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Article

Coastal Erosion Caused by River Mouth Migration on Cuspate Delta: An Example from Thanh Hoa, Vietnam

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Abstract: Coastal erosion is threatening the infrastructures of coastal community at the Ma River mouth in Thanh Hoa province, Vietnam. When coastal erosion happens, people try to prevent it by emergency solutions like hard structures. However, this approach usually leads to more severe problem since the mechanism for coastal erosion is not investigated and understood properly. In this study the long-term configuration of Ma River mouth in Thanh Hoa Province, Central Vietnam has been investigated using Landsat imagery from 1987 to 2023. An analytical solution of one-line model for shoreline change was also employed to calculate the sand discharge from Ma River as well as the diffusion coefficient for sand transported along-shore by breaking waves. There was an asymmetric configuration of the Ma River mouth during the last 37 years. The sand supply from the Ma River is around 350,000 m³/year. This amount of sand is going mainly to the northern beach of the Ma River delta (from 55% to 75%). The asymmetric morphology of the Ma River mouth delta is the trigger for severe coastal erosion reported recently. This study provides a quick and easy method to investigate the morphological change of cuspate deltas where measured data is limited.

Keywords: coastal erosion; cuspate delta; wave-dominated delta; analytical solution; sediment supply; Landsat; Ma River; Thanh Hoa

1. Introduction

Cuspate delta, which was first defined by Wright and Coleman [1] as an example of a slightly protruding delta into open water, has been the object of numerous studies by coastal scholars [2–18] because they are one of the three most popular delta shapes in the world classified by Galloway [3]. According to Galloway [3], this type of landform was designated as a wave-dominated delta because the morphologies of these deltas were formed mainly by the interaction between riverine (sediment supply) and ocean forces (waves).

Because of urbanization and the need for natural resources, 31% of Vietnam's population is living on the coast [19]. However, this narrow belt is susceptible to the threads of waves, flooding, storm surge, and sea level rise [20]. Along the coastline of Vietnam, coastal erosion has become a natural hazard in the last decade and come with significant cost to communities and nature [21–24]. In the Ma River mouth delta, a similar disaster is happening on the cuspate of the delta. Coastal erosion has negatively affected infrastructures and livelihoods in recent years. The cuspate delta has experienced a significant loss of agricultural land and infrastructure to the sea. In this study, the long-term evolution of Ma River mouth in Thanh Hoa province, Vietnam will be investigated to find out the relationship between coastal morphological change and coastal erosion in this cuspate delta.

Ma River delta is a cusplate delta located in Thanh Hoa province, Vietnam as shown in **Figure 1**. The protruding part of this delta is formed by sediment supplied from Ma River and under effects of waves from the East Sea of Vietnam [12]. According to Quang et al. [25], Ma River delta is the third largest river delta in Vietnam which drains to the East Sea 5.17 million tons of sediment every year. Under action of the waves, sediment supplied from the river is transported along-shore to nourish the beaches on both sides of the Ma River mouth. It should be noticed that at the end of the southern coast, there is a mountainous region that acts as a boundary to block the longshore sediment transport (**Figure 1c,d**).

