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*Technical Note*

# Field Information Modeling (FIM)<sup>TM</sup>: Best Practices using Point Clouds

Reza Maalek <sup>1,\*</sup>

<sup>1</sup> Endowed Chair of Digital Engineering and Construction, Institute of Technology and Management in Construction, Karlsruhe Institute of Technology, Karlsruhe, Germany; reza.maalek@kit.edu

\* Correspondence: reza.maalek@kit.edu

**Abstract:** This study presents established methods, along with new algorithmic developments, to automate the point cloud processing within the Field Information Modeling (FIM)<sup>TM</sup> framework. More specifically, given an n-D designed information model, and the point cloud's spatial uncertainty, the problem of automatic assignment of the point clouds to their corresponding elements within the designed model is considered. The methods addressed two classes of field conditions, namely, (i) negligible construction errors; and (ii) existence of construction errors. The emphasis was given to describing and defining the assumptions in each method and shed light on some of their potentials and limitations in practical settings. Considering the shortcomings of current point cloud processing frameworks, three new and generic algorithms were developed to help solve the point cloud to model assignment in field conditions with both negligible, and existence (or speculation) of construction errors. The effectiveness of the new methods was demonstrated in real-world point clouds, acquired from construction projects, with promising results.

**Keywords:** Field Information Modeling (FIM)<sup>TM</sup>; point cloud to BIM; point cloud vs. BIM; n-D information modeling; digital engineering and construction

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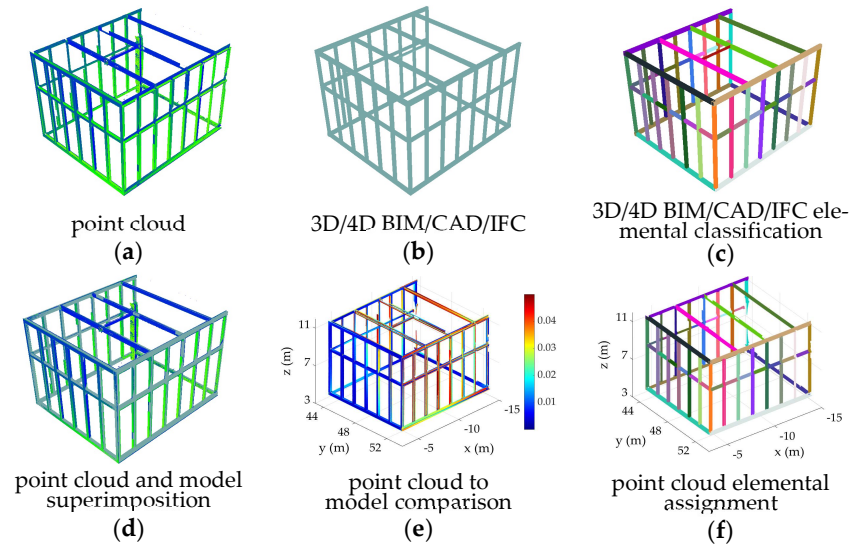
## 1. Field Information Modeling (FIM)<sup>TM</sup>

Construction project information modeling frameworks, such as building information modeling (BIM), heritage building information modeling (H-BIM), or bridge information modeling (BrIM), involve modeling and integrating intelligent and semantic information within multi-dimensional (n-D) computer-aided design (CAD) models [1–3]. During the design stages, the 3-dimensional (3D) digital model of a construction project can be created, whereby each element is classified based on attributes such as functional type (e.g. structural wall), elemental relationships (e.g. structural wall and floor slab connectivity and interaction), and geometric properties (e.g. shape and size) [4,5]. Further modeling can be carried out so as to integrate project planning and control information, such as work sequences and duration (e.g. 4D BIM [6]), as well as cost (e.g. 5D BIM [7]), enabling the project management team to directly evaluate the impact of design changes on the project's schedule and cost. During construction, the designed n-D model serves as a detailed baseline to aid field construction work. Relevant field data must then be collected and compared to the designed model to ensure compliance. Particularly within the lean project delivery [8], recording fast, frequent, and reliable field data is desired to foster continual improvement. In the context of schedule and cost control for instance, daily measurement of percent planned complete, recommended as a part of the Last Planner® system [9], combined with frequent earned value analysis [10], require up-to-date knowledge of the progress of activities. Hence, Field Information Modeling (FIM)<sup>TM</sup> is essential to model and transform collected field data into intelligent, tangible, and semantic digital models as a means of promoting the seamless flow of information between the field and the digital world.

### 1.1. FIM and Point Clouds

Amongst the various types of field information that can be acquired, 3D point clouds provide a unique opportunity to represent surrounding real-world surfaces as discrete point coordinates (and in some cases with surface color or intensity information). This enables simultaneous control of dimensional quality (e.g. size or plumbness) as well as progress (e.g. percentage of complete) of field objects corresponding to work packages/activities [11]. Irrespective of the method used to acquire the point clouds, automated assignment of points to their corresponding element in the designed n-D information model is an integral part of automating the FIM process.

A typical framework to automate the assignment of points to the corresponding elements in the designed model consists of the following steps (shown visually in Figure 1):



**Figure 1.** Typical framework for automatic assignment of points to the designed n-D information model elements: a) point cloud, b) n-D information model; c) elemental classification of the designed model; d) superimposition of the point cloud and the model; e) point cloud to model comparison -in this case the color represents the distance of the points to the closest element's surface-; f) result of assignment of the point cloud to their corresponding elements.

1. Elemental classification of the design information model (Figure 1c), which is the process of labeling and detecting every element within the designed model based on criteria such as functional type and surface geometry. The surface geometry information related to element type already exists within the BIM, and the industry foundation class (IFC) models [4,5] and if available, can, be utilized directly. In other cases, particularly when only 3D CAD models exists, the STereoLithography (STL) file format can be deployed, which approximates CAD object surfaces with triangular planes [12]. To further group triangular planes of the same surface together, a mesh segmentation process can be deployed, such as those proposed in [13–15]. The output of this stage is presented visually in Figure 1c, where each element is separated and shown with a unique color.
2. Superimposition of the point cloud and the classified model (Figure 1d), which involves the registration of the coordinate systems of the point cloud and the model. To perform point cloud to model registration, at least three non-colinear point correspondences between the model and the point cloud is necessary [16]. In the presence of construction error, which is a reasonable assumption given the statistics related to rework due to poor construction [17], it is recommended to use identifiable and signaled targets on pre-surveyed site control points to perform the registration [11,18]. In other cases where construction errors is less prominent, iterative closest point (ICP) registration of the point cloud and the closest surfaces on the model can be performed through methods such as scan vs. BIM proposed in [12].

3. Point cloud to model comparison and assignment, which is used to aid with the assignment of the point cloud to the model elements. In the case of negligible construction errors, the distance of the point to the closest element can be used to determine correspondence, which is the metric used in the scan vs. BIM of [12]. A distance threshold must then be deployed to reject far points. Figure 1e shows an example of a heatmap used to visualize the distance of the points to the closest classified elements. Figure 1f presents the resulting point cloud to element assignment with an arbitrarily defined distance threshold of 50mm. In other cases, where possible local construction errors exist (e.g. a set of elements are incorrectly constructed or assembled), the correct assignment may require to use additional information such as point neighborhood behavior [18], curvature [19], color [20] or intensity [21].

