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## Article

# Ballast Pick-Up Velocity in the Vertical Pipe of an Excavating Machine Slurry System

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**Abstract:** Ballast pick-up velocity is a crucial parameter in the design of a slurry system in excavating devices, it influences transportation stability, safety, and economy of the system. In this study, a vertical pipe hydraulic experiment with different ballast sizes under water condition was conducted to investigate the ballast pick-up velocity in the shield slurry system. Thereafter, the coupling method of computational fluid dynamics and the discrete element method (CFD-DEM) was employed to establish a numerical model of the ballast pick-up velocity in the shield slurry system, and its applicability was verified by comparing the experiment results. Additionally, the ballast pick-up process under actual working conditions was studied, and the influence of ballast size and concentration on pick-up velocity was investigated. Overall, the primary findings of the study are as follows: 1) The numerical method can be applied to predict both the single ballast and ballast group pick-up velocity. 2) The pick-up process of the ballast group can be divided into four stages. 3) The conveying velocity of the ballast should be higher than the pick-up velocity of the single ballast to facilitate smooth discharge. 4) For a certain increase in ballast concentration, the pick-up velocity tends to be stable.

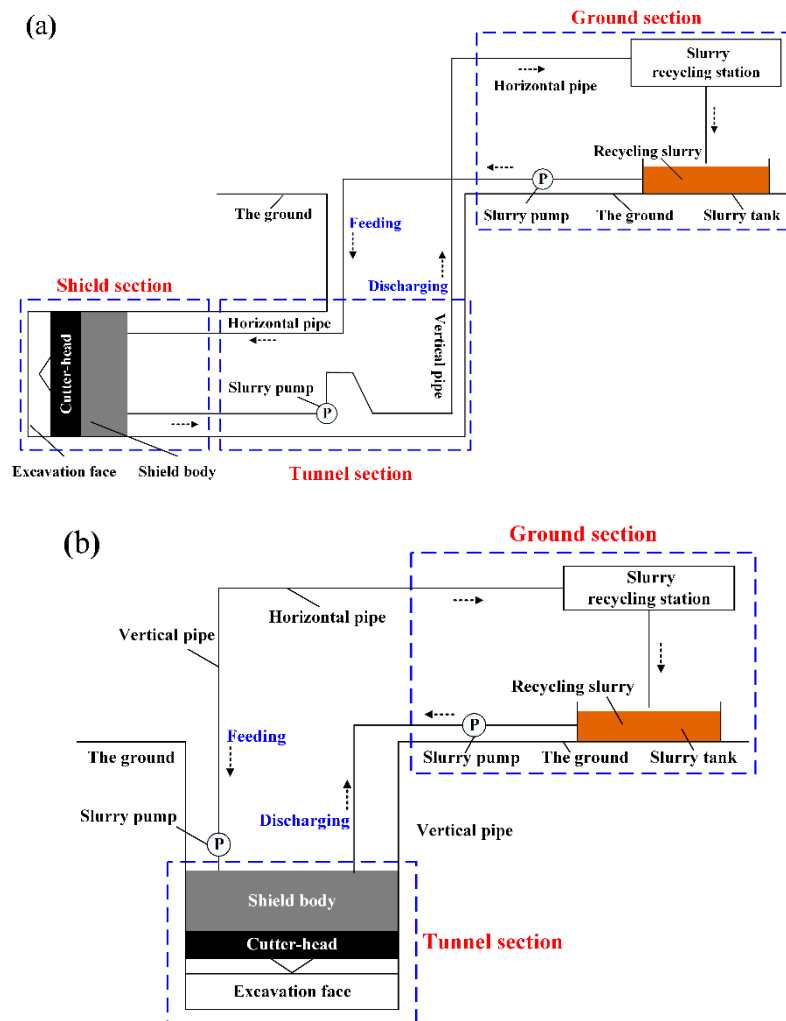
**Keywords:** shaft boring machine; slurry shield machine; slurry system; vertical pipe; ballast pick-up velocity; CFD; DEM

## 1. Introduction

With an increasing demand for utilising underground spaces, the application of various excavating devices is growing rapidly. During excavations, a slurry system is widely used to transport the soil excavated underground in front of the cutter-head to the surface. The slurry shield machine and shaft boring machine commonly adopt a slurry system to manage waste ballast. The slurry shield machine conducts excavations in the horizontal direction and the shaft boring machine vertically excavates to a target depth. For both excavating machines, the slurry system can be divided into two parts: the tunnel section and the ground section. Applying the slurry pump as the driver and the slurry as the vehicle, the excavated ballast is absorbed and delivered along the transporting pipe. In the ground section, the slurry system filters out the ballast, which enables the reuse of the slurry.

A schematic of the slurry system is shown in **Figure1**. In the slurry shield machine, shown in **Figure1 (a)**, the pipe of the slurry system is primarily arranged horizontally, while the vertical pipes are generally placed between the tunnel and ground sections at a height of 10-30 m. In the shaft boring machine, shown in **Figure1 (b)**, the pipes are primarily arranged vertically, with a height equal to the excavation depth.

Compared with ballast transportation in horizontal pipes, vertical pipes require a significantly higher velocity and energy consumption to lift the ballast, owing to the pressure loss. An insufficient capacity to carry the ballast in the vertical pipe may result in pipe jams, abnormal wear, and excess pressure loss. Therefore, studying the pick-up process of ballast in a vertical pipe is essential for the design of the slurry system.



**Figure 1.** Pipe layout schematic diagram of the slurry system: a) slurry shield machine; b) shaft boring machine.

In the pipe system hydraulic transportation design, pressure loss and ballast pick-up velocity must be involved. However, when the flow rate is much greater than the ballast pick-up velocity, excess energy is consumed, which reduces the economic benefits. Conversely, if the flow rate is less than the ballast pick-up velocity, the ballast would deposit at the bottom of the pipe, which limits the safe operation of the tunnelling project. Therefore, understanding the ballast pick-up velocity is important for the design of the pipeline conveying system.