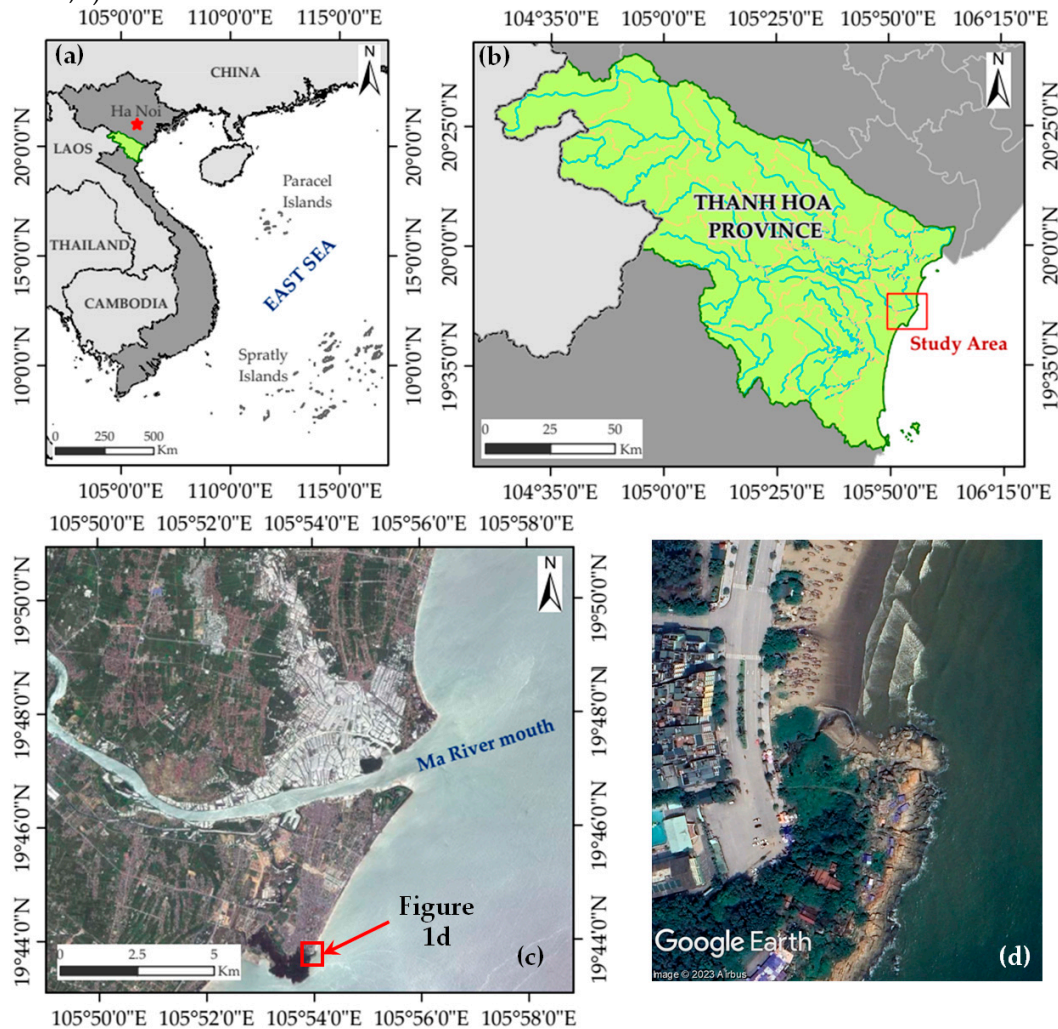


Figure 1. Study area. (a) Vietnam map, (b) Thanh Hoa province, and (c) Ma River delta, and (d) Mountainous region which is considered as a no sand transport boundary.

2. Materials and Methods

2.1. Landsat Image Analysis

Landsat imagery has been applied intensively for coastal studies especially in areas with significant temporal variation of the shorelines such as river mouths, sand spits, and around coastal structures [26–31]. In this study, Landsat images from 1987 to 2023 were collected (**Table 1**) for mapping the changes of the shorelines around the Ma River mouth. Shoreline detection was performed based on the Normalized Difference Water Index (NDWI) [32]:

$$NDWI = \frac{X_{green} - X_{nir}}{X_{green} + X_{nir}} \quad (1)$$

where X_{green} and X_{nir} are the GREEN and NIR wavelengths, respectively. Subsequently, the shorelines of several years from 1987 to 2023 are mapped together to observe the temporal variation of the delta's cusplate.

Table 1. Information of Landsat images.

No.	Date (yyyy/mm/dd)	Sensor	Resolution (m)	Data Source
1	1987/03/07	TM	30	Landsat 5
2	1988/05/28	TM	30	Landsat 5
3	1989/06/16	TM	30	Landsat 5
4	1990/07/05	TM	30	Landsat 5
5	1991/02/14	TM	30	Landsat 5
6	1992/06/24	TM	30	Landsat 5
7	1993/07/29	TM	30	Landsat 5
8	1994/05/29	TM	30	Landsat 5
9	1995/09/05	TM	30	Landsat 5
10	1996/08/06	TM	30	Landsat 5
11	1997/06/06	TM	30	Landsat 5
12	1998/10/15	TM	30	Landsat 5
13	1999/10/18	TM	30	Landsat 5
14	2000/11/05	TM	30	Landsat 5
15	2001/07/19	TM	30	Landsat 5
16	2002/09/24	TM	30	Landsat 5
17	2003/05/06	TM	30	Landsat 5
18	2004/05/24	TM	30	Landsat 5
19	2005/05/11	TM	30	Landsat 5
20	2006/11/06	TM	30	Landsat 5
21	2007/05/01	TM	30	Landsat 5
22	2008/06/20	TM	30	Landsat 5
23	2009/07/09	TM	30	Landsat 5
24	2010/11/01	TM	30	Landsat 5
25	2011/04/26	TM	30	Landsat 5
26	2013/10/08	OLI/TIRS	30	Landsat 8
27	2014/07/23	OLI/TIRS	30	Landsat 8
28	2015/01/15	OLI/TIRS	30	Landsat 8
29	2016/10/07	OLI/TIRS	30	Landsat 8
30	2017/07/31	OLI/TIRS	30	Landsat 8
31	2018/07/02	OLI/TIRS	30	Landsat 8
32	2019/05/18	OLI/TIRS	30	Landsat 8
33	2020/05/20	OLI/TIRS	30	Landsat 8
34	2021/05/23	OLI/TIRS	30	Landsat 8
35	2022/04/08	OLI/TIRS	30	Landsat 8
36	2023/05/21	OLI/TIRS	30	Landsat 9

2.2. Application of Analytical Solutions for Shoreline Change

In this section, an analytical solution of one-line model for shoreline changes in a wave-dominated delta will be utilized to quickly estimate the diffusion coefficient for longshore sand transport (ε). Currently, there are two types of analytical solutions for modelling wave-dominated river delta evolution. The first solution was introduced by Larson et al. [33] for infinite shorelines (**Figure 2a**). Later, Duy et al. [18], derived another closed-form solution for shorelines with definite lengths (L) (**Figure 2b**).

As can be seen in **Figure 1**, there is a headland at the end of the southern shoreline in Ma River delta. This headland can be considered as a rigid boundary as shown in **Figure 2b**. Hence, the southern shoreline of Ma River delta well fits the analytical solution proposed by Duy et al. [18] as follows:

where y is the shoreline position, q_0 is the sand discharge from the river, ε is the diffusion coefficient, D is the sum of depth of closure (D_C) and beach berm height (D_B), x is the along-shore distance, and L is the length of the shoreline.

