the ranking of the input parameters and the robustness of ranking. It should be noticed that IVARS<sub>50</sub> achieves stable values after just 50 star centers, which amounts to 7700 runs. The largest sensitivity indexes for MAE<sub>g</sub> correspond to the aquifer hydraulic conductivity in material zone 3 (K<sub>3</sub>), the boundary inflow Q<sub>2</sub> which corresponds to groundwater flow coming from the Sardas landfill, the vertical conductivity of the silts (K<sub>Vs1</sub>) and the aquifer/river conductance ( $\alpha_r$ ). IVARS<sub>50</sub> of

the Darcy velocity,  $q_{av}$ , is largest for  $Q_2$ ,  $K3$  and  $K2$ . The rest of the parameters have much smaller indexes.



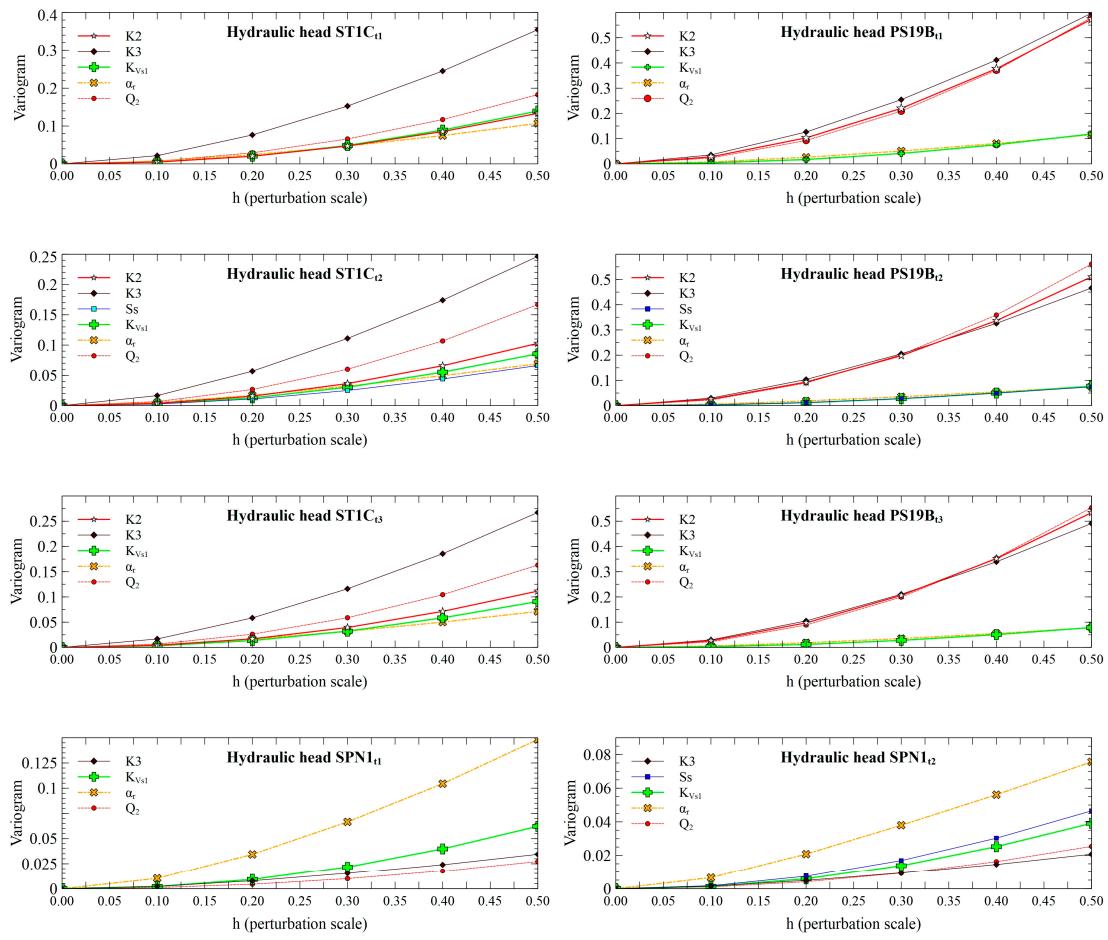
**Figure 9.** IVARS<sub>50</sub> indexes of input parameters as a function of the number of star centers for MAEg (upper plot), and robustness of ranking as a function of the number of star centers (bottom plot).



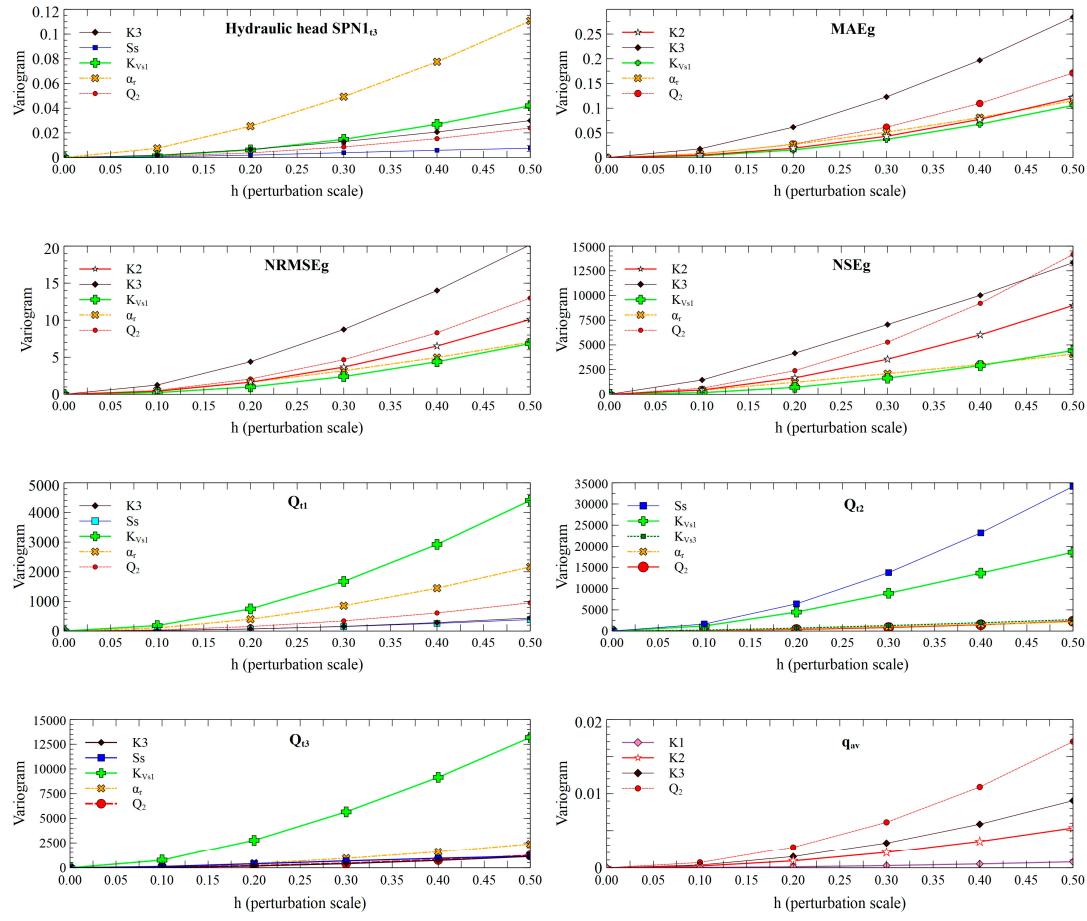
**Figure 10.** IVARS<sub>50</sub> indexes of input parameters as a function of the number of star centers for the average Darcy velocity ( $q_{av}$ ) (upper plot), and robustness of ranking as a function of the number of star centers (bottom plot).