Many experimental and numerical studies have been conducted on the ballast pick-up velocity in pipes. Van Wijk et al. [1] reported that the mineral pick-up velocity significantly impacts the transportation stability of vertical pipes in deep-sea mining. Zhang et al. [2] studied the ballast pick-up velocity and blockage mechanisms in vertical pipes by monitoring the mass flow rate, pressure fluctuation, and motion state information. Liu et al. [3] studied the influence of lateral vibrations on the ballast pick-up velocity in vertical pipelines and concluded that the ballast pick-up velocity significantly impacts the pipeline system's stability, safety, and energy consumption rate. Several studies [4–6] have investigated the ballast pick-up velocity in horizontal pipes by building a pneumatic conveying platform and analysing the influences of the pipe diameter, ballast shape, and stacking height. Other studies [7–9] have determined the ballast pick-up velocity using the weighing method and the image method, based on the pipeline experiment platform. In addition, experimental studies [10–15] have been conducted on the ballast pick-up velocity in the gas-solid two-phase flow system, where the impact of physical properties and motion state on the ballast pick-up velocity and the force characteristics were analysed.

In summary, the ballast pick-up velocity has been evaluated primarily through experimental studies by building a corresponding experimental platform through the weighing and graph method. As is evident from the previous research, the gas-solid two-phase flow system has been studied, while the object was mainly the horizontal pipe; furthermore, water was considered as the transportation medium. However, limited research has been conducted on vertical pipes with a slurry medium. Although ballast pick-up velocity results were directly observed via experimental methods, there were certain limitations. Different experimental platforms were required for other observed objects, which utilise vast resources and funds. To guarantee the accuracy of experimental results, the media was often required to be transparent. Furthermore, measurement methods and instruments required high specifications. Under actual working conditions, the conveying medium in the pipeline was always turbid and accompanying ballasts of a much larger size. Therefore, the existing research results cannot be directly used to predict the ballast incipient velocity under actual conditions.

With the rapid development of computer technology, various numerical simulation methods [16] have been applied to solve solid-liquid two-phase flow problems. Two methods, namely the Euler-Euler method and the Euler-Lagrangian method, have been commonly used to solve fluid-solid problems. The Euler-Euler multiphase model offers the advantage of considering the ballast volume effect on the fluid. Although there is limited research on the ballast pick-up velocity in pipes employing this method, there are more applications for ballast movement in multiphase flows. For example, Fatahi et al. [17] obtained the trajectory, residence time, and velocity of minerals in the machine, and Zhou et al. [18] obtained the volume fraction and the axial velocity of the ballast in the vertical pipe.

In addition, Zeng et al. [19] used the coupling method of computational fluid dynamics and the discrete element method (CFD-DEM) to obtain the influence of the ballast shape, ballast movement trajectory, and gas flow field distribution on the wear characteristics of the natural gas pipeline. Moreover, applying this approach, Petit et al. [20] studied the influence of airflow velocity on the result of the sand screening. Li et al. [21] also studied the ballast adsorption phenomenon and flow process in the gas-solid two-phase flow vortex through this method. Li et al. [22] introduced a specific application of the CFD-DEM method to transport ballast adsorbents in gas-cooled reactors in nuclear engineering. Naukkarinen et al. [23] used this method to study the ballast filtration process of the ion membrane. Furthermore, Zhao et al. [24] proposed a new wall roughness model and discrete random walk model based on this method to study the ballast flow characteristics in the turbulent flow of the horizontal tube under the dilute phase and dense phase conditions. Uzi et al. [25] applied this method to study the sodium chloride dissolution process in the brine, which proved the adaptability when analysing the ballast transport characteristics under the dilute phase flow conditions. Vango et al. [26] used the CFD-DEM unresolved method to study the flow characteristics of the fluidised bed under the dense phase flow conditions. Based on the time relaxation, a smoothing model was established with the strengthened robustness.

In summary, compared with experiments, simulation methods offer distinct advantages in terms of the completeness and convenience of capturing ballast movement information, scalability of the model, and applicability of working conditions. Therefore, in this study, the CFD-DEM method was used to investigate the ballast pick-up velocity in the vertical pipeline with the shield slurry system. First, an experiment of the ballast pick-up velocity in water was performed. Following the investigation, a simulation model of the single ballast and ballast group pick-up velocity was constructed, which was verified using the experimental results. Finally, based on the numerical simulation method, the incipient motion of the ballast group in the slurry was analysed to conduct a further study related to the influence of the ballast size and concentration on the pick-up velocity.

## **2. Ballast pick-up velocity theoretical modelling**

### *2.1. Ballast motion model*

Based on the discrete phase model, the ballast in the vertical pipeline interacts with the slurry, ballast, and inner pipe wall during the pick-up process. The translation and rotation of the ballast are accomplished by solving the Newton equation of motion as follows:

$$m_{b,i} \frac{d\mathbf{u}_{b,i}}{dt} = \sum_j \mathbf{F}_{C,ij} + \mathbf{F}_{bs,i} + \mathbf{F}_{g,i} \quad (1)$$

$$\mathbf{I}_{b,i} \frac{d\boldsymbol{\omega}_{b,i}}{dt} = \sum_j (\mathbf{M}_{t,ij} + \mathbf{M}_{r,ij}) \quad (2)$$

where  $\mathbf{u}_{b,i}$  and  $\boldsymbol{\omega}_{b,i}$  are the  $i$ th ballast translational and angular velocity, respectively;  $m_{b,i}$  is the ballast mass;  $\mathbf{F}_{C,ij}$  represents the contact force between the ballast-ballast and the ballast-pipe, where the contact force follows the Hertz-Mindlin non-slip model [27];  $\mathbf{F}_{bs,i}$  is the interaction force between the slurry and the ballast, which is described later;  $\mathbf{F}_{g,i}$  is the gravity of the ballast;  $\mathbf{I}_{b,i}$  represents the ballast inertia moment;  $\mathbf{M}_{t,ij}$  is the ballast tangential torque;  $\mathbf{M}_{r,ij}$  is the ballast rolling friction torque.