In order to make the comparison between the shoreline extracted from Landsat image and the shoreline calculated using the analytical solution, a Cartesian Coordinate system was used consistently in this study. In which, the abscissa is 198° clockwise from the north and crossing the point O (595,288.00 m E; 2,188,386.00 m N) in the UTM system (**Figure 3b**). In the interest of expediency, a 108° counterclockwise rotation is applied to the coordinate system to establish the configuration depicted in **Figure 3a**.

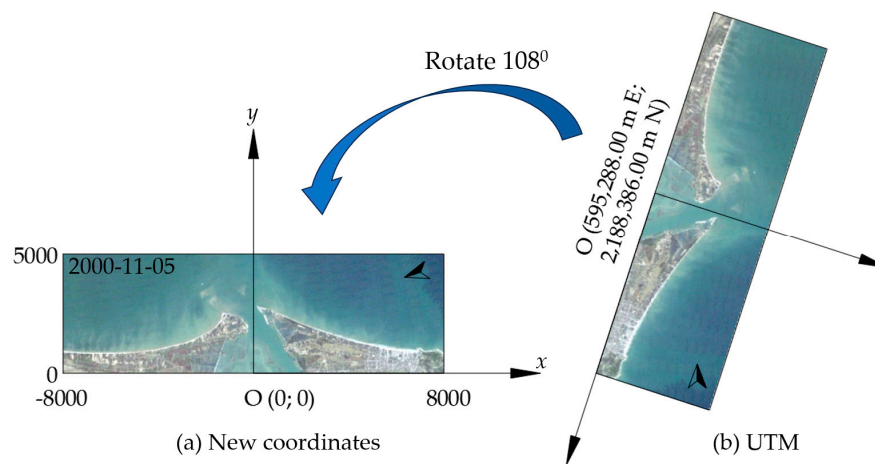


Figure 3. The Cartesian Coordinates used in this study by rotating the images 108° and taking the Origin O (0;0) in the Cartesian Coordinates as the O (595,288.00 m E; 2,188,386.00 m N) in the UTM system.

The shoreline position extracted from the Landsat image on 1991/02/14 will be used as the measured data to compare with the computed data. The shoreline position on 1991/02/14 was chosen since it has the most protruding part of the cusplate and a symmetric configuration concerning the y -axis. These conditions satisfy the shoreline configuration in **Figure 2b**.

To apply Equation (2) for modeling the shoreline position in 1991, all variables are collected and presented in **Table 2**.

Table 2. Initial values of variables used for modeling the evolution of the Ma River delta using the analytical solution of Duy et al [18].

Diffusion coefficient, (m^2/day)	unknown
Sediment supply from the river, q_0 (m^3/y)	390,000
Formation time, t (yr)	500
Depth of closure, D_c (m)	5.6
Berm height, D_B (m)	1.8
Length of the shoreline, L (m)	6,900

Quang et al. [25], reported that the Ma River discharges a total of 5.17 tons of sediment annually into the Vietnamese East Sea. Among this amount, 20% constitutes bed load sediment [34]. Consequently, the annual contribution of bed load sediment from the Ma River to the river mouth beaches is estimated to be 1.03 tons. To determine the sand transport rate, considering a sand density of $2,065 \text{ kg/m}^3$ [35], the calculated value is $390,000 \text{ m}^3/\text{year}$. This value is utilized as the initial value for sand supply from the Ma River, as presented in **Table 2**.

The determination of the formation time (t) draws upon the research conducted by Yen et al. [36]. It is worth noting that while Yen et al.'s [36] investigation focuses on the Red River delta, it also includes a map encompassing the neighboring Ma River delta. This map provides distinct evidence that the historical shoreline positions in the Late Holocene originated between 500 to 200 years Before Present (BP). Consequently, the establishment time of the Ma River delta is considered to be 500 years.

The depth of closure is calculated based on the following equation of Hallermeier [37] as follows:

$$D_c = 2.28H_{se} - 68.5 \left(\frac{H_{se}^2}{gT_{se}^2} \right) \quad (3)$$

where D_c is the depth of closure, H_{se} and T_{se} are the significant wave height and wave period of the extreme wave condition (extremely high waves expected 12 hours per year), and g is the gravitational acceleration. According to Thomson & Harris [38], T_{se} should be taken to be the typical period of the measured wave height. The extreme high wave conditions were taken from the study

of Hung et al. [39] with $H_{se} = 3$ m and $T_{se} = 7$ s, respectively. Substituting H_{se} and T_{se} to Equation (3) yields $D_c = 5.6$ m.

The beach berm height is calculated based on the expression proposed by Uda [40] as follows:

$$D_B = 0.32 \times D_C \quad (4)$$

The value of $D_c = 5.6$ m yields $D_B = 1.8$ m.

Finally, the length of the shoreline is measured from the Landsat image captured on 1991/02/14 as shown in **Figure 4**.