The output of stage 3 above can then be employed for progress monitoring [11,22], dimensional quality checking [11], and 3D BIM updating [23].

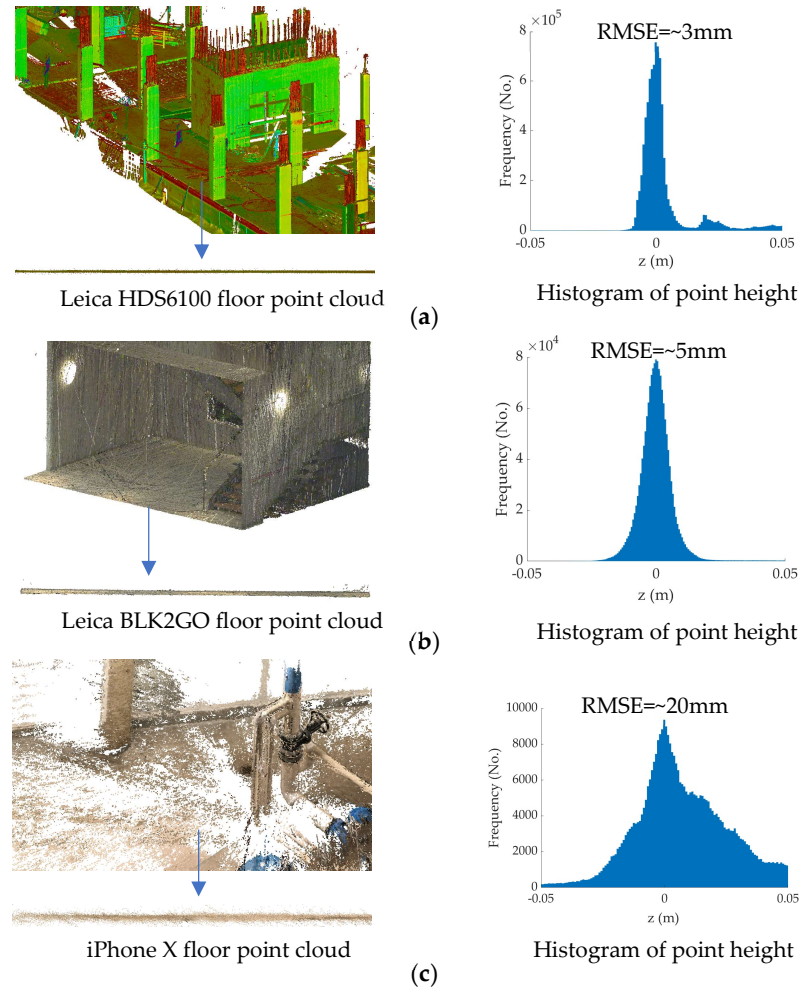
### 1.2. Manuscript Objectives, Scope, and Structure

The goal of this manuscript is to present the best practices, along with new algorithmic developments, to automate the FIM process using point clouds, given a designed n-D information model. The scope of this study is to focus particularly on methods to solve stage 3 (and to a lesser extent stage 2) of the framework presented in Section 1.1 (Figures 1e and 1f) and using only the geometric primitives (i.e. Cartesian coordinates) of point clouds. The emphasis is given to direct frameworks, which are generalizable, robust to changes in scenery and point cloud type, and do not contain subjectively defined thresholds. To this end, the remainder of the manuscript is structured as follows:

- Section 2 describes the spatial uncertainty of point clouds and the necessity to incorporate this spatial uncertainty within a successful point cloud processing framework.
- Section 3 introduces point cloud processing when construction errors can be considered negligible. A new process for point cloud vs. BIM is proposed along with a demonstration of its effectiveness on real world point clouds.
- Section 4 reveals the basic considerations for analysis of point clouds when the impact of construction errors cannot be neglected. New methods for point cloud neighborhood definition, surface hypothesis-based classification, and surface segmentation, along with demonstrated real world examples are thoroughly discussed.
- Section 5 summarizes the findings and provides avenues for further explorations.

## 2. Point Cloud Spatial Uncertainty

Point clouds can be acquired through different means, such as terrestrial laser scanners (TLS: Figure 2a), simultaneous localization and mapping (SLAM)-based mobile systems (Figure 2b), and structure-from-motion (SfM)-based photogrammetric 3D reconstruction (Figure 2c). Each point cloud collection technique has unique characteristics that impact the level of spatial uncertainty of each point measurement. A generic framework to automate point cloud processing on construction projects must, hence, incorporate the impact of point measurement uncertainties corresponding to the optical instrument [24]. Figure 2 sheds light on the importance of integrating the expected point measurement uncertainties within automated point cloud processing frameworks. Figure 2-right shows the histogram of point height, which has been shown to be an effective tool in extracting flat slab floors and ceilings from point clouds [25,26]. The points on the vicinity of the mode of the histograms represent the points of the floor (base slab). For reference, the root mean squared error (RMSE) of the points on the floor for each histogram is also shown. As illustrated, the RMSE changes from 3mm for TLS (Figure 2a-right) to 20mm for dense 3D reconstruction from smartphone cameras (Figure 2c-right). This shows that if, for instance, a fixed distance threshold of say 5mm is adopted to assign points around the mode of the histogram as floors, 99% of the TLS and only 32% of the smartphone 3D reconstruction points will be identified as belonging to the floor. Therefore, a successful distance threshold (Figure 1f) must incorporate the expected point measurement uncertainties related to a particular point cloud instrument [11,27–29].



**Figure 2.** Sample point cloud (left) and histogram of point height (right) for: a) Leica HDS 6100 TLS; b) Leica BLK2GO mobile scanner; and c) 3D reconstruction using iPhone X images.

Given a calibrated point cloud collection instrument (assumption of modeled systematic errors), the point measurement uncertainties in the Cartesian coordinate system (i.e. spatial uncertainty [28]) can be modeled through the propagation of instrumental measurement errors, typically approximated to the first order [30]. For instance in TLS, the spatial uncertainty of points must incorporate the raw instrumental measurement errors (e.g. range and angular errors; see equation 11 of [31]) along with possible scan station registration errors -in case of multiple scans- (see equation 5 of [11]). For each point, the variance propagation process provides a  $3 \times 3$  covariance matrix,  $\Sigma_X$ , as measurement of spatial uncertainty. Closed formulations of the 3D positional uncertainty,  $\Sigma_X$ , as a function of instrumental measurement errors can be found in Maalek et al. [11] for TLS (Figure 1a), Zhengchun et al. [28] for laser radar measurement system (LRMS), Mourikis and Roumeliotis [32] for SLAM-based instruments (Figure 1b), and Beder and Steffen [33] for SfM-based techniques (Figure 1c). For each point,  $X_i$ , the corresponding spatial uncertainty,  $\Sigma_{X_i}$ , can be used to construct an error ellipsoid as follows:

$$(X - X_i)^T \Sigma_{X_i}^{-1} (X - X_i) \leq \chi^2_{p,\alpha'} \quad (1)$$

where  $\chi^2_{p,\alpha'}$ , the amplification coefficient, is considered a chi-squared probability with confidence,  $p$  (in this study,  $p = 97.5\%$  is used), and  $\alpha$  degrees of freedom ( $\alpha = 3$  for 3D data). Throughout the remainder of the manuscript, every point will be represented by its error ellipsoid using equation 1.

