

Article

Effects of Vehicle-Induced Vibrations on the Tensile Performance of Early Age PVA-ECC

Xiaodong Zhang ¹, Shuguang Liu ^{1,2*}, Changwang Yan ^{2*}, Xiaoxiao Wang ³ and Huiwen Wang ⁴

¹ School of Materials Science and Engineering, Inner Mongolia University of Technology, Hohhot 010051, China; zhangxd8808@126.com

² School of Mining and Technology, Inner Mongolia University of Technology, Hohhot 010051, China;

³ School of Civil Engineering, Inner Mongolia University of Technology, Hohhot 010051, China; wxiaoxiao.good@163.com

⁴ Beijing Tuowei Times Architectural Design Co., Ltd., Beijing 100020, China; wanghw0717@126.com

* Correspondence: liusg6011@126.com (S.L.); ycw20031013@126.com (C.Y.)

Abstract: Polyvinyl alcohol-engineering cementitious composites (PVA-ECC) has been widely applied in bridge deck repairing or widening, the common practice for doing this is that a portion of a bridge is left open to traffic while the closed portion is constructed, which expose the early age PVA-ECC to the vehicle-induced vibrations. However, whether vehicle-induced vibrations affect the tensile performance of early age PVA-ECC remains unknown. The purpose of this study was to conduct laboratory test programs on how much vehicle-induced vibrations during early ages affected the tensile performance of PVA-ECC. A self-improved device was used to simulate the vehicle-induced vibrations, and after vibrating with the designed variables, both a uniaxial tensile test and a grey correlation analysis were performed. The results indicated that: the effects of vehicle-induced vibrations on the tensile performance of early age PVA-ECC were significant, and they generally tended to be negative in this investigation. In particular, for all of the vibrated PVA-ECC specimens, the most negative age when vibrated occurred during the period between the initial set and the final set. We concluded that although vehicle-induced vibrations during the setting periods had no substantial effects on the inherent strain-hardening characteristics of PVA-ECC, the effects should not be ignored.

Keywords: PVA-ECC; vehicle-induced vibrations; setting periods; tensile performance; grey correlation analysis

1. Introduction

PVA-ECC exhibit remarkable strain-hardening characteristics, and feature outstanding tensile ductility and energy dissipation properties with the strategically designed method of the performance driving design method (PDDA) based on micromechanics [1,2]. Their ultimate tensile strain can steadily stay above 3.0% under a uniaxial tensile load [2,3]. By reducing the brittle nature of concrete, PVA-ECC has opened a new world of possibilities to enhance the safety [4], durability [5,6], and sustainability [7,8] of civil infrastructure.

PVA-ECC has been widely applied in bridge deck pavements [9,10], overlays [11,12], and link slabs[13,14]. However, PVA-ECC does not have obvious economic advantages if it is utilized as the main structural material of a bridge. This material seems more practical and economical if it is used as part of bridge deck repairs. Completed studies have shown that PVA-ECC repairs could effectively deal with bridge deck defects [15] and greatly improve mechanical performance [13] and durability [6]. Nonetheless, the question remains, whether vehicle-induced vibrations affect the tensile performance of PVA-ECC bridge deck repairs. In particular, PVA-ECC is most vulnerable to vibrations at setting periods when vibrations might disrupt its states or physicochemical processes.

The series of physicochemical processes of PVA-ECC might be affected by vehicle-induced vibrations at different setting periods. The severe traffic pressure in modern cities increases the

possibility that vehicle-induced vibrations will take place adjacent to the setting processes of newly placed PVA-ECC. This will result the ingredients, such as the hydrates, free water, and fibers in the matrix, being subjected to a continuous disturbance of "inertial force" or a certain extent of acceleration caused by the driving force of vehicle-induced vibrations. Similar to normal concrete, early age PVA-ECC has rheological properties [16,17], and thus presents different physicochemical states at different setting periods. Therefore, it could be speculated that, a series of processes, such as the transportation of water between the pores in the matrix or the interface of the fiber/matrix, the lapping of the structural bonds among hydrates, and the connection of the solid-phase skeleton, might be affected by vehicle-induced vibrations at different setting periods ranging in time from before the initial set, to during the period between the initial set and the final set, and after the final set.

There are possible effects of vehicle-induced vibrations on PVA-ECC before the initial set. On the one hand, due to good flowability of the PVA-ECC before the initial set [18], the continuous disturbance of vibration at this period might induce a certain degree of bleeding for the PVA-ECC slurry. On the other hand, due to the hydrophilic characteristics of PVA fibers [19], the bleeding process might be accompanied by the floatation of the PVA fibers, resulting in a different distribution density for PVA fibers along the transverse section. Therefore, the tensile performance of PVA-ECC may be affected by the two possible effects before the initial set.