The robustness of ranking of input parameters K2, K3, S<sub>s</sub>, r<sub>u</sub>, and Q<sub>7</sub> for both outputs is greater than 90% after 50 stars. However, for the rest of the input parameters, the robustness is smaller than 80 %, and, sometimes, not stable even after 200 stars. Input parameters  $\alpha_r$  and K<sub>Vs1</sub> are very influential for MAEg, but they show a robustness measure that ranges from 50 to 70 % after 100 star centers. Robustness does not directly increase with the number of star centers for some variables. This could be caused by interactions among S<sub>s</sub>, aquitard vertical conductivity K<sub>Vs1</sub> and aquifer/river conductance  $\alpha_r$  at some water level rise events. Another reason for the lack of stability of the robustness of rankings is that the least relevant variables interfere with the calculation of sensitivity indexes of the most relevant parameters. However, the rankings of the most and the least significant input parameters are stable with just 50 star centers (7700 runs). Input parameters with intermediate influence are also well identified, despite not being ranked reliably.

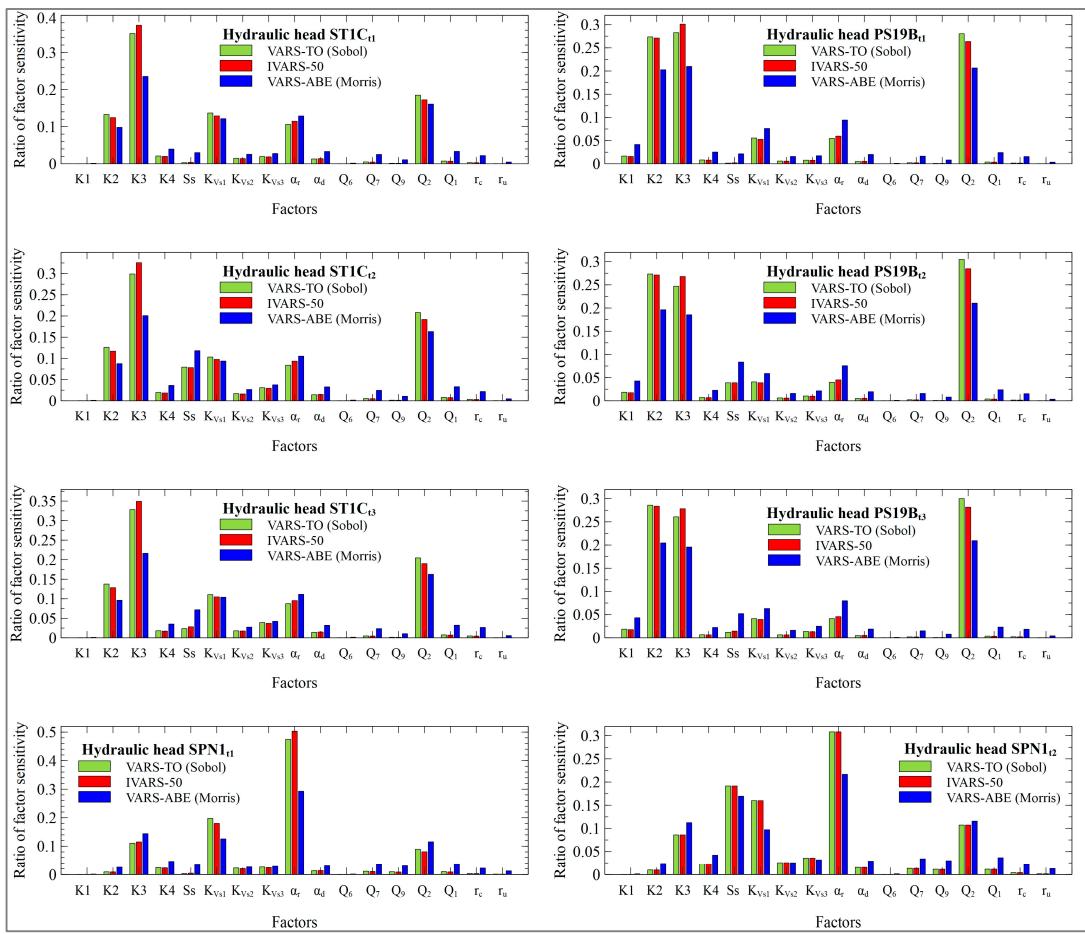
Sample variograms along the directions of the 17 input parameters were computed for the piezometric heads in wells ST1C, PS19B and SPN1 at times t<sub>1</sub>, t<sub>2</sub> and t<sub>3</sub>, the calibration metrics, the aquifer/reservoir fluxes at times t<sub>1</sub>, t<sub>2</sub> and t<sub>3</sub> and the average groundwater Darcy velocity modulus near well PS16C. Only a limited number of parameters are relevant for each output variance. Figures 11 and 12 show the sample variograms along the directions of the 5 most influential parameters. VARS-TO, IVARS<sub>50</sub> and VARS-ABE metrics are computed from the variograms. Figures 13 and 14 show the VARS-ABE, IVARS<sub>50</sub> and VARS-TO indexes for each output considering all the input parameters.