## 2.2. Slurry flow model

Regarding the slurry flow in the vertical pipeline, the Euler-Euler multiphase flow model was chosen as the corresponding solution where the slurry was treated as a single continuous phase. Described by the volume-averaged governing equations, the slurry obeys the continuity and momentum equations.

The continuity equation is as follows.

$$\frac{\partial}{\partial t}(\varepsilon_s \rho_s) + \nabla \cdot (\varepsilon_s \rho_s \mathbf{u}_s) = 0 \quad (3)$$

The momentum equation is as follows.

$$\frac{\partial}{\partial t}(\varepsilon_s \rho_s \mathbf{u}_s) + \nabla \cdot (\varepsilon_s \rho_s \mathbf{u}_s \mathbf{u}_s) = -\varepsilon_s \nabla p + \nabla \cdot (\varepsilon_s \boldsymbol{\tau}_s) + \varepsilon_s \rho_s \mathbf{g} + \mathbf{S} \quad (4)$$

where  $\varepsilon_s$  is the slurry volume fraction;  $\rho_s$  is the slurry density;  $\mathbf{u}_s$  represents the slurry velocity;  $p$  represents the slurry pressure;  $\boldsymbol{\tau}_s$  indicates the slurry shear stress;  $\mathbf{g}$  is the acceleration of gravity;  $\mathbf{S}$  reflects a term of the momentum exchange between ballast and slurry in a grid element.

## 2.3. Slurry-ballast interaction

The solution operations for the fluid phase and ballast phase are coupled together by the slurry volume fraction  $\varepsilon_s$  and the interaction force  $\mathbf{F}_{bs,i}$ . In this study, the minimum mesh size of the numerical model was 15 mm, while the size of the target ballast was in the range of 15-75 mm. The slurry volume fraction  $\varepsilon_s$  was calculated by the porous model [28], which has been proven to be applicable when the ballast size is larger than the fluid grid. The interaction force between the ballast and slurry on the unit grid is given as:

$$\mathbf{S} = \frac{\sum_{i=1}^m \mathbf{F}_{bs,i}}{\Delta V} = \frac{\sum_{i=1}^m (\mathbf{F}_{d,i} + \mathbf{F}_{p,i} + \mathbf{F}_{s,i} + \mathbf{F}_{M,i})}{\Delta V} \quad (5)$$

where  $\Delta V$  is the calculated grid volume,  $m$  indicates the number of ballasts in a single grid. The interaction force  $\mathbf{F}_{bs,i}$  contains variable forces; considering a series of forces such as the drag force  $\mathbf{F}_{d,i}$ , pressure gradient force  $\mathbf{F}_{p,i}$ , Saffman lift force  $\mathbf{F}_{s,i}$ , and the Magnus lift force  $\mathbf{F}_{M,i}$ , which plays an important role in solid-liquid flows [29].

The drag force  $\mathbf{F}_{d,i}$  [30] can be calculated based on the free stream drag force model as follows.

$$\mathbf{F}_{d,i} = 0.125 \pi d_b^2 C_d \rho_s |\mathbf{u}_{bs}^{rel}| \mathbf{u}_{bs}^{rel} \quad (6)$$

$$C_d = \begin{cases} 24/\text{Re}_b & \text{Re}_b \leq 0.5 \\ 24(1.0 + 0.15 \text{Re}_b^{0.687})/\text{Re}_b & 0.5 < \text{Re}_b \leq 1000 \\ 0.44 & \text{Re}_b > 1000 \end{cases} \quad (7)$$

$$\text{Re}_b = \frac{\varepsilon_s \rho_s d_b |\mathbf{u}_{bs}^{\text{rel}}|}{\mu_s} \quad (8)$$

The pressure gradient force  $F_{p,i}$  [31] is written as follows.

$$\mathbf{F}_{p,i} = -V_b \frac{d\mathbf{p}}{dx} = -V_b (\rho_s \mathbf{g} + \rho_s \mathbf{u}_s \frac{d\mathbf{u}_s}{dx}) \quad (9)$$

The Saffman lift force  $F_{s,i}$  [32] is expressed as follows.

$$\mathbf{F}_{s,i} = 1.61 d_b^2 (\mu_s \rho_s)^{1/2} |\boldsymbol{\omega}_s|^{-1/2} [(\mathbf{u}_s - \mathbf{u}_b) \times (\boldsymbol{\omega}_s)] \quad (10)$$

The Magnus lift force  $F_{M,i}$  [33] is given by.

$$\mathbf{F}_{M,i} = 0.125 \pi d_b^2 \rho_s [(0.5 \boldsymbol{\omega}_s - \boldsymbol{\omega}_b) \times (\mathbf{u}_s - \mathbf{u}_b)] \quad (11)$$

where  $C_d$  is the drag coefficient;  $d_b$  is the ballast size;  $\mathbf{u}_s$  is the slurry velocity;  $\mathbf{u}_b$  is the ballast velocity;  $\mathbf{u}_{bs}^{\text{rel}}$  is the velocity difference between the slurry and ballast;  $\text{Re}_b$  implies the ballast Reynolds number;  $\mu_s$  is the slurry viscosity;  $\boldsymbol{\omega}_s$  is the local fluid rotation, which is defined as  $\boldsymbol{\omega}_s = \nabla \times \mathbf{u}_s$ ;  $\boldsymbol{\omega}_b$  is the ballast rotation.