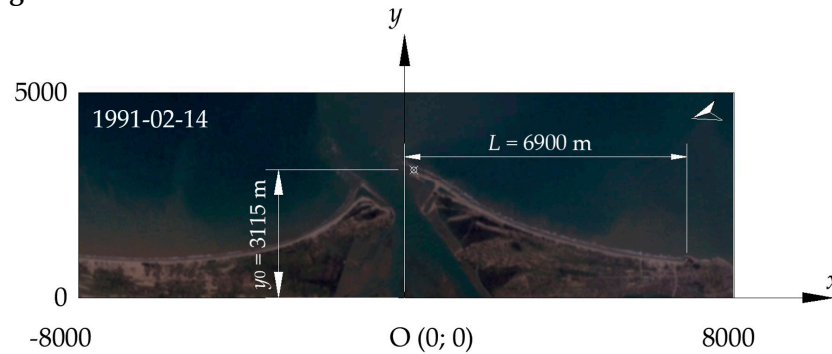


Figure 4. Shoreline configuration on 1991/02/14 is used to compare with the computed shoreline using the analytical solution.

Table 2 contains the initial values employed for computing the shoreline position in 1991. A comparison between these calculated positions and the shoreline positions extracted from the Landsat image on 1991/02/14 will be performed. Given that the value of e is unknown, an iterative calculation process is adopted, considering a range of e values. The root mean square error (RMSE) of the shoreline positions is utilized to determine the optimal value of e that best fits the data. Consequently, the iterative process terminates once the minimum RMSE is achieved.

2.3. Geometrical Characteristics of the Cuspate Delta

2.3.1. Evolution of the Cuspate Delta

In this section, the along-shore and cross-shore coordinates of the cuspate tips on both sides of Ma River mouth will be measured and used for discussing the morphological behavior of the river mouth. The method of using the coordinates of special points such as sand spit's tips or the central of gravity to discuss the evolution of a sand body has been applied widely in coastal engineering [26,28,29,31,41,42]. **Figure 5** illustrates the schematic for measuring the coordinates of the cuspate tips, which are considered as the most prominent protrusions into the open water. The schematic elucidates the fundamental variables involved in the process, namely x_s and x_n , signifying the along-shore coordinates of the cuspates' tips on the southern and northern sectors, respectively. Additionally, the variables y_s and y_n are designated to represent the cross-shore extents of the cuspate tips on the southern and northern parts, respectively. It is imperative to note that all measurements of these variables are expressed in the metric unit of meters.

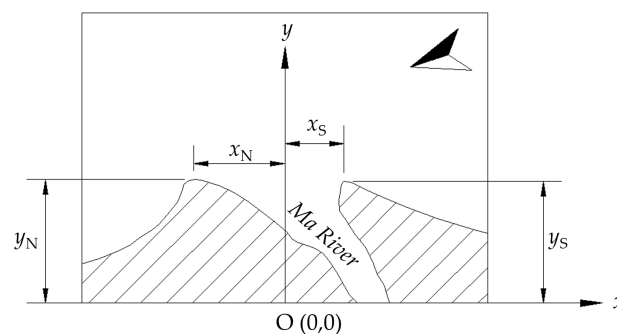


Figure 5. The sketch for measuring the tips' coordinates on the north (x_n, y_n) and south (x_s, y_s) of the Ma River mouth.

2.3.2. Shoreline Orientations at the River Mouth

Moreover, the investigation of the Ma River mouth encompasses not only the geometrical characteristics of the delta cuspate but also the determination of shoreline orientations. In the context of wave-dominated river deltas, the assessment of shoreline orientations on both sides of the river mouth assumes paramount significance for understanding the evolution of this geographical feature [2,23,43,44]. To facilitate the estimation of shoreline orientations, it is imperative to define shoreline angles. As elucidated in [43], shoreline angles correspond to the angles formed between the trendlines (red lines) and a horizontal reference line, as illustrated in **Figure 6**. Denoting the shoreline angles on the north and south of the Ma River mouth as θ_N and θ_S , respectively, the expressions for the shoreline orientations at the river mouth can be represented as follows:

$$\beta_N = \tan(\theta_N) \quad (5)$$

and

$$\beta_S = \tan(\theta_S) \quad (6)$$

where β_N and β_S are the shoreline orientations on the north and the south of the river mouth, respectively.

In **Figure 6**, we examine the angles of the delta cuspate formed on the northern (θ_N) and southern (θ_S) sections. To ascertain shoreline orientations, we employ the linear regression method, utilizing shoreline positions depicted as blue dots, extracted at regular 50 m intervals. For visual clarity, the figure displays shoreline positions at 300 m intervals. The measurement range for shoreline orientations encompasses $-3000 \text{ m} \leq x \leq -1200 \text{ m}$ for the northern shoreline and $500 \text{ m} \leq x \leq 2500 \text{ m}$ for the southern shoreline

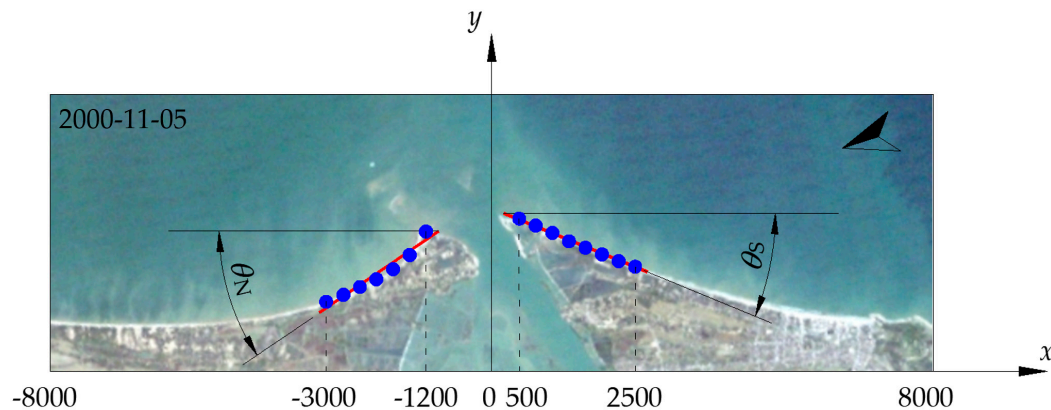


Figure 6. Angles of the delta cuspate on the north (q_N) and the south (q_S).