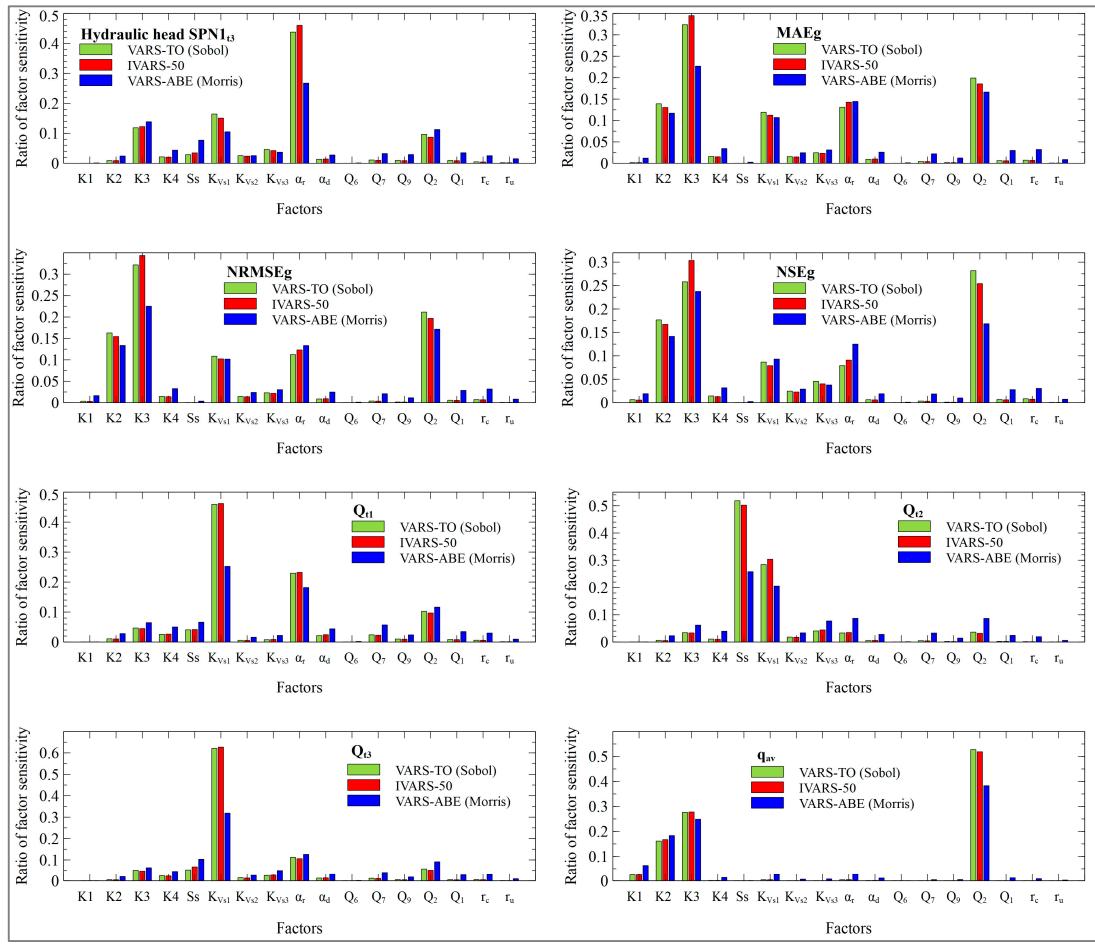
**Figure 11.** Sample variograms of the computed heads in monitoring wells ST1C and PS19B at times  $t_1$ ,  $t_2$  and  $t_3$  and monitoring well SPN1 at times  $t_1$  and  $t_2$ . Only the variograms of the five most influential parameters are shown in the plots.



**Figure 12.** Sample variograms of the computed head in well SPN1 at time t3, MAEg, NRMSEg, NSEg,  $Q_{11}$ ,  $Q_{12}$ ,  $Q_{13}$  and  $q_{av}$ . Only the variograms of the five most influential parameters are shown in the plots.



**Figure 13.** VARS-TO, IVARS<sub>50</sub> and VARS-ABE indexes for the computed heads in wells ST1C and PS19B at times t1, t2 and well SPN1 at times t1 and t2.



**Figure 14.** VARS-TO, IVARS<sub>50</sub> and VARS-ABE indexes for the computed head in well SPN1 at time t3, MAEg, NRMSEg, NSEg,  $Q_{t1}$ ,  $Q_{t2}$ ,  $Q_{t3}$  and  $q_{av}$ .

The most influential parameters for the computed head in well ST1C at times t1, t2 and t3 are K3 and Q<sub>2</sub>, followed by K<sub>Vs1</sub>,  $\alpha_r$  and K2. The ranking in well PS19B is similar to the ranking in well ST1C. However, the sensitivity indexes of K3, K2 and Q<sub>2</sub> in well ST1C are much larger than those of well PS19B. K3 is much more relevant than K2 in well ST1C while K2 and K3 have almost similar sensitivity indexes in well PS19B. The sensitivity indexes of the hydraulic conductivities depend on the location of the wells. Material zone 3 is the largest zone of the Sardas site. Material zone 2 is located between material zones 1 and 3. Material zone 1 is much smaller than the other two and is located just downstream the Sardas landfill. K<sub>Vs1</sub> is more influential for the head in well ST1C than for the well PS19B. ST1C is located right next to the reservoir maximum flood area and PS19B is located 135 m to the east of the reservoir. Tidal effects on the aquifer depend on the duration of the high reservoir level, its amplitude, and the distance to the reservoir. Likewise, the head in well PS19B is most sensitive to Q<sub>2</sub> because this well is near the boundary just downstream the Sardas landfill.

The sensitivity indexes for the computed heads in well SPN1 differ from those of other monitoring wells because well SPN1 is near the Gállego riverbed.  $\alpha_r$  is the most influential parameter in well SPN1 followed by K<sub>Vs1</sub>.

Despite not being an influential input, the relevance of S<sub>s</sub> for the computed heads in the monitoring wells increases from time t1 (low level) to time t2 (peak reservoir water level). The sensitivity indexes of S<sub>s</sub> decrease when the reservoir water level descends at time t3. The time change of the sensitivity of S<sub>s</sub> in well SPN1 is more significant than in wells ST1C and PS19B.

The most influential parameters for the calibration metrics (MAEg, NRMSEg and NSEg) are K3, K2 and Q<sub>2</sub>, followed by K<sub>Vs1</sub> and  $\alpha_r$ . Most of the monitoring wells are located in material zones 2 and 3 and near the boundary zone corresponding to Q<sub>2</sub>. Aquifer/reservoir and aquifer/river interactions affect the computed head gradient,  $\alpha_r$  and A<sub>h</sub>. S<sub>s</sub> only affects the temporal variability of computed heads and not their average values. Since calibration metrics evaluate the average fit of the computed heads in the monitoring wells, the relevance of S<sub>s</sub> for the calibration metrics is very low compared to its relevance on the computed heads at specific times. S<sub>s</sub> is especially relevant during events of sudden rise of the reservoir water level.