The CFD-DEM coupling procedure was achieved using CFD software (ANSYS FLUENT 15.0) and DEM software (EDEM 2020). Firstly, the slurry flow field was solved in the CFD software. Subsequently, the DEM provided ballast position and velocity information, and the interaction force  $F_{bs,i}$  was calculated according to Eq. (6)-(11). In the EDEM software, the position and velocity of ballasts were updated based on Eqs. (1) and (2). According to updated ballast status, the slurry volume fraction  $\varepsilon_s$  was calculated. Finally, the interaction force  $F_{bs,i}$  reacted on the fluid phase, based on Newton's third law.

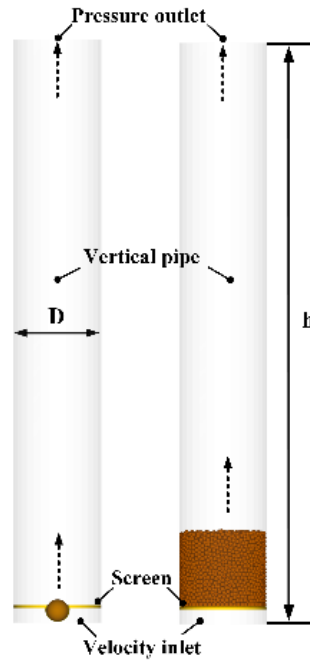
#### 2.4. Simulation model and boundary conditions

The numerical model of the ballast pick-up velocity in the vertical pipe was established by the CFD-DEM method, as shown in **Figure2**. The model mainly consisted of two parts: a vertical pipe and the ballasts. The pipe diameter was 300 mm, and the bottom part of the pipe was set as the velocity inlet for both conditions, while the top part of the pipe was employed as the pressure outlet. Furthermore, the slurry density was 1100 kg/m<sup>3</sup>, and the lower limit value of slurry viscosity was 0.01 Pa.s. The slurry properties were tested using a rotation viscosimeter, which has been used in previous work [34]. The rheological property model for the slurry was given, as shown in Eq. (12), while the non-Newtonian fluid model (power-law model) in CFD software was used to describe the slurry rheological characteristics.

$$\tau_s = 2.1 \gamma^{0.22} \quad (12)$$

where  $\gamma$  shows the slurry shear rate;  $\tau_s$  is shear stress.





**Figure 2.** Numerical model of the ballast pick-up velocity.

The no-slip boundary was adopted for the inner pipe wall. Furthermore, a user-defined function was applied to define the inlet velocity at the bottom, while the API programme removed the ballast that flowed through the top outlet. The pipe entrance velocity was in the range of 0.3-3.5 m/s. The slurry Reynolds number  $Re_s$  was between 4950 and 115500, which implied that the slurry flow state in the pipe was turbulent. Therefore, the standardised  $k-\epsilon$  turbulence model was used for the solution. The velocity and pressure of the numerical model were solved using the simple coupled method. The convective and diffusive terms were discretised by the second-order upwind and the second-order central difference method, respectively. The convergence criteria of the continuity and the energy residual were set as  $10^{-5}$ . To guarantee the stability of the convergence, the time calculation step size used in the CFD software was 0.004 s, while the time calculation applied in the DEM software had a step size of 0.0002 s.

As previously mentioned, a user-defined function was applied to endow the fluid at the inlet with a varying velocity over time. Because the ballast stress was in a state of dynamic equilibrium under this condition, for the single ballast, the judgement criteria for reaching the pick-up velocity was the transformation from static to upward movement to a dynamic force balance state of the ballast. In the simulation, for a single ballast, the pipe entrance velocity  $u_s$  was defined by the velocity function in Eq. (13). As a piecewise function, the entrance velocity was constant at 0 m/s during the period  $0 < t < 0.5$  s, and increased uniformly during the period  $t \geq 0.5$  s.

$$\begin{cases} u_s = 0 & 0 \leq t \leq 0.5 \\ u_s = 0.2(t - 0.5) & t \geq 0.5 \end{cases} \quad (13)$$

In terms of the ballast group, the pipe entrance velocity  $u_s$  was set as a periodic increasing function, where the initial velocity was 0 m/s. For every 2 s, the pipeline entrance velocity would be set stable, while for the next 1 s, the velocity would be increased by 0.05 m/s. Hence, for each 3 s, the velocity would be initially stable and then increased by 0.05 m/s for the next 3 s.

$$u_s = 0.05 \left[ \frac{(t+1)}{3} \right] \quad 0 \leq t \leq 60s \quad (14)$$

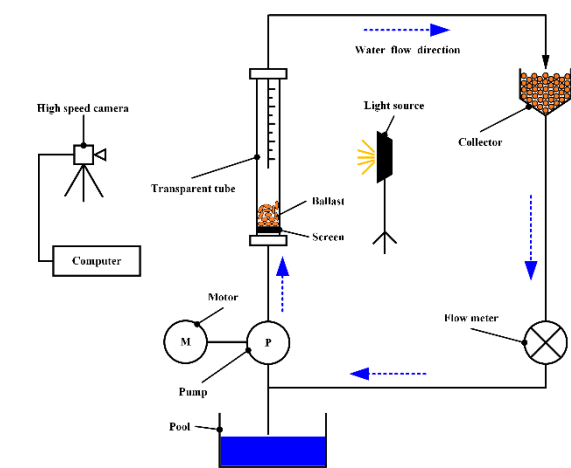
where  $[(t+1)/3]$  represents the integer part of  $(t+1)/3$ .