According to Hu et al. [45], on wave-dominated river deltas, the coastline shapes affect the distribution of sand supply from the river. This mechanism was also studied by Ashton et al. [46] and Ashton and Giosan [47]. Therefore, it would be useful to investigate the relationship between the shoreline orientations and the distribution rate of sand supply from the Ma River. If the distribution rate of sand supply from the Ma River to the north is α , it can be expressed as a function of the shoreline orientations as:

$$\alpha = \frac{\beta_N}{\beta_N + \beta_S} \quad (7)$$

3. Results

3.1. Temporal Variation of Shorelines at the River Mouth

Shoreline positions extracted from Landsat imagery are plotted in **Figure 7**. Only shoreline positions in 1987, 2000, 2010 and 2023 are plotted to ensure legibility. As can be seen from the figure, the southern tip of the cuspate has retreated significantly by a distance of approximately 1000 m from 1987 to 2023. On the other hand, the shoreline position of the northern tip has moved to the north.

Moreover, a significant northward displacement of the shoreline at the erosion hot spot (indicated by the red rectangle in **Figure 7a**) has also been observed, measuring a distance of

approximately 200 m. These shifting shoreline patterns are attributed to severe coastal erosion, which has prompted local communities to implement the use of hard structures as a protective measure, as illustrated in **Figure 7b**.

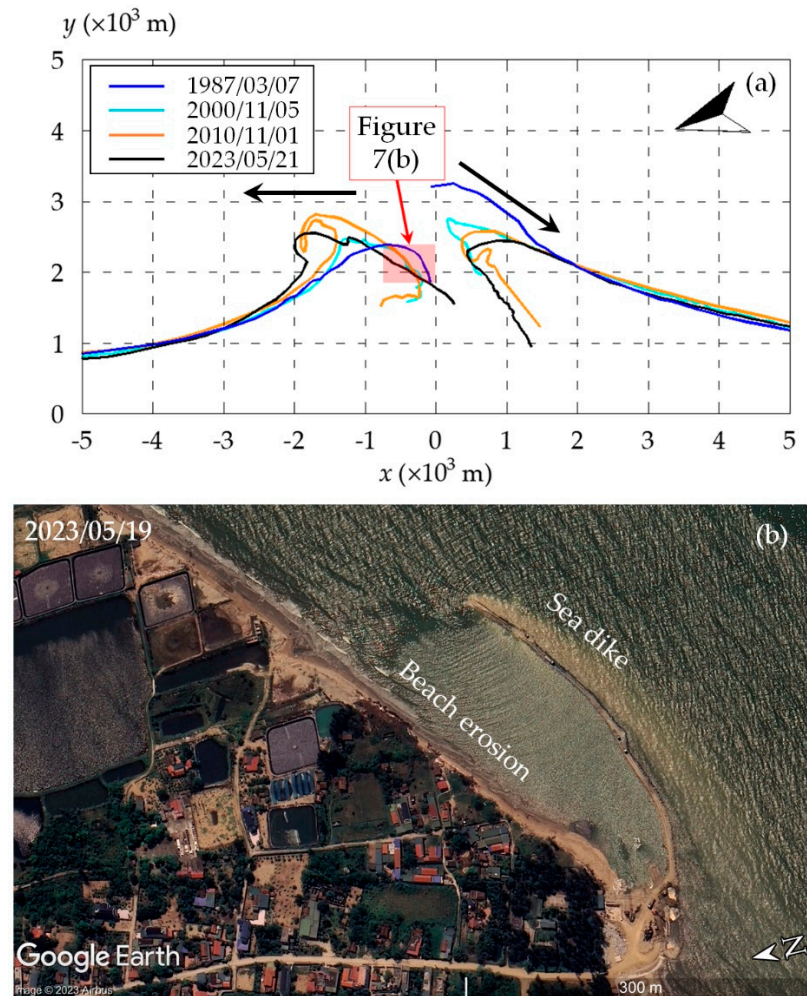


Figure 7. (a) Temporal variation of shorelines at the Ma River mouth, (b) Beach erosion due to northward shifting of the river mouth and a sea dike was built by the local government to reduce wave forces prior to the construction of an embankment to prevent coastal erosion (image downloaded from Google™ earth).

3.2. Diffusion Coefficient for Sand Transport Induced by Breaking Waves

Figure 8 displays the optimal alignment between the computed shoreline and the observed shoreline. In this context, the shoreline position in 1991, determined through Equation (2), is depicted by the red line, while the shoreline position obtained from the Landsat image taken on 1991/02/14 is represented by the blue dots. The minimum Root Mean Square Error (RMSE) between these shoreline positions amounts to 26.4 m. For reference, **Table 3** presents the final values of the variables employed in the calculation procedure.