### 2.1. Assumptions of Input Parameters

Before describing the methods of processing point clouds in Sections 3 and 4, it is worth outlining the basic assumptions and available input parameters, as follows:

- It is expected that stages 1 and 2 of the typical point cloud processing, corresponding to Figures 1a through 1d are reliably completed. This means that a point cloud from the field is acquired, the instrument by which the point cloud was collected is known, the elemental classification of the n-D designed information model is carried out, and an initial target-based registration of the point cloud and the model is performed.
- The point cloud instrument is assumed to be calibrated, containing no (or negligible) systematic error trends.
- The spatial uncertainty of each point is modeled and the error ellipsoid as per equation 1 is constructed for each point.

### 3. Point Cloud Analysis: Case of Negligible Construction Errors

This section describes the methods that can be incorporated when construction errors are considered negligible (for instance in cases of reliable pre-fabrication [34] and/or 3D printing [35] processes). This suggests that the designed n-D model and the point cloud acquired from the constructed scene must comply with little to no construction errors. Therefore, given an initial reliable point cloud to model registration, the problem of point cloud processing reduces to assigning the point to its closest element and rejecting points that are farther than a pre-defined threshold. This type of procedure was adopted for 3D reconstructed photogrammetric point clouds in Golparvar-Fard [22], and for TLS point clouds within scan vs. BIM of Bosche [12]. The two methods, however, differ in some detail. For the photogrammetric case, since the position and orientation of the images are estimated during the dense 3D reconstruction stage [36], Golparvar-Fard [22] proposed to perform a perspective projection of the 3D model with the same view angle at each camera position and orientation. The 3D model is now converted into a 2D image and superimposed onto the original image to enable pixel to model assignment within each image. The image pixel is assigned to the closest 2D model pixel. The conversion of the 3D model to a 2D image possesses some merits. Firstly, since the positional uncertainties of the cameras have been already calculated during the SfM process, it can be incorporated within the assignment of each pixel to the corresponding model elements in the image. Furthermore, the assignment is performed in 2D rather than 3D, which can be faster, and if required, can allow the utilization of the many available and established image processing frameworks. The process will, however, lose efficiency when occlusions are present on site due to the loss of information with dimensionality reduction of 3D to 2D. Therefore, additional steps, such as supervised learning may be required to improve the pixel to model assignment in the existence of occlusions [37]. Furthermore, the process is only suited for images and cannot be generalized for point clouds acquired by other means, such as TLS.

The scan vs. BIM method [12] first generates a template designed point cloud by parallel projection of the acquired point clouds onto the closest surfaces in the designed model. The method then performs an iterative closest point (ICP) registration between the point clouds and the designed template point clouds until convergence. At each iteration only points within a distance threshold -a function of a constant distant,  $\epsilon_{Const.}$ , and the registration errors in the previous iteration- are used for registration and assignment. The method is attractive since it reduces the point cloud to model assignment into a template matching problem [38,39], and aims to also minimize possible target-based registration errors. The method, however, does not incorporate the impact of instrumental measurement errors pertaining to the point's spatial uncertainties (see Section 2). Furthermore,  $\epsilon_{Const.}$  was arbitrarily defined as 50mm in the study, which may not be generic for all datasets (the impact of  $\epsilon_{Const.}$  on the ICP registration will be demonstrated later). Finally, the ICP registration is based on point-to-point correspondences, which is known to be less accurate than point to plane registration [40,41]. Therefore, direct point cloud to plane



registration can not only improve the registration accuracy (and convergence rate), but it can also remove the necessity for generating the designed template point cloud altogether.

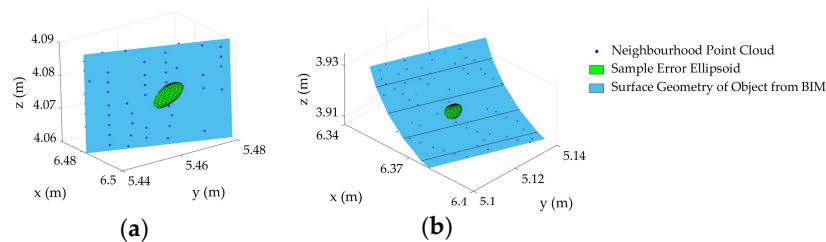
### 3.1. A New Generic Process for Point Cloud vs. Model

With due consideration of the limitations of the aforementioned methods, a new generic method, **Algorithm 1: Point Cloud vs. Model**, given the assumptions and inputs of Section 2.1, is proposed as follows:

1. Iterative improvement of the point cloud to model registration:
  - 1.1. Assign points to corresponding planar surfaces in the model as below:
    - 1.1.1. For each point, find the closest planar surfaces whose distance to the point is smaller than the semi-major length of the error ellipsoid (see Algorithm 5-step 2 of [11]).
    - 1.1.2. Assign the point to the closest planar surface that intersects the error ellipsoid (ellipsoid to surface intersection is treated in Appendix A).
  - 1.2. For each planar surface, find the inlier points of the set of points, which were assigned to the plane in step 1.1, using the method proposed in Appendix B.
  - 1.3. Perform the closed-form point to plane registration, proposed by Khoshelham [41] to transform the inlier points onto the corresponding model planes.
  - 1.4. Transform all points using the registration parameters of step 1.3.
  - 1.5. Perform steps 1.1-1.4 until the points used for registration between two consecutive iterations remain constant.
2. Assignment of non-planar points to non-planar model surfaces:
  - 2.1. Identify the points which were not assigned to a planar surface (the remaining points after step 1).
  - 2.2. Find the closest non-planar surface that intersects the error ellipsoid of the point (Appendix A).
  - 2.3. For each surface in the model, perform the robust method of Appendix B to find the inlier points.
  - 2.4. Assign the inlier points to the corresponding surface and exit the algorithm.

In Algorithm 1, given a reliable registration, a point is assigned to a surface if the following two conditions are met:

- Its error ellipsoid intersects the surface. This is schematically shown in Figures 3a and 3b for planar and cylindrical surfaces, respectively. The intersection of ellipsoids and common surfaces such as planes, cylinders, spheres, and ellipsoids will be covered in Appendix A.
- The point follows the pattern of the assigned surface (an inlier point). The algorithm for detecting inlier points, given a particular model will be discussed in Appendix B.



**Figure 3.** Schematic intersection of a sample ellipsoid with 3D model: a) planar surface; and b) cylindrical surface.

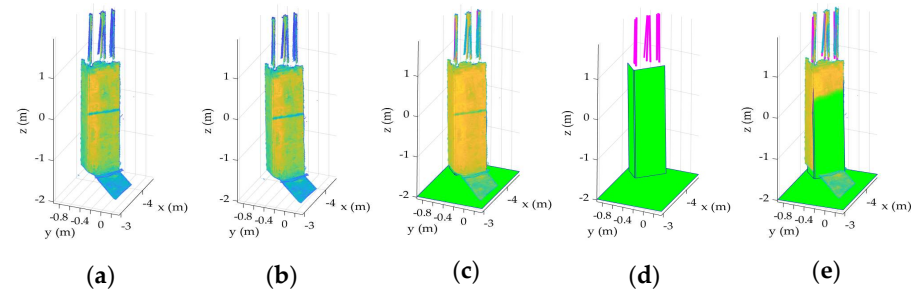
Step 1 of Algorithm 1 involves an iterative process to improve the registration in the absence of a reliable initial target-based registration (similar to the ICP of [12]). The registration process, however, only uses planar surfaces. Point to plane registration is used particularly since it is known to be accurate [40], and reliable closed-form linear solution to the point to plane registration problem already exists [41] -unlike say cylinder-based registration, which is non-linear at best [42].