3. Model verification

To verify the accuracy of the simulation model, the ballast pick-up process was initially studied by experiments, following which the corresponding simulation model was constructed. Moreover, in the final stage, the simulation was compared with the experiment result.

3.1. Experimental scheme

The experiments were conducted for the ballast with a diameter of 10-50 mm in the water to study the ballast pick-up velocity, with the vertical pipe diameter and lifting height being 200 mm and 4 m, respectively. For the convenience of observation, a 2 m long transparent plexiglass pipe was installed in the middle of the pipe. The water flow rate of the vertical pipe was increased as the initial step by adjusting the pump working frequency. Following the monitoring of the ballast motion state information via the high-speed camera (Phantom V310, Vision Research Inc.), the ballast pick-up velocity was determined by the image and weight-loss methods. The working principle of the experimental platform is shown in **Figure3**.



**Figure 3.** Working principle of the experimental platform.

The ballast in the experiment was made of cement to ensure the sphericity of the ballast, thereby leading to the difference in ballast density between the experiment and the working condition. With the approximately spherical ballast production of ballast size 10-50 mm by a granulator, the experimental parameters of hydraulic lifting for the single ballast and the ballast group are listed in **Table 1**.

**Table 1.** Experimental operating parameters.

Experiment parameters	Value(s)
Ballast equivalent diameter (mm)	10, 20, 30, 40, 50
Ballast stacking quality (kg)	0.001-10
Ballast density (kg/m3)	2140
Water density (kg/m3)	1000
Water viscosity (Pa·s)	0.001
Ballast shape factor	0.91

3.2. Ballast group verification

In terms of the ballast group, first, the ballasts weighing 10 kg were arranged above the screen with the same ballast size. In line with the experiment, when the ballast could be lifted and distributed throughout the pipeline and more than 80 % of the ballast was transported out of the pipeline, the



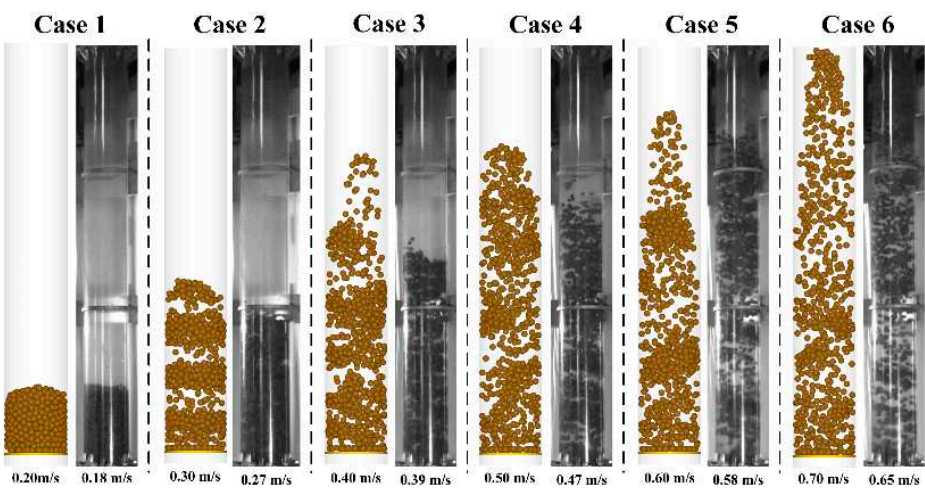
minimum corresponding water velocity was considered as the pick-up velocity of the ballast group. The ballast transport concentration is defined as follows:

$$C_v=\frac{V_b}{V_p}\times100\%$$

(14)

where  $V_b$  is ballast volume in the initial stage of accumulation;  $V_p$  is the volume of the transparent plexiglass.

According to Eq. (14), six groups of simulation cases under different water velocities were conducted for the ballast group, where the simulation time of each group was 10 s. The distribution state of the ballast group in the simulation conditions was compared with the experimental results, as shown in **Figure4**. With the increase in water flow rate, the velocity difference between the ballast and water increased, thereby increasing the liquid force acting on the ballast, and thus increasing the hovering height of ballast in the vertical pipeline. It can be concluded that the simulation results of the 20 mm ballast group distribution state under the different water velocities are consistent with the experimental observation results.



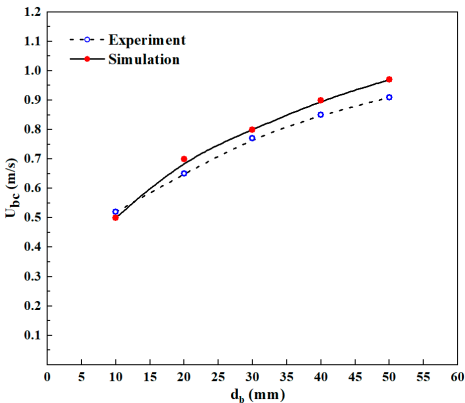
**Figure 4.** Simulation and experiment results of 20 mm ballast group distribution state. ( $d_b = 20\text{ mm}$ ,  $m_{bg} = 10\text{ kg}$ ,  $D = 200\text{ mm}$ ,  $h = 2\text{ m}$ ,  $C_v = 7.5\%$ ).

In the simulation, the ballast group pick-up velocity was determined according to the ballast distribution state and the ballast residual mass rate, as listed in **Table 2**.