To facilitate the interpretation of the sensitivities of reservoir/aquifer fluxes (Q<sub>t1</sub>, Q<sub>t2</sub>, Q<sub>t3</sub>), one should recall that the silting sediments (K<sub>Vs1</sub> and K<sub>Vs2</sub>) are more permeable than alluvial silts (K<sub>Vs3</sub>). Groundwater discharges mainly through the former course of the Gállego river, where only silting sediments confine the aquifer (K<sub>Vs1</sub>). If the aquifer is less connected to the Gállego river (lower values of  $\alpha_r$ ), groundwater discharges to the reservoir. On the other hand, if the aquifer and the river are more connected (higher values of  $\alpha_r$ ), groundwater discharges to the reservoir and the river. As expected, the greater the inflow of water from the landfill (Q<sub>2</sub>), the more groundwater discharge to the reservoir.

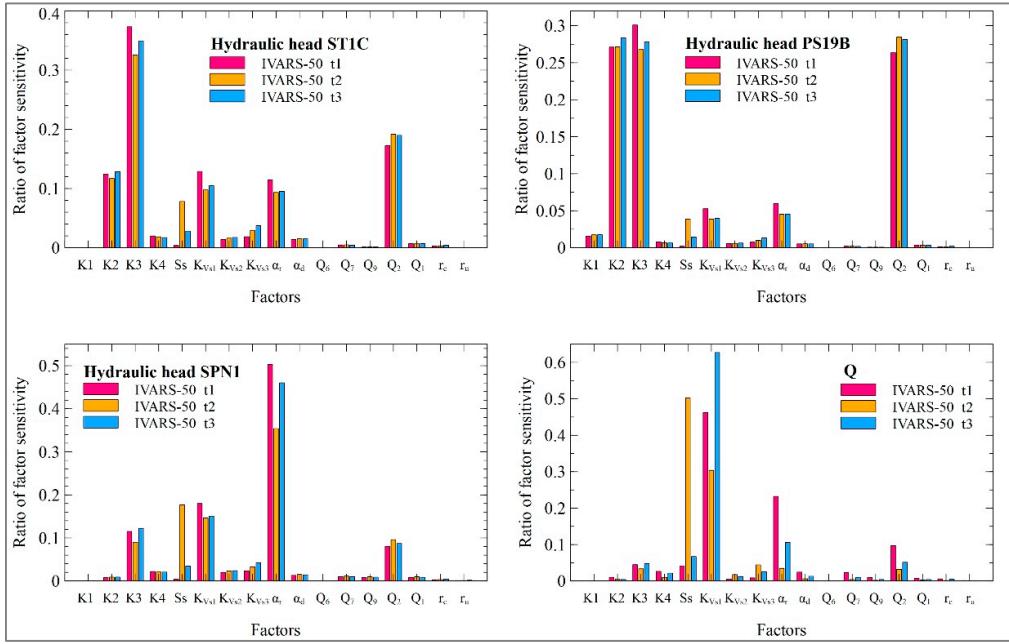
The aquifer/reservoir groundwater flow changes with time due to the reservoir tidal effect. The most influential parameters for the aquifer/reservoir groundwater flow are also time dependent. K<sub>Vs1</sub> is the parameter with the largest sensitivity index at time t1 when the reservoir water level is low. The second influential parameter is the aquifer/river leakage coefficient,  $\alpha_r$ . The boundary inflow Q<sub>2</sub> is the third most relevant parameter. However, when the reservoir water level rises suddenly rises at time t2, the parameter sensitivities change. The most influential parameter becomes S<sub>s</sub> followed by K<sub>Vs1</sub>. We recall that in the model reference run [38], there is flow from the reservoir into the aquifer when the reservoir water level rises above the piezometric head in the aquifer. If the specific storage of the aquifer is high, a larger part of the flow coming from the reservoir is stored in the aquifer, which results in a smaller rise of the piezometric head in the aquifer.

The reservoir flood area depends on its water level, so when the water level rises at time t2, the area where the alluvial silts confine the aquifer underneath the reservoir increases. The areas of the reservoir away from the former course of the Gállego river are assumed to be confined by both silting sediments (K<sub>Vs2</sub>) and alluvial silts (K<sub>Vs3</sub>). The relevance of K<sub>Vs3</sub> increases slightly when the reservoir floods more parts of the alluvial, but its relevance is still much smaller than those of K<sub>Vs1</sub> and S<sub>s</sub>. When the water level of the reservoir descends at time t3, the parameter sensitivities tend to be similar to those of Q<sub>t1</sub> at time t1. For Q<sub>t3</sub>, the sensitivity of K<sub>Vs1</sub> is much larger than those of  $\alpha_r$  and Q<sub>2</sub>. The sensitivity of S<sub>s</sub> remains, but it is much less relevant than for Q<sub>t2</sub>.

The most influential parameters for the average modulus of the Darcy velocity (q<sub>av</sub>) near well PS16C are Q<sub>2</sub>, K3 and K2. K1 is slightly relevant, and the contribution of the rest of the parameters is negligible. Darcy velocity in the aquifer mainly depends on the boundary inflow Q<sub>2</sub> because this well is located in material zone 3 and near the boundary downstream the Sardas landfill.

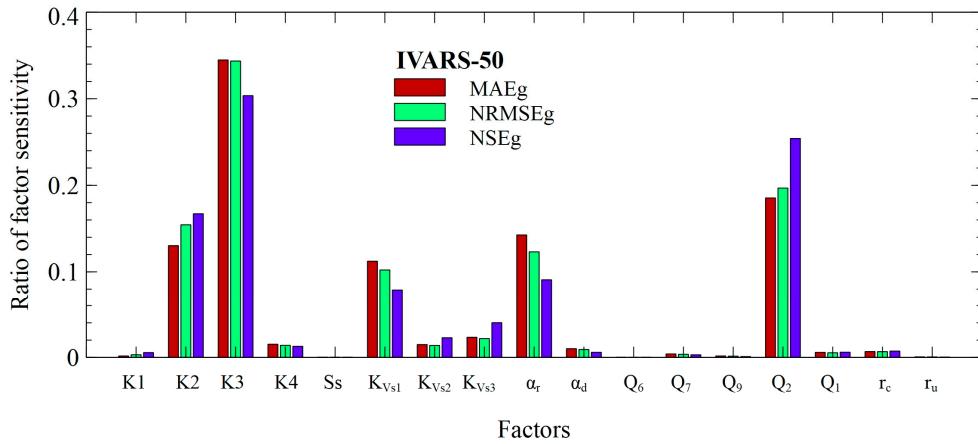
The sensitivity indexes of S<sub>s</sub> are time dependent for the computed heads in the wells, and the aquifer/reservoir fluxes (Figure 15). When the reservoir water level rises above the piezometric head in the aquifer at time t2, the reservoir starts recharging the aquifer, and the piezometric head in the alluvial starts rising. If the specific storage is small, the amplitude of the oscillation of the piezometric head increases. This affects especially the aquifer/reservoir fluxes (Q<sub>t1</sub>, Q<sub>t2</sub> and Q<sub>t3</sub>). The sensitivities of the computed head in well SPN1, located near the Gállego river, are largest for  $\alpha_r$  and K<sub>Vs1</sub> and S<sub>s</sub> at time t2.