### 3.2. Demonstration 1: Comparison of Scan vs. BIM and Point Cloud vs. Model

#### 3.2.1. Experimental Setup

Point cloud data from one rectangular concrete column with rebars on top, acquired from a building construction project is used (Figure 4a; data presented in [11,25]). To simulate a model with no construction errors, the following processes are carried out:

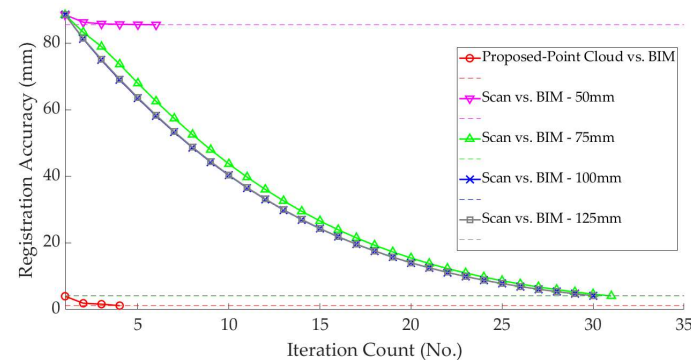
1. The point cloud of the sample column was subjected to a rigid body transformation (translation and rotation; Figure 4b).
2. The as-built model of the rectangular column surfaces as well as the cylindrical rebars for the transformed point cloud was generated manually using the FARO As-built Modeler software [43] (Figures 4c and 4d).
3. The original point cloud is then superimposed onto the as-built model of the transformed point cloud for comparison (Figure 4e).
4. The proposed point cloud vs. model was compared to the scan vs. BIM method. The scan vs. BIM method was performed using four distance configurations with  $\epsilon_{Const.}$  changing from 50mm to 125mm in 25mm increments.



**Figure 4.** Experimental setup for demonstration 1: a) original point cloud; b) translated and rotated point cloud; c) as-built model of the transformed point cloud of Figure 4b superimposed onto the point cloud; d) as-built model of the transformed point cloud; and e) as-built model of the transformed point cloud superimposed onto the original point cloud.

#### 3.2.2. Results: Registration Error Comparison

Figure 5 shows the results of the registration error in each iteration using our method and the four configurations of scan vs. BIM. Two main observations were made from the results shown in Figure 5. Firstly, scan vs. BIM appears to be considerably impacted by the change in  $\epsilon_{Const.}$ . In our data,  $\epsilon_{Const.}$  of 100mm and 125mm were the most optimum and produced almost identical registration results, whereas  $\epsilon_{Const.}$  of 50mm produced poor registration results. The second observation was that the proposed method, which is independent of arbitrarily defined thresholds, significantly outperformed the scan vs. BIM even at the most optimum distance of  $\epsilon_{Const.} = 100mm$ . In fact, the proposed method after the first iteration produced better results than the converged scan vs. BIM after 30 iterations. The proposed method converged after only four iterations and produced a final registration error of 0.9mm, about four times better than the best scan vs. BIM, which achieved 3.2mm after 30 iterations.



**Figure 5.** Results of the point cloud to model registration error vs. iteration number for the proposed method, compared to scan vs. BIM [12] with  $\epsilon_{Const.} = 50mm, 75mm, 100mm$  and  $125mm$ .

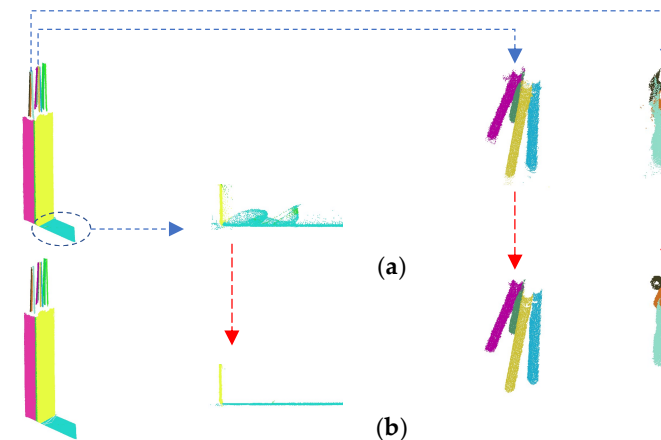
### 3.2.3. Results: Quality of Point to Surface Assignment

Given the impact of  $\epsilon_{Const.}$  on the registration errors, it is also important to evaluate the quality of point to model assignment for each method. Here, the precision, recall, accuracy and F-measure, introduced in [44] are used to define the quality of the point cloud to model assignment. Table 1 provides the summary of the assignment quality for each method. The scan vs. BIM at  $\epsilon_{Const.}$  of 50mm achieved comparatively poor results in both precision (Type I errors) and recall (Type II errors). The scan vs. BIM, however, appears to become more robust to Type I errors (i.e. the correct detection of points belonging to the surface) as the  $\epsilon_{Const.}$  increases from 50mm to 125mm. In fact, scan vs. BIM slightly outperforms our proposed method in Type I errors. Our method, however, considerably outperformed scan vs. BIM in Type II errors (i.e. the correct rejection of points not belonging to the surface). The latter is a consequence of the robust inlier detection (Appendix II), which was used within the point to surface assignment process to reject outlying points.

**Table 1.** Summary of the quality of point cloud to model assignment using different methods

Method	Precision	Recall	Accuracy	F-Measure
Scan vs. BIM -5mm	87.9%	76.9%	69.7%	82.1%
Scan vs BIM -75mm	98.8%	87.5%	86.6%	92.8%
Scan vs. BIM -100 mm	<b>100.0%</b>	87.5%	87.5%	93.3%
Scan vs. BIM -125 mm	<b>100.0%</b>	86.8%	86.8%	92.9%
Proposed- Point Cloud vs. BIM	98.8%	<b>99.9%</b>	<b>98.7%</b>	<b>99.3%</b>

\* Bolded numbers represent the best value in each column



**Figure 6.** Comparison of Type II errors during the point to model assignment: a) scan vs. BIM with  $\epsilon_{Const.} = 100mm$ ; and b) proposed point cloud vs. model.

The robustness of our proposed method to Type II errors, compared to scan vs. BIM with  $\epsilon_{Const.} = 100mm$  is visually presented in Figure 6. It can be observed that many points of both planar and cylindrical surfaces, which are within the distance threshold of  $\epsilon_{Const.} = 100mm$ , are incorrectly assigned to the surface. In our method, other than the fact that the distance threshold (error ellipsoid) is systematically defined based on the spatial uncertainty of each point, a robust surface fitting is performed to further reduce the impact of outliers and consequentially improve Type II errors.