**Table 2.** Ballast group pick-up velocity.

Parameters	Values
Ballast diameter (mm)	10, 20, 30, 40, 50
Experiment value (m/s)	0.52, 0.65, 0.77, 0.85, 0.91
Simulation value (m/s)	0.50, 0.70, 0.80, 0.90, 0.97

Under the different water velocity conditions, divergence existed in terms of the ballast lifting height and concentration degree, as shown in **Figure4**. As mentioned previously, the ballast group pick-up velocity was determined according to the ballast group distribution state. The pick-up velocity of the 10 kg weighted ballast group under the different ballast sizes was obtained through simulation. The comparison with the experimental results is shown in **Figure5**, where the maximum error rate was 7.69 %, and the average error rate was 5.61 %. While for the trend of variation, both the experiment and simulation provided a similar result. Hence, it can be determined that the simulation model is proven to be reliable.



**Figure 5.** Simulation and experimental results of the ballast group pick-up velocity with different ballast sizes.

4. Ballast group flow characteristics

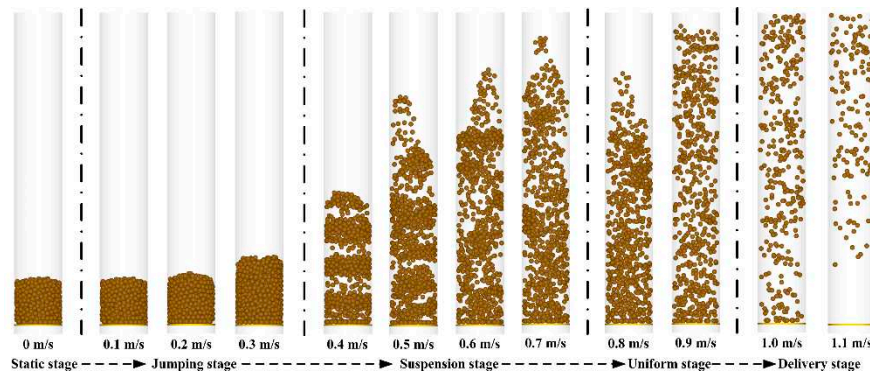
Based on the simulation model above, the ballast group initiation process and the influence factors of the pick-up velocity in the actual working condition were studied. The specific parameters used in the simulation are listed in **Table3**.

**Table 3.** Simulation setting parameters.

Simulation Parameters	Value(s)	Simulation Parameters	Value(s)
Ballast diameter (mm)	15-75	Sliding friction	ballast-ballast 1
Ballast stacking quality (kg)	0.001-110	coefficient	ballast-wall 0.5
Ballast density (kg/m3)	2650	rolling friction	ballast-ballast 0.05
Slurry density (kg/m3)	1100	coefficient	ballast-wall 0.15
Slurry viscosity (Pa·s)	0.01	restitution	ballast-ballast 0.35
Ballast shape factor	0.91	coefficient	ballast-wall 0.55

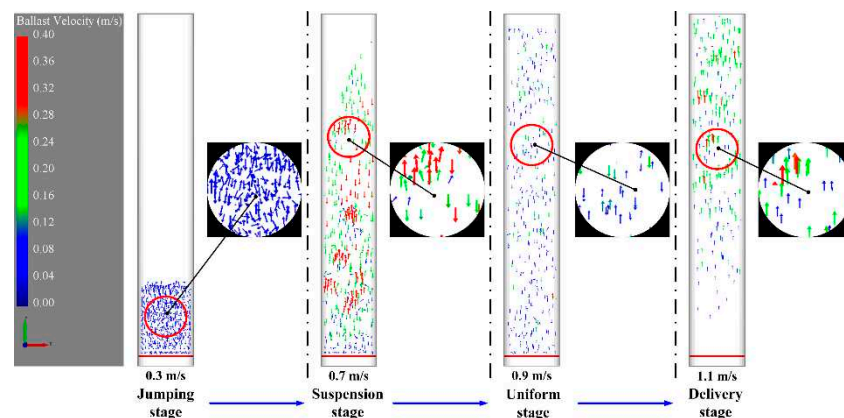
4.1. Ballast group initiation process in the vertical pipe

As shown in **Figure6**, the ballast group's initiation process in the vertical pipeline was studied when the slurry velocity increased from 0.1 to 1.1 m/s. Based on the four aspects (including the distribution state, velocity vector, average velocity, and mass residual rate of the ballast group), the initiation process of the ballast group was divided into four stages; jumping, suspension, uniform, and delivery. According to the ballast distribution state, in the jumping stage, the ballast mainly settled at the bottom of the pipe with the slurry velocity ranging between 0.1 and 0.2 m/s. Moreover, the ballast group distribution state appeared similar to that in the static stage. When the slurry velocity was at 0.3 m/s, the ballast group was lifted slightly. Further, as the slurry velocity increased to 0.4 m/s, the ballast group entered the suspension stage. While the slurry velocity was between 0.4 and 0.5 m/s, the ballast group changed from the bottom accumulation to the middle-upper intermittent distribution. In addition, when the slurry velocity was between 0.6 and 0.7 m/s, the ballast group switched to a uniform distributed state. Further, when the slurry velocity increased to 0.8 m/s, the ballast group entered the uniform stage, which exhibited greater uniform distribution than the suspension stage. Finally, when the slurry velocity was 1.0 m/s, the ballast group entered the delivery stage, where the ballast at the bottom was significantly reduced compared with other stages. With a gradual shift to the upper part of the pipe, the ballast group was finally transported out of the pipeline. Similar phenomena can also be observed in the experiment.



**Figure 6.** Ballast group distribution state under different slurry velocities ( $d_b = 30$  mm,  $m_{bg} = 30$  kg,  $D = 300$  mm,  $h = 2$  m,  $C_v = 8.1$  %,  $t = 8$  s).