On the other hand, well ST1C is closest to the reservoir and further away from the Gállego river. In this well the sensitivity index of S<sub>s</sub> is smaller than in well SPN1. Finally, the sensitivity of the head in well PS19B to S<sub>s</sub> is irrelevant because the well is far away from the Gállego river and the reservoir flood area.



**Figure 15.** IVARS<sub>50</sub> sensitivity indexes for computed heads in wells ST1C (top left plot), PS19B (top right plot), and SPN1 (bottom left plot) and aquifer/reservoir flow (bottom right plot) at times t1, t2 and t3.

The IVARS<sub>50</sub> sensitivity indexes for calibration metrics, MAE<sub>g</sub>, NRMSE<sub>g</sub> and NSE<sub>g</sub> are very similar (Figure 16). The most influential parameters are the same for all the metrics. The rankings show some slight differences. Input ranking for the MAE<sub>g</sub> is K3, Q<sub>2</sub>,  $\alpha_r$ , K2 and K<sub>Vs1</sub>, while  $\alpha_r$  and K2 switch places for NRMSE<sub>g</sub> and NSE<sub>g</sub>. NRMSE<sub>g</sub> and NSE<sub>g</sub> are more prone to the presence of outliers because their formulas include squared residuals.



**Figure 16.** IVARS<sub>50</sub> sensitivity indexes for calibration metrics MAE<sub>g</sub>, NRMSE<sub>g</sub> and NSE<sub>g</sub>.

Sensitivity indexes are large for K3, Q<sub>2</sub>,  $\alpha_r$ , K2 and K<sub>Vs1</sub>. Some input variables have very small sensitivity indexes.

### 3.2.3. HDMR Results and Analysis of Interactions for the Sobol Sequence

Tables S1 to S23 in the SM show the values of the first order (main effects),  $S_i$ , and 2<sup>nd</sup> order effects,  $S_{ij}$ , calculated with SALib [63] and GUI-HDMR [64] and the parameter ranking for: (1) The computed piezometric heads in wells PS19B, SPN1 and ST1C at times t1, t2 and t3; (2) The global

mean absolute error (MAEg); (3) The global Nash–Sutcliffe index (NSEg); (4) The global normalized root mean squared error (NRMSEg); (5) The average groundwater Darcy velocity modulus near well PS16C ( $q_{av}$ ); and (6) The computed aquifer/reservoir fluxes at times t1, t2 and t3 ( $Q_{t1}$ ,  $Q_{t2}$  and  $Q_{t3}$ ) by using 16 384 runs.

HDMR lower bound estimates for the total effects, ( $S_i + S_{ij}$ ), are compared to the VARS intervals of the total effects for each parameter. SALib and GUI-HDMR indexes generally agree, although they show some small differences. The analysis of the interactions among parameters is an important part of the global sensitivity analysis. Interactions represent the joint influence of parameters on the model outputs. Usually, interactions are revealed when the sum of the Sobol's 1<sup>st</sup> order indexes are significantly smaller than 1.

In the next paragraphs the Sobol based lower bound estimates for the total effects will be denoted simply as "total effects" to shorten the presentation of the HDMR results. The largest main effect  $S_i$  for the piezometric head in well ST1C at time t1 is equal to 0.242 and corresponds to K3 (see Table S1 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.644. The total effects of the piezometric head in well ST1C at time t1 are slightly larger than 1 (see Table S1 in SM). Second order effects for this output are important especially due to the interaction between K3 and K2. Interactions of smaller relevance occur between  $Q_2$  and K3, between  $K_{Vs1}$  and  $\alpha_r$ , and between  $Q_2$  and K2. Sobol total effects  $S_{Ti}$  are typically larger and fall outside the ranges of the VARS total effects.

The largest main effect  $S_i$  of the piezometric head in well ST1C at time t2 is equal to 0.194 and corresponds to K3 (see Table S2 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.628. The total effects of the piezometric head in well ST1C at time t2 are slightly larger than 1 (see Table S2 in SM). Second order effects here are relevant especially due to the interaction between  $Q_2$  and K3. There are also interactions of smaller relevance between K3 and K2, between  $K_{Vs1}$  and  $\alpha_r$ , and between  $Q_2$  and K2. Sobol total effects  $S_{Ti}$  and those of VARS generally agree, although they show some discrepancies especially for the K1 and  $r_u$ .

The largest main effects  $S_i$  for the piezometric head in well ST1C at time t3 correspond to K3 and  $Q_2$  (see Table S3 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.644. The total effects of the piezometric head in well ST1C at time t3 are equal to 1.223 (see Table S3 in SM). Second order effects for this output are relevant due to interactions of K3 with K2 and  $Q_2$ , and  $K_{Vs1}$  with  $\alpha_r$ . The Sobol total effects  $S_{Ti}$  generally fall within the intervals of VARS total effects.

The largest main effects  $S_i$  for the piezometric head in well PS19B at time t1 correspond to  $Q_2$ , K3 and K2 (see Table S4 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.71. The total effects  $S_{Ti}$  for the piezometric head in well PS19B at time t1 are slightly greater than 1 (see Table S4 in SM). Second order effects here are relevant especially due to the interactions among  $Q_2$ , K3 and K2. The total effects  $S_{Ti}$  fall within the intervals of VARS total effects.

The largest main effects  $S_i$  for the piezometric head in well PS19B at time t2 correspond to  $Q_2$ , K2, and K3 (see Table S5 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.699. The total effects  $S_{Ti}$  for the piezometric head in well PS19B at time t2 are slightly greater than 1 (see Table S5 in SM). Second order effects for this output are important especially due to the interaction between  $Q_2$  and K2. There are also interactions of smaller relevance between  $Q_2$  and K3, between K3 and K2, and between  $\alpha_r$  and  $K_{Vs1}$ . The Sobol total effects  $S_{Ti}$  are typically larger and fall outside the ranges of the VARS total effects.

The largest main effect  $S_i$  for the piezometric head in well PS19B at time t3 corresponds to  $Q_2$ . K2 and K3 are in second and third position (see Table S6 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.703. Second order effects for this output are important especially due to the interaction between  $Q_2$  and K2. There are also interactions of smaller relevance between  $Q_2$  and K3, between K3 and K2, and between  $\alpha_r$  and  $K_{Vs1}$ . The Sobol total effects  $S_{Ti}$  are generally larger and out of the intervals of the VARS estimates.