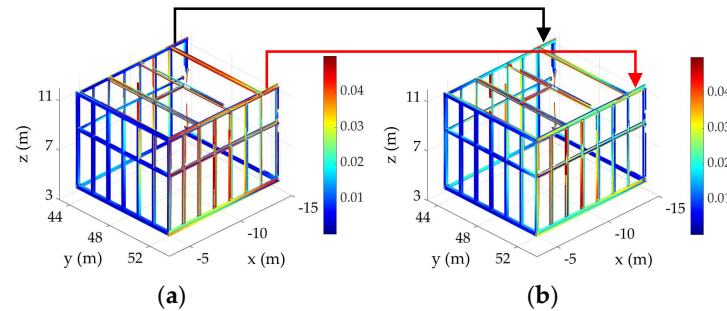
### 3.3. Constraint for Negligible Construction Error

The aforementioned methods, e.g. scan vs. BIM, which use a distance metric to assign points to their corresponding elements, can only produce reliable results when the constraint of negligible construction error is met. This is attributed to the fact that the presence



of construction errors cannot be quantified a-priori -i.e. before the point cloud is analyzed. The existence of construction errors will hinder these methods in the following two ways:

1. Distance metric: since the distance threshold is the basis for assignment or rejection of a point to an element, without a-priori knowledge of the construction errors, the threshold will be a guess at best. The impact of the subjective definition of  $\varepsilon_{const.}$  (introduced in the previous section), even for the case without construction errors, was thoroughly discussed in Section 3.2.
2. Iterative registration: since the iterative ICP registration is performed on all elements, those elements with additional construction errors might also be used within the least-squares registration. Similar to any least squares adjustment in the presence of outliers [45], the outlying elements due to construction errors will negatively impact the overall registration quality. An example of this phenomena for scan vs. BIM is provided in Figure 7. In this example, even though the overall registration RMSE improved from 21mm in Figure 7a to 18mm in Figure 7b, the registration process -as an example- sacrificed the accuracy of the element, showed with the black arrow, to accommodate the element, showed in red. However, Figure 7a represents the correct point to model distance, and, hence, the element, shown in red, must not be allowed to influence the registration. It is possible to solve this problem by adopting a local registration and matching strategy, which will be treated in Section 4.



**Figure 7.** Distance of point to 3D element model using: a) only the initial reliable target-based registration; b) the ICP fine registration [12].

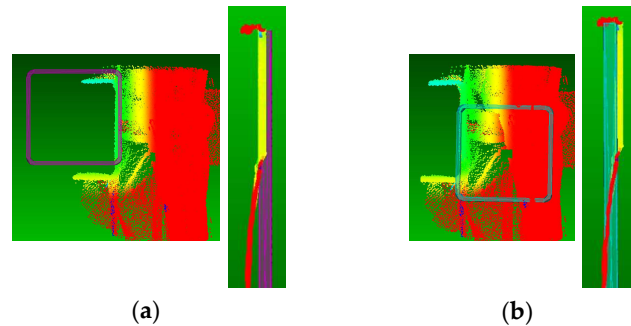
#### 4. Point Cloud Analysis: Existence of Construction Errors

Construction errors is defined, here, as the possibility of a set of model elements to be installed and built in the field incorrectly, particularly in terms of their position, orientation and/or dimensions. The problem of damaged elements, where the geometry of the element is altered (e.g. a bent rectangular column) will not be considered in this manuscript. In the existence (or speculation) of construction errors, given an initially reliable registration of the point cloud and the model, each element can be treated separately and point cloud processing can be carried out locally (unlike the global methods described in Section 3). To this end, for each element, a local set of points are first isolated [27] using a relaxed local tolerance to account for possible construction errors (say 200mm). The assignment of the isolated points to the model elements can then be treated in the following two ways (or in combination):

1. Fitting the model directly to the point cloud, which aims at finding the group of points that match the geometry of the element. This can be accomplished by means of heuristic methods such as template matching [38,39], and robust least squares adjustment [27,45,46].
2. Local point cloud to model hypothesis testing, which aims at finding the points that locally follow the pattern of the element's geometry.

The difference is that the second class of methods incorporates the local geometry of each of the isolated points to enable point to model assignment with higher confidence. This additional local information may become particularly useful when assigning non-

analytical shapes in the presence of outliers. To provide some perspective, an example of a rectangular hollow structural section (HSS), analyzed using Verity [47] -the first approach- is shown in Figure 8. As illustrated, due to the presence of outliers, the heuristic search of Verity has found a non-optimal and incorrect transformation for the HSS element (Figure 8b). This is particularly attributed to the fact that only a limited combination of points (in this case 1,000 combinations) are used, and the combination satisfying some decision criteria (e.g. highest consensus [48]) is selected. Additional combinations might improve the results, but increases the computation time without guarantee of optimality [49]. Local neighborhood information, such as curvature, may provide additional means to support the point cloud to model assignment [19]. The remainder of this section is dedicated to presenting new methods to address class 2, local point cloud to model hypothesis testing.



**Figure 8.** Results of heuristic model fitting of a hollow structural section (HSS) to contaminated point clouds using Verity: a) original registered model and point cloud; and b) result of the incorrect model fitting.

#### 4.1. Local Point Cloud to Model Hypothesis Testing

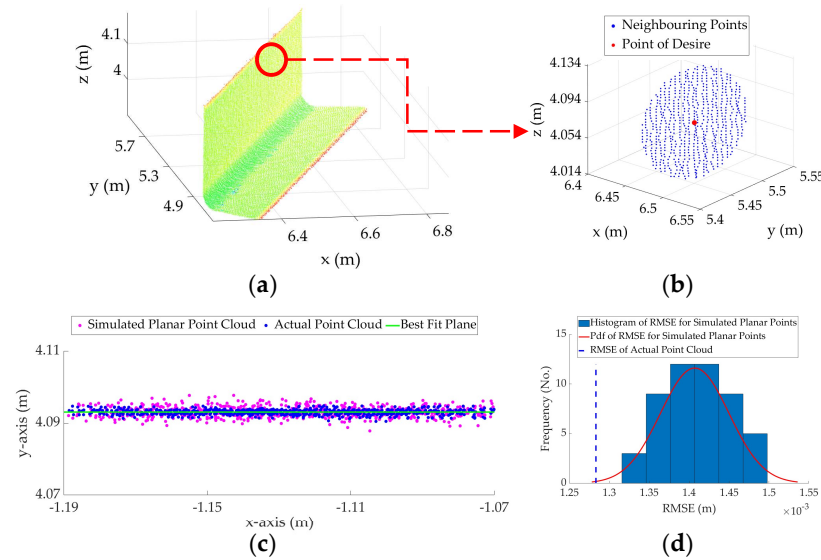
Given the geometric representation of the element in the design model, the problem is to find the sets of points which locally follow the element's geometric pattern. The problem can be solved using **Algorithm 2: Hypothesis Testing** as follows:

1. Find the local neighborhood of each point (Figures 9b and 10b).
2. For each neighborhood, estimate the best fit parameters of the element's surface geometry (e.g. best fit plane Figures 9c and 10c, cylinder Figure 10e, and etc.), and record the RMSE of the fit<sup>1</sup>,  $RMS_{Real}$ .
3. Determine the distribution of the RMSE (Figures 9d, 10d and 10f) if the points were to follow the hypothesized model as follows:
  - 3.1. Project the points onto the best fit model (perspective projection is recommended for optical instruments such as perspective cameras and TLS)
  - 3.2. Subject the projected points to additive measurement errors (Figures 9c, 10c and 10e) by simulating a point within the error ellipsoid of the original point to account for the spatial uncertainty (equation 1).
  - 3.3. Calculate the RMSE of the best fit to the points subjected to spatial uncertainties
  - 3.4. Perform steps 3.1 through 3.3  $N_T$  times to develop the distribution of the RMSE.
4. From step 3, record the maximum,  $RMS_{Max}$ , mean of the RMSE,  $RMS_{mean}$ , and standard deviation of the RMSE,  $\sigma_{std}$ , of the simulated RMSE distribution.
5. The point belongs to the surface type if and only if the following condition is met:

$$RMS_{Real} \leq \max\{RMS_{Max} | RMS_{mean} + 3\sigma_{std}\}, \quad (2)$$

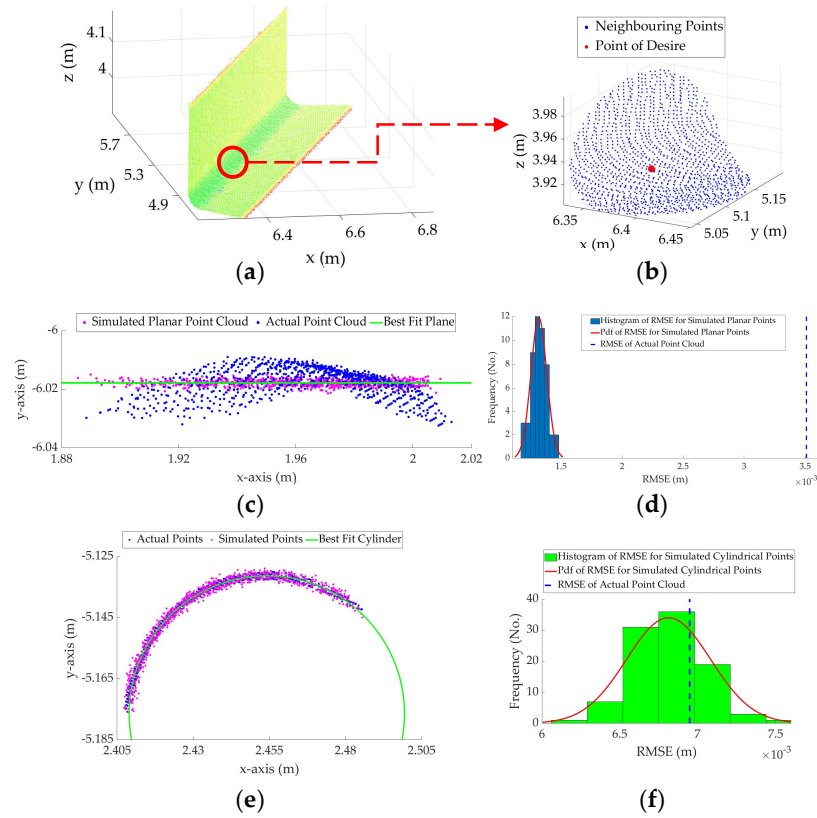
<sup>1</sup> If an element consists of more than one surface geometry (e.g. HSS, which includes both cylindrical fillets and planar surfaces), perform the remaining steps on the model which achieved the lowest RMSE.

Algorithm 2 uses the surface geometry of the element, along with the spatial uncertainty of the points to simulate a probability distribution of the RMSE if the points were to follow the hypothesized geometry. The point will be designated to the geometry if and only if its RMSE complies with that of the simulated distribution. The use of Monte-Carlo simulation, here, enables the generation of the distribution of the RMSE, which in general form, is challenging to formulate analytically (e.g. finding the distribution of smallest eigenvalue [50]). Furthermore, the distribution of the best fit parameters, such as surface normal, can also be estimated during the process, which is shown to be an asset in systematically determining the correct threshold for surface segmentation [31].



**Figure 9.** Sample of the planar hypothesis testing using Algorithm 2, applied to a neighborhood from a planar surfaces: a) point cloud; b) point cloud neighborhood of a point of interest; c) best fit plane (green), actual neighborhood points (blue), and simulated neighborhood points (magenta) - rotated such that the plane's normal vector is parallel to the y-axis-; and d) distribution of the RMSE for the simulated planar points, where the RMSE of the actual neighborhood satisfied equation 2, indicating the point follows a planar pattern.

Figure 9 shows the process, described in Algorithm 2, for a neighborhood of points following a planar pattern. It can be observed from Figure 9d that the RMSE of the original point cloud neighborhood of Figure 9b, is much smaller than even the mean of the probability distribution function of the RMSE of the simulated points, indicating that the point considered in Figure 9 satisfies equation 2 and follows a planar pattern. This is, in fact, not the case for the cylindrical neighborhood, shown in Figure 10. For the cylindrical neighborhood, if the points are projected onto the best fit plane, the RMSE of the best fit plane to the cylindrical points considerably exceeds the maximum of the probability distribution of the RMSE of the simulated planar points (Figure 10d). Alternatively, if a cylinder is fitted to the cylindrical points, the RMSE of the best fit cylinder is within the probability distribution of the RMSE of the simulated cylindrical surface, and satisfies the requirement set in equation 2 (Figure 10f). Therefore, the point considered in Figure 9 will be classified as a plane, and the point considered in Figure 10 will be classified as a cylinder.



**Figure 10.** Sample of the planar and cylindrical hypothesis testing using Algorithm 2, applied to a neighborhood from a cylindrical surfaces: a) point cloud; b) point cloud neighborhood of a point of interest; c) best fit plane (green), actual neighborhood points (blue), and simulated neighborhood points (magenta) -rotated such that the plane's normal vector is parallel to the y-axis; d) distribution of the RMSE for the simulated planar points, where the RMSE of the actual neighborhood does not satisfy equation 2, indicating the point does not follow a planar pattern; e) best fit cylinder (green), actual neighborhood points (blue), and simulated neighborhood points (magenta) -rotated such that the cylinder's axis is parallel to the z-axis; and f) distribution of the RMSE for the simulated cylindrical points, where the RMSE of the actual neighborhood satisfies equation 2, indicating the point follows a cylindrical pattern.

Algorithm 2 is non-parametric and only requires the spatial uncertainties of the points from equation 1 as well as the type of geometric surface from the element's design information model. Once the points are designated (classified) to the desired surface model (e.g. plane) using Algorithm 2, Algorithm 1 (or template matching [38]) can be utilized on only the designated points for final point to model assignment. The final consideration for Algorithm 2 is to define a reliable neighborhood around each point, and to determine the required number of Monte Carlo iterations,  $N_T$ , to form a reliable distribution of RMSE, which are discussed in the following.