The ballast velocity vector diagram can be used to obtain the ballast motion direction and velocity during the initiation process, as shown in **Figure 7**. When the slurry velocity was 0.3 m/s, the ballast group was located at the jumping stage. It can be observed that the ballast group exhibited a phenomenon of ascending and descending alternately, with no visible movement trend and a small motion velocity. However, at the slurry velocity of 0.7 m/s, the ballast group was in the stage of suspension, where an apparent downward movement was observed. At this stage, the ballast group in the central area of the pipe was initially lifted upward, and after reaching a certain height, the group moved towards the inner pipe wall and exhibited a downward trend. Furthermore, for the slurry velocity of 0.9 m/s, the ballast group reached the force equilibrium state at the uniform stage. With a motion trend similar to the jumping stage, the ballast velocity experienced a significant decline. Finally, when the slurry velocity was 1.1 m/s, the ballast group was in the delivery stage, where the upward motion of the ballast was visible and the falling movement disappeared. Moreover, the ballast lifting velocity was higher than the downward falling speed. During the delivery stage, the pick-up velocity of the 30 mm single ballast was 1.0 m/s. Furthermore, when the slurry velocity appeared higher than the single ballast pick-up velocity, the whole ballast group was uplifted, enabling the ballast to be smoothly exported outside the pipeline.



**Figure 7.** Ballast group velocity vector diagram under different stages.

Considering the various directions of the ballast movement during the initiation process, the upward motion was set as a positive value, while the downward motion was set as a negative value. The average velocity of ballasts under the different slurry flow rate conditions is displayed in **Figure 8**. It can be observed that the average velocity was relatively small during the jumping stage. When the slurry velocity increased from 0.1 to 0.3 m/s, the ballast group fell, while the velocity exhibited a slightly increasing trend. Subsequently, with the increase from 0.3 to 0.65 m/s, the ballast group entered the suspension stage, where the ballast group falling velocity continued to increase rapidly.

Furthermore, when the slurry velocity increased from 0.65 to 0.75 m/s, the upward motion of the ballast was enhanced, and the falling velocity of the ballast group exhibited a downward trend. When the slurry velocity increased from 0.75 to 0.9 m/s, the ballast group entered the uniform stage where the average velocity was higher than 0 m/s, and the movement trend of the ballast group was an overall upward lifting. However, when the slurry velocity increased from 0.8 to 0.95 m/s, the upward lifting velocity of the ballast group decreased, owing to the balance of the interaction between the remaining ballast in the pipeline and slurry. Finally, when the slurry velocity exceeded 0.95 m/s, the ballast group entered the delivery stage. Following a further increase in the slurry velocity, the ballast group motion velocity gradually increased, while the ballast exhibited an upward motion trend, with the disappearance of the falling phenomenon. During this stage, the ballast group was smoothly transported outside the pipe.

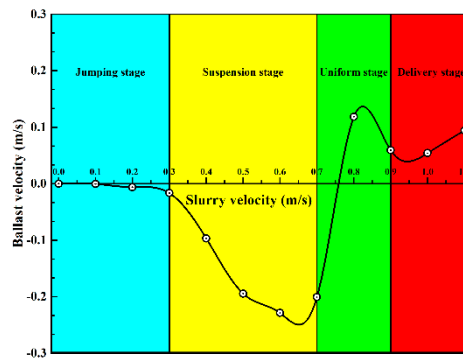


Figure 8. Law of ballast group average velocity under different slurry velocities.

After analysing the ballast mass residual rate in the pipeline at the different motion stages, the variation curve was obtained, as shown in Figure 9. It is evident that in the velocity range of 0.0 and 0.6 m/s, the ballast group experienced the jumping stage and the initial suspension stage, where the ballast in the pipeline could not be discharged. Therefore, the ballast mass residual rate was 100 %. Regarding the slurry velocity between 0.6 and 0.9 m/s, the ballast group was at the end of the suspension stage or the uniform stage, while only part of the ballast was discharged until the end of the suspension stage. Furthermore, when the slurry velocity was higher than 0.9 m/s, the ballast group entered the delivery stage, and the ballast in the pipe gradually decreased until it was completely discharged.

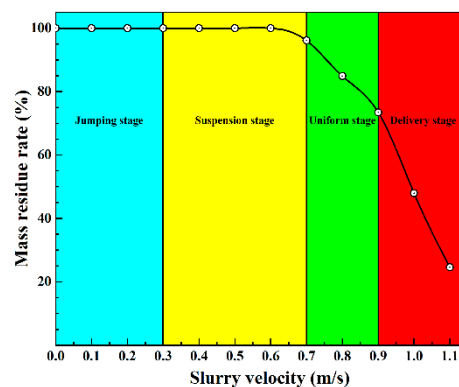
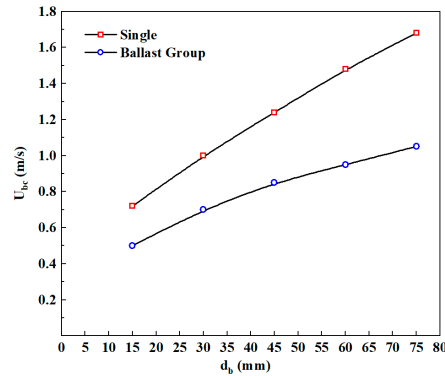


Figure 9. Variation curve of the ballast mass residual rate in the vertical pipeline.