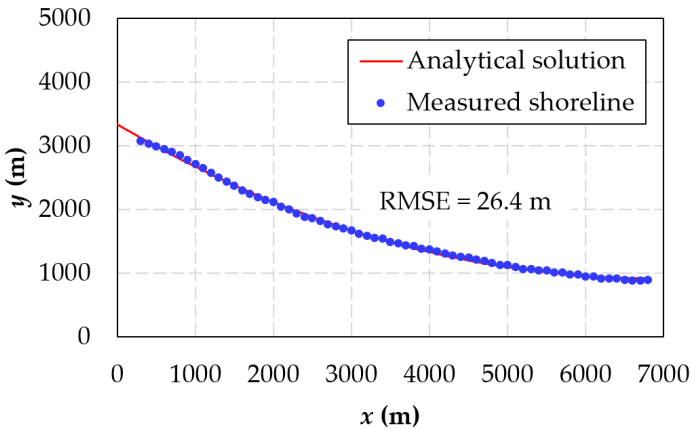


Figure 8. Comparison between computed shoreline and measured shoreline in 1991.

Table 3. Final values of variables used for modeling the evolution of the Ma River delta using the analytical solution of Duy et al. [18].

Diffusion coefficient, (m ² /day)	90
Sediment supply from the river, q_0 (m ³ /y)	350,000
Formation time, t (yr)	500
Depth of closure, D_c (m)	5.6
Berm height, D_B (m)	1.8
Length of the shoreline, L (m)	6,900

3.3. Geometrical Characteristics of the Cuspate Delta

3.3.1. Evolution of the Delta Cuspate

Figure 9 shows the long-term variation of the northern and southern tips' coordinates of the sand cuspsates over time in the along-shore direction. The horizontal axis represents the timeline, ranging from 1987 to the present year (2023). The vertical axis represents the along-shore coordinates of the cuspsates' tips, measured in meters. The blue circles on the graph represent the along-shore coordinate for the southern cuspate (x_s), while the orange squares represent the along-shore coordinate for the northern cuspate (x_n).

From the data presented in **Figure 9**, it is evident that both cuspsates have been moving in opposite directions along the shoreline over time. The southern cuspate (x_s) has shown an increasing trend, moving at a rate of 17.59 meters per year to the south. On the other hand, the northern cuspate (x_n) has been decreasing in its along-shore coordinate, moving at a rate of approximately -21.28 meters per year to the north. These observations are in line with the changes observed in the shoreline of the Ma River mouth, as depicted in **Figure 7a**.

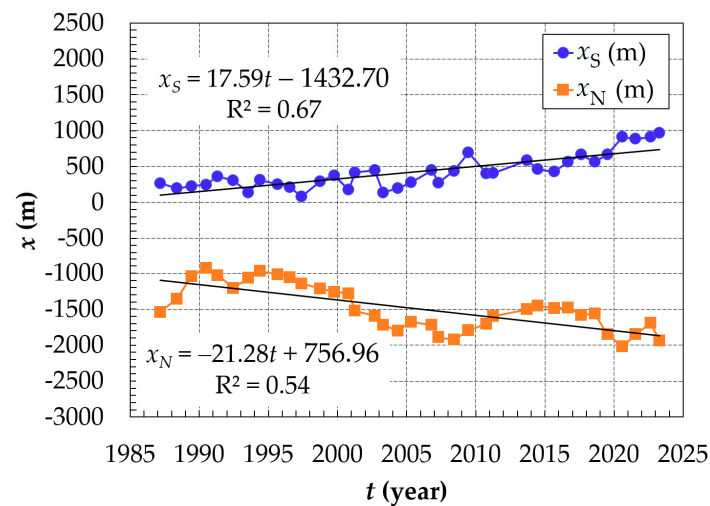


Figure 9. Temporal variation of the northern and southern cuspaters' tips coordinates in the along-shore direction.

Figure 10 presents a temporal variation diagram of the cuspaters' tips in the cross-shore direction from 1987 to the present. The diagram includes two sets of data points represented by different shapes and colors. The orange triangles represent the coordinates of the southern cuspatate (y_S), while the blue diamonds represent the northern cuspatate (y_N).

Concerning the southern cuspatate, the cross-shore coordinate of the southern cuspatate decreased steadily over the tracking time. This means that the position of the southern cuspatate moved closer to the shoreline as time progressed.

On the other hand, the cross-shore coordinates of the northern cuspatate showed fluctuations during the survey period. From 1987 to 2000, the northern cuspatate's cross-shore coordinate decreased. This indicates that the position of the northern cuspatate moved closer to the shoreline during this time period. Between 2001 and 2005, there was a sharp increase in the northern cuspatate's cross-shore coordinate. It expanded from 2000 meters to 3600 meters, indicating that the northern cuspatate moved further away from the shoreline during this period. Since 2006, the northern cuspatate's cross-shore coordinate decreased significantly and reached 2500 meters again at the present time. This suggests that the position of the northern cuspatate moved closer to the shoreline again in recent years.

In addition, the significant increase in the northern cuspatate's cross-shore coordinates from 2001 to 2005 indicates a large sediment supply from the Ma River, but the protrusion towards the sea of only the northern cuspatate implies an asymmetric distribution of this sand supply, favoring the northern cuspatate.