#### 4.2. Robust Neighborhood Definition

The neighborhood of points can be defined as fixed [25] or variable [51,52]. For the fixed neighborhood case, either a fixed radius around the point or a fixed number of neighboring points is considered. In practical settings, since the local point cloud resolution and density vary throughout the dataset, a fixed neighborhood cannot capture the impact of the change in point density effectively. Variable neighborhood sizes are generally more effective in capturing the local point density variations and consequentially better representing the behavior of the neighborhood of the points [51,52]. The most reliable methods for adaptively selecting the neighborhood of each point [51,52] utilize basic principles from decision and information theory [53] to find the set of points achieving the

minimum information entropy. Theoretically the set of points with the smallest entropy will contain the maximum information. To this end, a range of neighborhoods are considered, and that achieving the lowest information entropy is selected [51,52]. The two methods presented in [51,52] differ in their definition of features used to formulate the entropy function with Weinmann's formulation [51], achieving slightly better results.

The problem of variable neighborhood can be formulated in robust statistical sense as finding the set of points whose determinant of the covariance matrix is minimum (MCD [45,54]), which is the set of points with the least outliers. Turns out that minimizing the determinant of the covariance matrix also minimizes the differential entropy for Gaussian distributions (equation 1 of [55]), which is also consistent with the principles of decision and information theory [53]. Hence, the optimum neighborhood can be considered as that achieving MCD. However, to compare the MCD as the neighborhood size increases, the data must be normalized, otherwise larger neighborhoods will produce greater covariance determinants. Here, the data is normalized such that the total variance, the sum of the eigenvalues of the covariance matrix, equals unity. The process is formulated using, **Algorithm 3: Robust Neighborhood Definition**, given starting radius (or number of points),  $r_{min}$ , final radius,  $r_{max}$ , and sampling step,  $\delta_r$ , for each point as follows:

1. Generate the sampling radius:  $\{r_i = r_{min} + i \cdot \delta_r \mid i = 0:1:\frac{r_{max}-r_{min}}{\delta_r}\}$
2. For each sampling radius,  $r_i$ , find the closest points to the point of desire within the spherical radius,  $r_i$ .
3. Calculate the eigenvalues of the covariance matrix of the set of points from step 2,  $\Lambda_{r_i} = (\lambda_1, \lambda_2, \lambda_3)$ .
4. Calculate the normalized MCD,  $E_{MCD}$ , as follows:

$$E_{MCD} = \prod_{k=1}^3 \frac{\lambda_k}{\lambda_1 + \lambda_2 + \lambda_3}, \quad (3)$$

5. Select the  $r_i$  which received the smallest  $E_{MCD}$ .

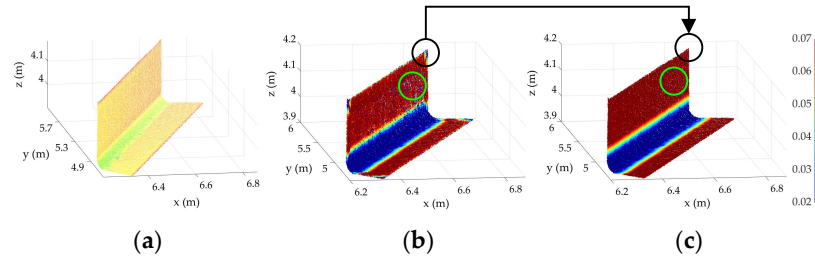
#### 4.3. Demonstration 2: Evaluation of Point to Model Quality

This sub-section involves the assessment of the effectiveness of the proposed methods on a point cloud of a sample HSS member of Figures 9a and 10a, acquired using the Leica HDS6100 TLS. The sample point cloud consists of two planar surfaces and a cylindrical fillet. Three experiments are described in the following, namely, robust neighborhood evaluation, impact of Monte Carlo iteration, and impact of neighborhood size.

##### 4.3.1. Robust Neighborhood Evaluation

The robust neighborhood definition of Algorithm 3 is compared to the reliable variable neighborhood definition of Weinmann [51] for the data presented in Figures 9a and 10a. The neighborhood radius was set to change from 20mm to 70mm in 1mm increments. The results of the defined neighborhood radius using Weinmann's [51] and ours is visually shown in Figure 11b and 11c, respectively. As visually illustrated, both methods produce similar pattern of neighborhood sizes for the planar (larger neighborhoods) and cylindrical (smaller neighborhood) regions. Our method, however, appears to be more consistent with defining the neighborhood of points for the planar and cylindrical regions. For instance, Weinmann's method finds a smaller neighborhood for the points of the same planar surface on the edges of the surface (marked in black oval), whereas ours provides a consistent neighborhood around the points of the same surface. Further observations can be made from the green ovals, which are again from the same planar surface. Overall, the accuracy of angle between the estimated normal and the ground truth normal vector was around 0.2° and 0.6°, using the proposed and Weinmann's methods.

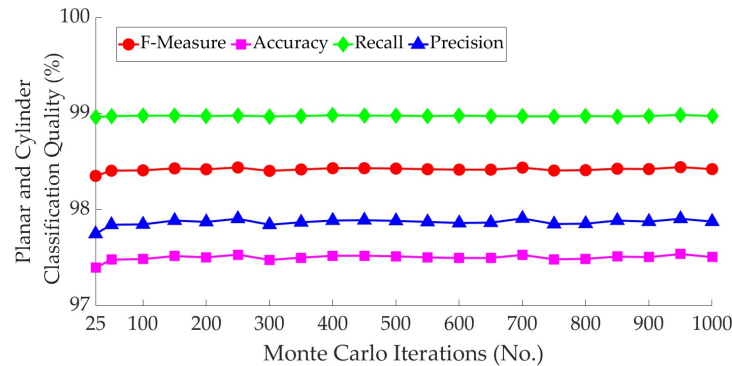




**Figure 11.** Results of the neighborhood definition: a) sample point cloud; b) Weinmann's [51] method; and c) using Algorithm 3.

#### 4.3.2. Impact of Monte Carlo Iteration

This experiment is designed to quantify the impact of the number of iterations,  $N_T$ , used to generate the probability distribution of the RMSE in Algorithm 2. To this end, Algorithm 2 in combination with Algorithm 3 was applied to the sample data with  $N_T$  increasing from 25 to 1000. The quality of the planar and cylindrical classification (precision, recall, accuracy, and F-measure) was quantified and recorded for each iteration. Figure 12 shows the quality of the classification as the number of iterations increase. It can be observed that the precision, recall, accuracy, and F-measure remain relatively constant as the number of iterations increase from 50 to 1000. The precision, and consequentially accuracy and F-measure were, however, relatively lower with 25 iterations, compared to 50 iterations. Therefore, 50 iterations appear to be an appropriate choice for the number of iterations.  $N_T = 50$  was also tested on the dataset shown in Figure 4a with similar results.