$\alpha_r$  shows the largest main effect  $S_i$  for the piezometric head in well SPN1 at time t1 which is equal to 0.394. The second largest effect corresponds to  $K_{Vs1}$  (see Table S7 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.69. The total effects of the piezometric head in well SPN1 at time t1 are close to 1 (see Table S7 in SM). Second order effects for this output are relevant especially due to the

interaction between  $Q_2$  and  $K3$ . The Sobol total effects  $S_{Ti}$  are overall higher and beyond the limits of the VARS estimates.

The largest main effects  $S_i$  for the piezometric head in well SPN1 at time  $t2$  correspond to  $\alpha_r$  and  $S_s$  (see Table S8 in SM). The sum of the Sobol's 1<sup>st</sup> order indexes is equal to 0.655. The total effects of the piezometric head in well SPN1 at time  $t2$  are slightly larger than 1 (see Table S8 in SM). Second order effects for this output are relevant especially due to the interaction between  $\alpha_r$  and  $K_{Vs1}$ . There are also smaller interactions of  $Q_2$  with  $K3$  and with  $\alpha_r$ . The Sobol total effects are generally larger and out of the intervals of the VARS estimates.

The largest main effects  $S_i$  for the piezometric head in well SPN1 at time  $t3$  correspond to  $\alpha_r$  and  $K3$  (see Table S9 in SM). The sum of the Sobol 1<sup>st</sup> order indexes is equal to 0.679. The total effects of the piezometric head in well SPN1  $t3$  are close to 1 (see Table S9 in SM). Second order effects are relevant mainly due to the interaction between  $\alpha_r$  and  $K_{Vs1}$ , and to a lower extent to the interactions of  $Q_2$  with  $K3$  and  $\alpha_r$ . The Sobol total effects are generally larger and out of the intervals of the VARS estimates.

The largest main effects  $S_i$  for the global mean absolute error correspond to  $K3$ ,  $Q_2$  and  $\alpha_r$  (see Tables S10 and S11 in SM). The sums of Sobol's 1<sup>st</sup> order indexes calculated with SALib and GUI-HDMR are equal to 0.682 and 0.734, respectively. The total effect of the global mean absolute error computed with SALib is equal to that of GUI-HDMR (see Tables S10 and S11 in SM). Second order effects for this output are important especially due to the interactions between  $Q_2$  and  $K3$  and between  $K3$  and  $K2$ . There are also interactions of smaller relevance between  $\alpha_r$  and  $K_{Vs1}$ , and between  $Q_2$  and  $K2$ . The SALib total effects  $S_{Ti}$  fall within the intervals of VARS estimates or are slightly larger than VARS total effects.

$K3$  shows the largest main effect  $S_i$  for the global normalized root mean squared error which is equal to 0.245 (SALib). The second largest effect corresponds to  $Q_2$  (see Tables S12 and S13 in SM). The sums of the Sobol's 1<sup>st</sup> order indexes calculated with SALib and GUI-HDMR are equal to 0.675 and 0.731, respectively. The total effects of the global normalized root mean squared error are close to 1 (see Tables S12 and S13 in SM). Second order effects for this output are relevant especially due to the interaction between  $K3$  and  $K2$ , and between  $Q_2$  and  $K3$ . The total effects  $S_{Ti}$  from SALib are either within the ranges of VARS estimates or slightly exceed the VARS total effects.

The largest main effect  $S_i$  for the global Nash–Sutcliffe index corresponds to  $K3$ .  $Q_2$  and  $K2$  are in second and third position (see Tables S14 and S15 in SM). The sums of the Sobol's 1<sup>st</sup> order indexes calculated with SALib and GUI-HDMR are equal to 0.411 and 0.477, respectively. The total effect of the global Nash–Sutcliffe index computed with SALib is smaller than that of GUI-HDMR (see Tables S14 and S15 in SM). Second order effects for this output are important especially due to the interaction between  $Q_2$  and  $K3$ . There are also interactions of smaller relevance between  $K3$  and  $K2$ , between  $Q_2$  and  $K2$ , and between  $\alpha_r$  and  $K_{Vs1}$ . The SALib total effects  $S_{Ti}$  fall within the intervals of VARS estimates or are slightly larger than VARS total effects.

The largest main effects  $S_i$  for the computed aquifer/reservoir flux at time  $t1$  correspond to  $K_{Vs1}$  and  $\alpha_r$  (see Tables S16 and S17 in SM). The sums of the Sobol's 1<sup>st</sup> order indexes calculated with SALib and GUI-HDMR are equal to 0.716 and 0.746, respectively. The total effects of the computed aquifer/reservoir flux at time  $t1$  are close to 1 (see Tables S16 and S17 in SM). The second order effects for this output are significant, particularly because of the interactions of  $K_{Vs1}$  with  $\alpha_r$ ,  $S_s$ ,  $Q_2$  and  $K3$ . Additionally, there are less significant interactions between  $\alpha_r$  and  $K3$ , and between  $\alpha_d$  and  $K4$ . The Sobol total effects  $S_{Ti}$  calculated by SALib are overall higher than the VARS estimates.

The largest main effect  $S_i$  for the computed aquifer/reservoir flux at time  $t2$  is equal to 0.362 (SALib) and corresponds to  $S_s$  (see Table S18 in SM). The sums of the Sobol's 1<sup>st</sup> order indexes calculated with SALib and GUI-HDMR are equal to 0.514 and 0.469, respectively. The total effects of the computed aquifer/reservoir flux at time  $t2$  are greater than 1 (see Tables S18 and S19 in SM). For this output, the second order effects are notable, mainly due to the interaction between  $S_s$  and  $K_{Vs1}$ . Sobol total effects  $S_{Ti}$  and those of VARS estimates show clear discrepancies, especially for  $K1$ ,  $K2$  and  $Q_7$ .

The largest main effect  $S_i$  of the computed aquifer/reservoir flux at time  $t_3$  is equal to 0.626 (SALib) and corresponds to  $K_{Vs1}$  (see Table S20 in SM). The sums of the Sobol's 1st order indexes calculated with SALib and GUI-HDMR are 0.812 and 0.819, respectively. The total effects of the computed aquifer/reservoir flux at time  $t_3$  are close to 1 (see Tables S20 and S21 in SM). Second order effects for this output are relevant to the interactions of  $K_{Vs1}$  with  $\alpha_r$ ,  $S_s$ ,  $Q_2$ ,  $K_{Vs3}$ ,  $K_4$  and  $K_3$ . The Sobol total effects  $S_{Ti}$  are generally larger and out of the intervals of the VARS estimates.