#### 4.2. Effect of ballast size on pick-up velocity

Under the actual working conditions, the pick-up velocity of the 30 kg ballast group was analysed, and the comparison of the pick-up velocity between a single ballast and the ballast group is shown in Figure 10. It can be seen that for both conditions, the pick-up velocity exhibited an

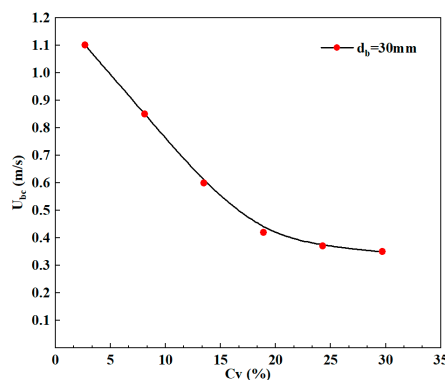
increasing trend with increasing diameter, while the slope of the ballast group decreased. The pick-up velocity of a single ballast was higher than that of the ballast group with the same ballast size. For example, the pick-up velocity corresponding to a 30 mm single ballast was 1.00 m/s, while that of the ballast group with the same size and the total weight of 30 kg was only 0.70 m/s. As the size increased to 60 mm, the ballast group pick-up velocity reached 0.95 m/s.



**Figure 10.** Single ballast and ballast group pick-up velocity under different ballast size conditions.

#### 4.3. Effect of ballast concentration on pick-up velocity

The ballast concentration  $C_v$  in the slurry system experienced a transformation, owing to the tunnelling speed and geology during practical operation. As shown in **Figure 11**, the ballast pick-up velocity  $u_{bc}$  with a diameter of 30 mm under the different concentration conditions was obtained through analysis. When  $C_v$  increased from 2.7 to 18.7 %,  $u_{bc}$  rapidly decreased from 1.1 to 0.42 m/s. However, when  $C_v$  was higher than 18.7 %,  $u_{bc}$  decreased slowly and gradually tended to be stable. According to the simulation and experiment results, the increase in the ballast concentration significantly reduced the ballast group pick-up velocity. This is because the collision force between the ballast reduced the effective ballast gravity such that the demand for drag force of the ballast was reduced accordingly. The increase in ballast concentration improved the ballast collision frequency. When  $C_v$  was between 2.7 and 18.7 %, the ballast collision frequency increased from 20 to 523. Therefore, it is evident that the ballast collisions may be beneficial for picking up the ballast in the vertical pipe and significantly reducing the ballast pick-up velocity. In addition, with the increase in  $C_v$  from 18.7 to 24 %, the frequency of ballast collisions increased from 523 to 615, which is close to saturation. Generally, the transportation of a single ballast with a large size should be prevented during the transportation to lower the risk of blockage in the vertical pipe.



**Figure 11.** Ballast group pick-up velocity under different ballast concentration conditions.



## 5. Conclusion

This study determined the ballast group pick-up velocity by using the hydraulic lifting experimental platform. Based on the CFD-DEM method, a simulation model of the ballast pick-up velocity was constructed. In addition, the influencing factors of the ballast pick-up velocity under the actual working conditions were also studied. The main findings of the study are as follows:

(1) The simulation model of the ballast pick-up velocity was proposed based on the experiment of the ballast pick-up velocity. Moreover, the accuracy of the simulation model was verified by comparison with the experimental results.

(2) Based on the simulation model, the ballast initiation process was studied. It was observed that when the conveying speed of the ballast group was higher than the pick-up velocity of a single ballast, the ballast in the pipeline could be smoothly discharged.

(3) The ballast pick-up velocity increased with the increase in ballast size. In the working conditions, the ballast pick-up velocity with a diameter of 75 mm was as high as 1.68 m/s. Thus, the minimum slurry velocity should be higher than 2 m/s, which implies that the minimum flow rate of the slurry pump should be controlled over 510 m<sup>3</sup>/h.

(4) The increase in the ballast concentration evidently reduced the ballast pick-up velocity. However, the ballast pick-up velocity was less affected when the local ballast concentration in the vertical pipe increased to more than 18.7 %. It is suggested that the local ballast concentration in the vertical pipe of the slurry system should be controlled by approximately 20 %.

In engineering applications, due to the limits of the environment, the slurry system consists of vertical, horizontal, and inclined parts. It is not sufficient to study only the ballast pick-up velocity in the vertical pipe; further studies on the horizontal and inclined pipes are required. Meanwhile, the follow-up research will focus on the prediction model of the ballast pick-up velocity.

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## Appendix

Notation	Definition	Units
$u_b$	ballast translational velocity	m/s
$\omega_{b,i}$	ballast angular velocity	rad/s
$m_{b,i}$	ballast mass	kg
$F_{C,ij}$	contact force	N
$F_{D,i}$	fluid drag force	N
$F_{p,i}$	pressure gradient force	N
$F_{M,i}$	Magnus lift force	N
$F_{S,i}$	Saffman lift force	N
$F_{g,i}$	ballast gravity	N
$I_{b,i}$	ballast inertia moment	kg·m <sup>2</sup>
$M_{t,ij}$	tangential torque	N·m
$M_{r,ij}$	rolling friction torque	N·m
$C_d$	ballast drag coefficient	–
$\rho_s$	slurry density	kg/m <sup>3</sup>
$u_{bs}^{rel}$	relative velocity	m/s
Re	Reynolds number	–



$d_b$	ballast size	mm
$\mu_s$	slurry viscosity	Pa·s
$\varepsilon_s$	slurry volume fraction	–
$u_s$	slurry flow velocity	m/s
$\tau_s$	slurry shear stress	Pa
$p$	slurry pressure	Pa
$g$	acceleration of gravity	m/s <sup>2</sup>
$S$	momentum exchange term	kg/(m·s) <sup>2</sup>
$\gamma$	slurry shear stress rate	s <sup>-1</sup>
$F_{bs,i}$	interaction force	N
$\Delta V$	calculated grid volume	m <sup>3</sup>
$t$	simulation time	s
$C_v$	ballast concentration	%
$m_{bg}$	ballast stacking quality	kg
$Q_b$	ballast density	kg/m <sup>3</sup>
$h$	pipe height	m
$D$	pipe diameter	mm
$u_{bc}$	ballast pick-up velocity	m/s
API	Application Programming Interface	

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