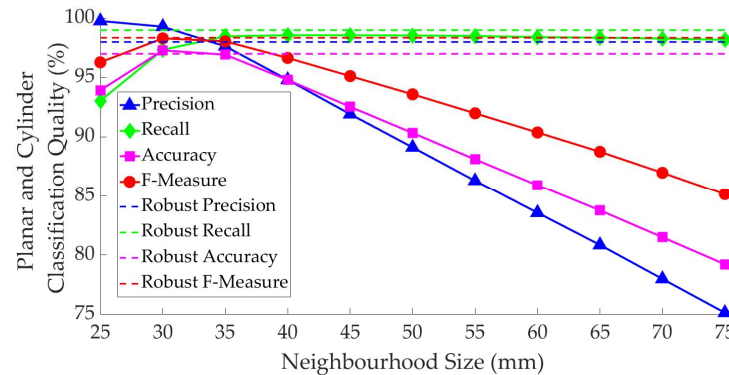
**Figure 12.** Quality of point to model designation using Algorithm 2 vs. number of iterations

#### 4.3.3. Impact of Neighborhood Size

In Section 4.3.1, it was established that the proposed neighborhood definition can provide accurate results, particularly to estimate the surface normal in comparison to an established variable neighborhood definition method. Here, the impact of fixed neighborhood sizes on the quality of the point to model designation using Algorithm 2 is evaluated. To this end, the neighborhood size is changed from 25mm to 75mm in 5mm increments, and the precision, recall, accuracy, and F-measure are recorded. Figure 13 shows the quality of planar and cylindrical classification of Algorithm 2 ( $N_T = 50$ ) using different fixed neighborhood sizes. The dashed lines show the classification quality using the robust neighborhood of Algorithm 3. As illustrated, the size of the neighborhood considerably impacts the quality of the classification. The precision appears to reduce as the neighborhood size increases. The recall, on the other hand, increases as the neighborhood size increased. Due to this counteraction between the precision and recall, the F-measure and accuracy peak at the neighborhood size of around 30mm. The robust neighborhood using Algorithm 3 provided a lower precision (indication of Type I errors), but a relatively higher recall (indication of Type II errors), compared to the neighborhood size of 30mm.

In fact, the F-measure and the recall using the robust neighborhood outperformed all neighborhood sizes of fixed radius.

The considerable impact of neighborhood size on the classification quality also demonstrates that if the data were to be isotopically scaled, the most optimum fixed neighborhood size (in this case 30mm) must also be scaled. Therefore, a fixed neighborhood cannot possibly provide the same point to model designation quality with models of different sizes and with varying point cloud densities.



**Figure 13.** Quality of point to model designation using Algorithm 2 vs. neighborhood size

## 5. Concluding Remarks and Avenues for Future Exploration

The automated analysis of point clouds, acquired from construction projects, enables frequent measurement and reporting of the project's performance, which is imperative to promote continual improvement. To this end, this study focused on the methods to assign point clouds to their corresponding elements, given a reliable designed n-D model, the spatial uncertainty of the point clouds, and a reliable registration between the model and the point cloud. The methods were further sub-categorized to address field conditions with negligible, and existence (or speculation) of construction errors.

In the case of negligible construction errors, a generic point cloud vs. model framework was proposed, which first performs an iterative global point to plane registration between the point cloud and the model. The process then assigns the points to the closest model elements, which intersect with the point's error ellipsoid (representative of spatial uncertainty). The proposed method outperformed the popular scan vs. BIM. Scan vs. BIM was also found to be considerably influenced by the choice of distance threshold.

In the existence of construction errors, it was argued that the local behavior of each point can provide additional information to aid with the point cloud to model assignment. To this end, a new generic method was proposed to designate points to classes of surface geometries of each element in the designed model. The method first simulates point clouds of the surface geometry using the spatial uncertainty of each point, and then decides if the neighborhood of the point follows the pattern of the considered class of geometry. The method is non-parametric and generic; however, it requires a process for neighborhood definition. Therefore, a new method for neighborhood definition was proposed, which is consistent with both robust statistics and information theory. The point cloud surface designation and the robust neighborhood definition was found to be promising in processing a real-world point cloud from an HSS member.

Given some of the assumptions presented in this study, the following remain possible avenues for further investigation:

1. Automated spatial uncertainty estimation: currently, the proposed frameworks are predicated on the formulation of the error ellipsoid of each point a-priori. While this can be accomplished for most instruments, it would still be attractive to formulate this spatial uncertainty automatically and directly from the point cloud.
2. Closed formulation of distribution of RMSE: Algorithm 2 involved simulating multiple sets of points to generate the distribution of the RMSE. Formulating this RMSE

in closed form will remove the requirement for simulating the multiple sets of points, which depending on the formulation, might improve computational efficiency.

3. Point cloud assignment in the presence of damages: This study focused on errors during construction, including orientational, positional, and dimensional errors of a particular element. Automatic assignment of points to damaged elements, such as cracks and deformations, will be an interesting avenue for further exploration.

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## Appendix A: Intersection of Error Ellipsoids with Common Surfaces

Strategies of finding intersection of ellipsoids with planes, cylinders, spheres, and ellipsoids are considered in the following:

1. Ellipsoid to Plane Intersection: This problem was treated in Algorithm 5 of [11]. First, the error ellipsoid is converted into a sphere with radius unity through the affine transformation, presented in Algorithm 5-step 3 of [11]. Subject the planar surface to the same affine transformation. If the distance of the center of the unit sphere to the transformed plane is less than unity, the plane and ellipsoid have an intersection.
2. Ellipsoid to Cylinder Intersection: This problem was treated in Algorithm 6 of [11]. First, the cylinder and ellipsoid are rotated such that the cylinder's axis is parallel to the z-axis. The problem is now reduced to checking the following two intersections in 2D: (i) ellipse and circle intersection in x-y plane; and (ii) ellipse and rectangle intersection in x-z (or y-z) plane. The first, ellipse and circle intersection, can be solved by calculating the distance of the center of the circle to the ellipse, using the method of Chernov [56]. If the distance is smaller than the radius of the circle (original cylinder), the two curves intersect. The second, ellipse and rectangle intersection, can be solved using the method of Eberly [57].
3. Ellipsoid to Ellipsoid Intersection: Wang [58] proposed an elegant algebraic condition for the intersection of two ellipsoids (extendable to spheres as well). It was shown that two ellipsoids are separated if and only if their characteristic polynomial contains exactly two distinct positive roots. This condition can be utilized to determine if the two ellipsoids intersect.

## Appendix B: Robust Outlier Detection

Given a best fit model to a set of points, the following robust outlier detection algorithm, first described in Algorithm 1 and Algorithm 2 of [27] for circles, is presented:

1. Perform the following concentration step:
  - 1.1. Calculate the squared residual of each point,  $r_i^2$ , to the best fit model.
  - 1.2. Estimate the mode of  $r_i^2$ ,  $Mod$ , for the intercept adjustment.
  - 1.3. Estimate the normalized median absolute deviation ( $MADN$ ; [59]) of  $r_i^2$ :
  - 1.4. Find all points satisfying the following condition:

$$\frac{|r_i^2 - Mod|}{MADN} \leq \sqrt{\chi_{0.975, \alpha'}^2} \quad (B1)$$

- 1.5. Consider the inliers as the new set of points:
  - 1.5.1. If the inlier points remain unchanged between two consecutive iterations, exit the concentration step and return the final set as the inliers.
  - 1.5.2. Else, estimate the best fit model parameters on the new points and return to step 1.1.
2. Using the best fit model from the inliers of step 1, estimate the residuals,  $Res_i$ .
3. Perform Algorithm 2 of [27] to determine all points inlier points as follows:

3.1. Calculate the sample standard deviation of the residuals,  $\sigma_f$ , using the final set of inlier points.

3.2. Find all points satisfying the following equation:

$$\frac{|Res_i|}{\sigma_f} \leq \sqrt{\chi_{0.975, \alpha'}^2} \quad (B2)$$

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