The largest main effects  $S_i$  for the average modulus of the Darcy velocity near well PS16C correspond to  $Q_2$  and  $K_3$  (see Tables S22 and S23 in SM). The sums of the Sobol's 1<sup>st</sup> order indexes calculated with SALib and GUI-HDMR are equal to 0.855 and 0.861, respectively. The total effects of the average modulus of Darcy velocity are slightly larger than 1 (see Tables S22 and S23 in SM). Second order effects for this output are relevant to the interaction between  $Q_2$  and  $K_3$ . There are also smaller interactions between  $Q_2$  and  $K_2$ , and between  $K_3$  and  $K_2$ . The Sobol total effects  $S_{Ti}$  are generally slightly larger and out of the intervals of the VARS total effects.

The 1<sup>st</sup> and 2<sup>nd</sup> order interactions among input parameters can also be presented visually through heatmaps. Figure S1 in SM shows the heatmaps of the 1<sup>st</sup> and 2<sup>nd</sup> order HDMR Sobol effects for all the outputs. The analysis of the heatmaps confirms that: (1) The largest components correspond to the main effects (which have been conveniently located along the diagonal of each of the maps); and (2) Off-diagonal terms corresponding to the 2<sup>nd</sup> order effects are important for all variables, especially the global Nash–Sutcliffe index and groundwater flow between aquifer and reservoir at time  $t_2$  due to the interactions between  $Q_2$  and  $K_3$  and between  $S_s$  and  $K_{Vs1}$ .

### 3.2.4. HDMR Results for the VARS Runs by Using the Halton Sequence

HDMR analyses can be performed reusing the simulations performed with the VARS-Halton sequence. However, preliminary analysis of the main effects,  $S_i$  and 2<sup>nd</sup> order effects,  $S_{ij}$ , calculated with SALib show that most outputs have Sobol total effects  $S_{Ti}$  that are consistently greater than 2, with some of them even reaching values of 3. Since the parameters are normalized, the sum of main and interaction effects of any order should equal the total variance, which should theoretically be equal to 1. It should be noticed that the VARS procedure to locate the star centers is either the Halton or the Sobol sequences, both of which generate low-discrepancy sequences [65]. They are intended to optimize quasi-Monte Carlo approaches by maximizing the efficiency to fill the parameter hypercube with sparsely located points. As the number of parameters increases, the capacity of the Halton sequence [50] to distribute points uniformly rapidly decreases [65], while Sobol sequence is more stable. This results in higher maximum errors of integration for high dimensional data related to the Halton sequence. The Halton sequence gives good quality, near uniform distributions when the number of parameters is lower than 10 [65], as it was demonstrated recently in a reactive transport model for a high-level radioactive repository in granitic rock [36].

### 3.3. Input Parameter Rankings

Table 2 shows the rankings of the parameters for the computed piezometric heads in 'well ST1C at time  $t_1$  (ST1C<sub>t1</sub>), global mean absolute error (MAEg), aquifer/reservoir flux at time  $t_1$  ( $Q_{t1}$ ), and average groundwater Darcy velocity modulus ( $q_{av}$ ) across the following methods: IVARS<sub>50</sub>, VARS-TO (equivalent to Sobol), VARS-ABE (equivalent to Morris), HDMR (SALib), GUI-HDMR and CUSUNORO plots.

**Table 2.** Ranking comparison of sensitivity results across various methods for the computed piezometric head in well ST1C at time  $t_1$  (ST1C<sub>t1</sub>), the global mean absolute error (MAEg), the aquifer/reservoir flux at time  $t_1$  ( $Q_{t1}$ ), and the average groundwater Darcy velocity modulus ( $q_{av}$ ).

Output	Methods	K1	K2	K3	K4	Ss	$K_{Vs1}$	$K_{Vs2}$	$K_{Vs3}$	$\alpha_r$	$\alpha_d$	$Q_6$	$Q_7$	$Q_9$	$Q_2$	$Q_1$	$r_c$	$r_u$
ST1C <sub>t1</sub>	VARS-TO	17	4	1	6	13	3	8	7	5	9	16	11	14	2	10	12	15
	IVARS <sub>50</sub>	16	4	1	6	12	3	8	7	5	9	17	11	14	2	10	13	15

Output	Methods	K1	K2	K3	K4	Ss	K <sub>Vs1</sub>	K <sub>Vs2</sub>	K <sub>Vs3</sub>	$\alpha_r$	$\alpha_d$	Q <sub>6</sub>	Q <sub>7</sub>	Q <sub>9</sub>	Q <sub>2</sub>	Q <sub>1</sub>	r <sub>c</sub>	r <sub>u</sub>
MAEg	VARS-ABE	17	5	1	6	9	4	11	10	3	8	16	12	14	2	7	13	15
	SALib	17	5	1	10	6	3	13	7	4	8	15	12	14	2	9	11	16
	GUI-HDMR	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	
	CUSUNORO	17	5	1	6	13	3	11	9	4	7	16	10	14	2	8	12	15
Q <sub>t1</sub>	VARS-TO	14	3	1	7	16	5	8	6	4	9	17	12	13	2	11	10	15
	IVARS <sub>50</sub>	14	4	1	7	16	5	8	6	3	9	17	12	13	2	11	10	15
	VARS-ABE	14	4	1	6	16	5	11	8	3	10	17	12	13	2	9	7	15
	SALib	14	5	1	6	17	4	12	7	3	10	16	11	13	2	9	8	15
	GUI-HDMR	14	4	1	7	17	5	11	6	3	8	15	12	13	2	10	9	16
	CUSUNORO	14	5	1	6	17	4	12	7	3	10	16	11	13	2	8	9	15
q <sub>av</sub>	VARS-TO	17	9	4	6	5	1	14	12	2	8	16	7	10	3	11	13	15
	IVARS <sub>50</sub>	17	9	4	6	5	1	14	11	2	7	16	8	10	3	12	13	15
	VARS-ABE	17	11	5	7	4	1	14	13	2	8	16	6	12	3	9	10	15
	SALib	17	9	13	10	6	1	15	12	2	5	16	4	11	3	7	8	14
	GUI-HDMR	17	9	5	8	4	1	14	13	2	7	16	6	11	3	10	12	15
	CUSUNORO	17	8	13	10	6	1	15	12	2	5	16	4	11	3	7	9	14

All the methods agree in identifying the most influential parameters for ST1C<sub>t1</sub>, although they show different positions for some input parameters. The most influential parameters for ST1C<sub>t1</sub> are K3 and Q<sub>2</sub>. Q<sub>2</sub> is the most influential parameter, followed by K3.

Output MAEg shows similar results with the following order of ranking: (1) K3; (2) Q<sub>2</sub>; (3)  $\alpha_r$  (except for VARS-TO); (4) K2 (except for VARS-TO and SALib); and (5) K<sub>Vs1</sub> (except for SALib). GSA methods also agree on the ranking of the least relevant input parameters but switching in some positions.