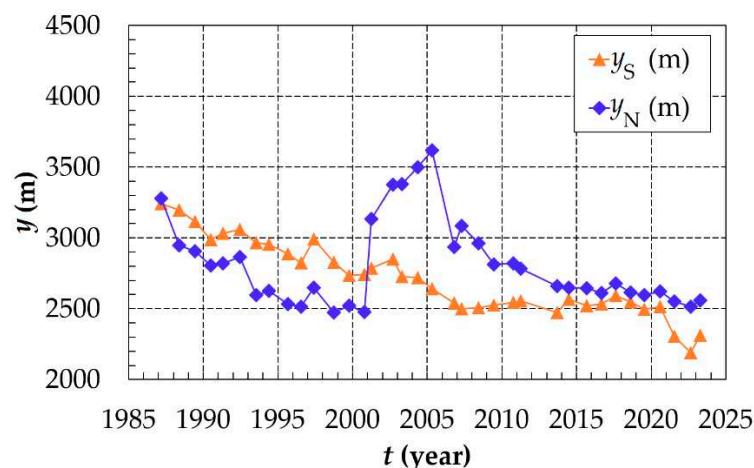


Figure 10. Temporal variation of the northern and southern cuspaters' tips coordinates in the cross-shore direction.

3.3.2. Shoreline Orientations at the River Mouth

Figure 11 illustrates the temporal variation of the shoreline orientations at the Ma River mouth. The diagram displays two key values: β_N , representing the shoreline orientation of the north, and β_S , representing the shoreline orientation of the south.

The diagram indicates that there is a significant gap between the shoreline orientations of the north and the south. This disparity suggests that the cusplate in the Ma River delta possesses an asymmetric shape. In other words, the two sides of the river mouth, the northern and southern tips, have different angles at which they meet the water.

Furthermore, the higher values of β_N compared to the values of β_S indicate that the northern tip of the river mouth protrudes more prominently into the water than the southern one. This difference in protrusion further contributes to the overall asymmetry of the cusplate delta.

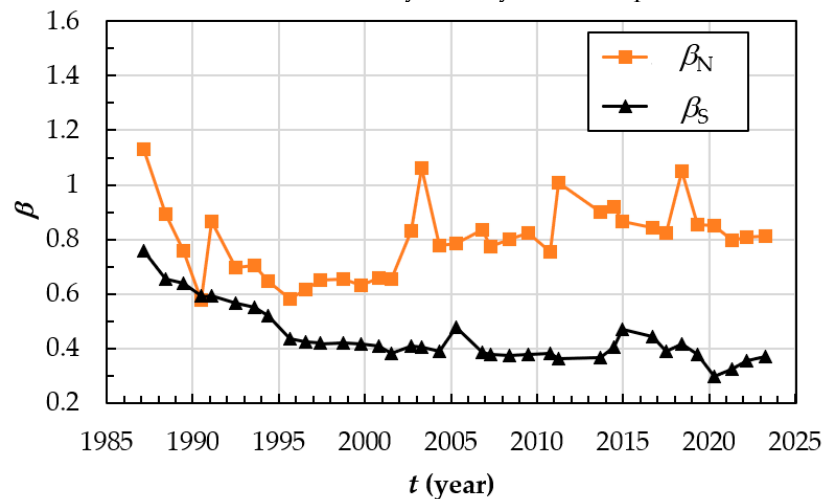


Figure 11. Shoreline orientations on both sides of the river mouth.

Figure 12 illustrates the calculated ratio of sand supply to the northern lobe of the Ma River delta from 1987 to 2023. The ratio is determined using Equation (7), and the results indicate that the amount of sand supplied to the northern lobe has consistently been larger than that supplied to the southern lobe during this period. The trend over time shows a continuous increase in the sand supply to the northern lobe, indicating a greater deposition of sand in that region compared to the south. This is evidenced by the entire data points consistently lying above the midpoint ($a = 0.5$), suggesting an asymmetric distribution of sand towards the north (from 55% to 75%). However, there was an exception in the year 1990 when the sand supply was equal to both sides of the Ma River mouth, resulting in a balanced ratio with a value of a equal to 0.5. This suggests that in 1990, the sediment distribution between the northern and southern lobes was approximately equal.

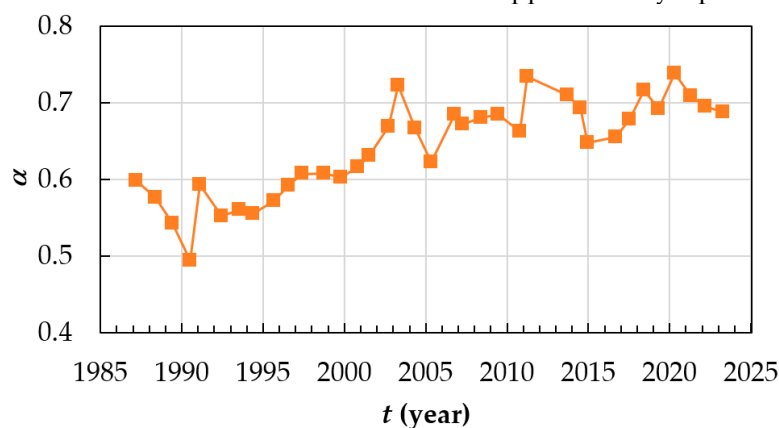


Figure 12. Ratio of sand supply to the north.

4. Discussion

Figure 13 shows a representation of a river mouth where sand is being supplied symmetrically to both sides of the river's opening. Additionally, the sketch includes illustrations of longshore sand

transport along the coastlines. Arrows are drawn parallel to the shorelines, indicating the direction of sand movement along the coast.

According to the study of Duy et al. [18], longshore sand transport can be mathematically expressed as:

$$Q = -\varepsilon D \frac{\partial y}{\partial x} \quad (8)$$

where Q is the longshore sand transport along the coastline, $\frac{\partial y}{\partial x}$ is the shoreline's orientations at the river mouth, $\frac{\partial y}{\partial x} = \tan \theta = \beta$.

By equating Q and $q_0/2$, it yields:

$$\frac{\partial y}{\partial x} = -\frac{q_0}{2\varepsilon D} \quad (9)$$

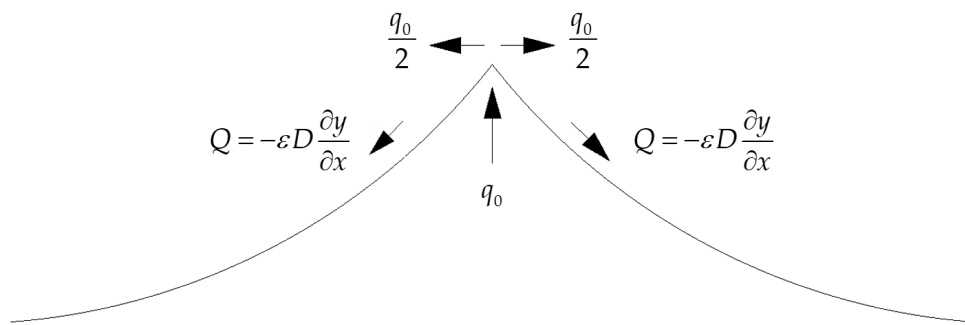


Figure 13. Symmetric sand distributed at the rivermouth and along-shore sand transport rate.

For the asymmetric condition at Ma River delta, the sand supply from the Ma River and the ratios of sand transported to the north and south at the Ma River mouth can be calculated as follows:

- Sand supply from the Ma River:

$$q_0 = (\beta_N + \beta_S) \varepsilon D \quad (10)$$

- Sand transported to the north:

$$q_N = \alpha q_0 \quad (11)$$

- Sand transported to the south:

$$q_S = (1 - \alpha) q_0 \quad (12)$$

Figure 14 shows the sand supply from Ma River (orange squares) and the amounts of sand distributed to the north (blue diamonds) and the south (black dots), respectively. There is an asymmetric of sand distributed to the north and the south of the delta which indicates the waves approach the delta from a dominant direction more often at this study site. It is interesting from **Figure 14** that the average amount of sand supply from the Ma River is around 380,000 m³/year. This value is very close to the sand supply estimated using the analytical solution shown in **Table 3** (350,000 m³/year).

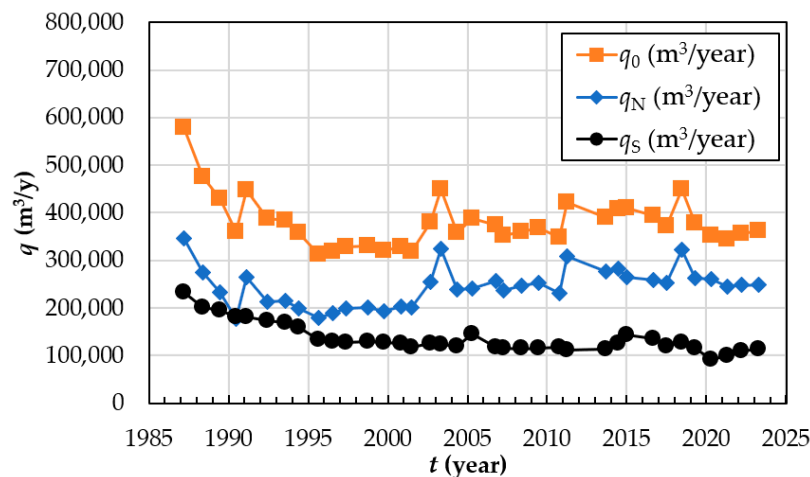


Figure 14. Ratio of sand supply to the north.

5. Conclusions

Coastal erosion at the Ma River mouth in Thanh Hoa province, Vietnam has been investigated using Landsat imagery from 1987 to 2023 and an analytical solution. The research findings reveal the following key points:

- The northern part of the Ma River delta is experiencing northward movement, leading to severe coastal erosion at the river mouth.
- In contrast, the southern part of the Ma River Delta is moving southward and landward.
- The sand diffusion coefficient at the Ma River delta has been calculated to be 90 m²/day, indicating the rate at which sand particles are transported.
- The present investigation provides an evaluation of the sand supply originating from the Ma River to the delta's lobes. According to the analytical solution, the estimated annual sand supply amounts to approximately 350,000 m³/year. Additionally, an alternative approach based on the shoreline orientations at the river mouth yields a slightly higher value, specifically 380,000 m³/year. This discrepancy underscores the reliability of the sand supply assessment derived through the current study's methodologies.
- The research also highlights an asymmetric configuration of shoreline shapes at the Ma River mouth, as evidenced by the uneven distribution of sediment to the delta flanks. In which, the ratio of sand supply to the north ranges from 55% to 75%.

These findings provide valuable insights into the erosion mechanism at the Ma River mouth and can serve as a basis for further studies and potential coastal management strategies in the area.

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