The three most influential parameters for Q<sub>t1</sub> are K<sub>Vs1</sub>,  $\alpha_r$  and Q<sub>2</sub>. There are discrepancies among the methods on the ranking of the 4<sup>th</sup> and 5<sup>th</sup> positions (K3 and S<sub>s</sub>). SALib reveals the highest discrepancy for Q<sub>t1</sub> compared to the other methods for the parameters ranging from inputs relevance from 4<sup>th</sup> to 10<sup>th</sup> rank.

All methods provide a similar ranking of the most influential parameters for q<sub>av</sub>. Q<sub>2</sub> is the most influential parameter followed by K3. The third most influential is K2 and K1 is the fourth.  $\alpha_r$  is at the 5<sup>th</sup> position and K<sub>Vs1</sub> is the 6<sup>th</sup> most influential parameter, except for the VARS-TO method which interchanges the positions of these two parameters.

Table S24 to Table S29 present the rankings of the parameters for all the outputs. For the most part, all methods agree in identifying the most and the least relevant inputs for all outputs. Some methods switch the order within the most and within the least influential inputs. On the other hand, the inputs located at intermediate positions (from 5<sup>th</sup> to 13<sup>th</sup> place) show less consistency across the different GSA methods. Their ranking does not usually change more than three places, but some outputs show important differences (K4 for Q<sub>t3</sub> ranges from 7<sup>th</sup> to 15<sup>th</sup> position; K<sub>Vs2</sub> for SPN1<sub>t3</sub> ranges from 7<sup>th</sup> to 13<sup>th</sup> position).

The rankings of the input parameters derived from CUSUNORO curves for computed heads in wells ST1C, PS19B and SPN1 at times t1, t2 and t3 agree with the rankings of VARS and HDMR methods. Unlike the Morris method, CUSUNORO plots are well suited to identify the most influential input parameters.

#### 4. Conclusions

VARS and HDMR GSA of the groundwater flow model of the Gállego alluvial aquifer has been presented. Computed piezometric heads in monitoring wells and aquifer/reservoir fluxes change with time due to the tidal effect caused by the daily oscillations of the water level in the Sabiñánigo reservoir. Therefore, the sensitivities of the heads and fluxes change with time. The results of the GSA lead to following conclusions:

- The most influential parameters for the selected outputs are consistently detected by all methods. They include: K2, K3,  $K_{Vs1}$ , Ss,  $Q_2$  and  $\alpha_r$ .
- While some parameter inputs such as K3 and  $Q_2$  are relevant for all the outputs, other parameter inputs such as K1 and Ss are influential only for some outputs
- The sensitivity indexes of the computed heads in monitoring wells and aquifer/reservoir fluxes with respect to Ss change with time
- Sensitivity indexes of the calibration metrics are similar. MAEg is less prone to model result outliers.
- The average groundwater Darcy velocity near well PS16C depends mainly on the boundary inflow  $Q_2$ .
- VARS achieves stable values for the most important and the least influential input parameters after 50 star centers, which amounts to 7700 runs. For other inputs, the robustness of the ranking does not increase monotonically with the number of star centers.
- VARS and HDMR methods provide similar results in terms of rankings and significance of the most influential parameters. However, they show slight differences in the ranking of parameters of intermediate and low influence. The ranking of the least relevant variables with the different methods is less consistent.
- Graphical methods and HDMR results highlight that the most important input parameter interactions occur between Ss and  $K_{Vs1}$  for groundwater flow between aquifer/reservoir groundwater flux when the water level of the reservoir is high at time t2.

Future work should be devoted to extending GSA to other model outputs such as aquifer/river fluxes, discharges underneath the dam foundation or aquifer/reservoir fluxes in other parts of the reservoir. In addition, an extension to more time-dependent outputs is needed to capture the tidal effect caused by the reservoir on computed heads and aquifer discharges. The influence of extreme results on some calibration metrics could be overcome by using calibration metrics immune to the outliers, such as the median absolute deviation. Furthermore, ranges in some inputs could be revised as more data becomes available, and extreme results are disregarded. On the other hand, only a limited number of parameters are relevant for each output variance, and some inputs' contribution to the variance of the results is negligible. It might be advisable to reduce the number of parameters and analyze further the results of the HDMR analyses for the Halton sequence.

Parameter ranking is useful to identify the most and the least influential input parameters. However, parameter ranking only provides information on the ordering, not on the values of the sensitivity indexes. The results presented in the preceding sections suggest that most outputs are mostly sensitive to 5 input parameters. The impact on model outputs of the uncertainty of the least relevant parameters is almost irrelevant.

Sensitivity indexes of the heads and aquifer/reservoir fluxes are time dependent due to the tidal effect of the Sabiñánigo reservoir. It might be advisable to include more time-dependent outputs to capture the effect of the oscillations of the water level of the reservoir. The influence of extreme results on NRMSEg and NSEg could be corrected by using a median absolute deviation which is immune to outliers. Furthermore, ranges in some input parameters could be revised and updated as more data become available. There are no reliable data on the boundary inflow,  $Q_2$ , from Sardas landfill. Leachate estimations based on groundwater flow models range from 17 to 32 m<sup>3</sup>/d. This range is

much smaller than the range used in the GSA presented here. Moreover, recent monitoring wells closer to the Sabiñánigo reservoir reveal that the hydraulic conductivity of the aquitard (silting sediments and alluvial silts) is very heterogeneous.

GSA methods presented here provide a quantitative tool to assess the impact of the uncertainty of parameters on the groundwater flow model outputs in alluvial aquifers. The findings of this study can guide future management and data acquisition in polluted sites to reduce uncertainties related to the most relevant parameters. Moreover, these methods can guide future uncertainty analysis of the total dissolved hexachlorocyclohexane transport model through the Gállego alluvial aquifer presented in Sobral *et al.* [38].

**Supplementary Materials:** The following supporting information can be downloaded at the website of this paper posted on Preprints.org, Figure S1: Heatmaps of the HDMR Sobol 2<sup>nd</sup> order effects for: (1) Hydraulic heads in wells PS19B, SPN1 and ST1C at time intervals t1, t2 and t3; (2) MAEg; (3) NRMSEg; (4) NSEg; (5)  $q_{av}$ ; (6)  $Q_{t1}$ ; (7)  $Q_{t2}$  and (8)  $Q_{t3}$ . The map visualizes both main and second order effects; Tables S1 to S23: 1<sup>st</sup> order (main effects),  $S_i$ , and 2<sup>nd</sup> order effects,  $S_{ij}$ , calculated with SALib and gui-HDMR of each output for the Sobol sequence. The largest 2<sup>nd</sup> order effects  $S_{ij}$ , are listed in the off-diagonal boxes. Parameter rankings are indicated within brackets. Total effects ( $S_i + S_{ij}$ ) are compared to the VARS interval total effects for each parameter; Tables S24 to S29: Ranking of the influence of parameters for each output.

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