There are possible effects of vehicle-induced vibrations on PVA-ECC during the period between the initial set and the final set. The period during the initial set and the final set, is the key period for the growth of hydrates and the formation of microstructures [20]. If vibrated at this period, the "inertial force" will probably inhibit the agglomeration of C-S-H particles, which is the main process for low-density C-S-H gel transfers to high-density C-S-H gel [21]. Additionally, the vibration will probably damage the bonds of C-S-H gel and result in a certain degree of physical damage or obstruction to the connecting of the solid-skeleton. Moreover, due to the good water absorbability of PVA fibers [22], if they are vibrated at this period, the vibration disturbance could promote free water aggregating surround the surface of PVA fibers. This would probably make the water-cement ratio around the interface of the interface of the fibers/matrix larger than that of other parts of the matrix, and then causes bond reduction for the interface of the fibers/matrix. Furthermore, a large number of ettringite (AFt) precipitates will have formed in the matrix at this stage [23], sufficient free water will make it more conducive to the hygroscopic expansion of the AFt at the interface of the fiber/matrix [24], resulting in a larger porosity at the fiber/matrix interface, which would cause further bond reduction for the fibers/matrix interface.

There are possible effects of vehicle-induced vibrations on the PVA-ECC after the final set. After the final set, the remnants of anhydrous cement grains, gradually consumed by hydration, will be enveloped by contiguous, gradually thickening, spherical barrier shells of calcium-silicate hydrate (C-S-H) [25]. The hydration progress of cement is controlled by the transport of water from capillary pores through the barrier shells toward the interface with anhydrous cement [25]. The continuous disturbance of vibration during this period will probably promote the consumption of anhydrous cement grains, thereby increasing the degree of hydration, and then enhancing the strength of the matrix.

As commonly known, C-S-H gel is the main source for the strength of a cement matrix, and good properties of a cement matrix and the bond of a fiber/matrix interface are necessary for PVA-ECC to exhibit its excellent tensile performance. The above possible effects of vehicle-induced vibrations have a great probability to affect the tensile performance of PVA-ECC at different setting periods. Therefore, it is necessary to clarify how much vehicle-induced vibrations affect the tensile strength and deformation of PVA-ECC at different setting periods ranging in time from before the initial set, to during the initial set and the final set, and after the final set.

Since the effects of vehicle-induced vibrations on early age bridge deck repairs are critical, they have been examined in a number of experimental investigations. Thus far, most of the previous studies, have focused on how the compressive and bond strengths of "early age" concrete bridge deck repairs that are affected by vehicle-induced vibrations showed no substantial effects [26,27]. In

fact, some of the results indicated that if high quality, low slump concrete was used, both the compressive and bond strength appeared to increase slightly [28]. However, the “early age”, in most of the related studies, was actually the period after the final set of concrete bridge deck repairs.

Some of the current studies, have focused on the effects of vehicle-induced vibrations on the compressive and bond strengths of concrete bridge deck repairs at the setting periods that were before the final set. Their results showed that if the vibration occurred before the final set, there would be a considerable reduction of the compressive and bond strengths of early age bridge deck repairs [29]. In particular, studies have shown that, when subjected to vehicle-induced vibrations during the period between the initial set and the final set, concrete bridge deck repairs experienced tensile strength and elastic coefficient losses [30]. Fernandes J F pointed out that special care must be taken with regard to the tensile strength of concrete repairs because its reduction can have a negative effect on the usability of a bridge deck, and it can result in a greater deflection [30]. The results of Zhang X [31] further confirmed that both the visible cracks and internal damage of concrete repairs occurred seriously when the concrete was vibrated during the period between the initial set and the final set. He defined this period as the disturbance-sensitive period and inferred that when the concrete was at the disturbance-sensitive period, the bond strength between the cement and the aggregate was incapable of resisting vibration. Therefore, microdefects were induced by vibration that degraded the performance of concrete repairs. However, he did not provide direct evidence for this explanation. Similar conclusions were drawn by Kwan A K H [32] and Ng P L [33]. Their results showed that if a vibration disturbance started immediately when the concrete repairs were just poured, and when the vibration amplitudes caused by a moving vehicle were greater than 4.5 mm, both the bond and contraflexure strengths were significantly affected (20% reduction). Only one study showed that vehicle-induced vibrations had no substantial effects (less than 10%) on the splitting strength of concrete bridge deck repairs during various setting periods [34].

To summarize, most of the previous studies showed that when vibration occurred after the final set, the effects of vibration had no substantial effects on the strengths of concrete bridge repairs, but when vibration occurred during the period between the initial set and the final set, there was a significant negative impact.

However, prior to our work, to the best of our knowledge, few of the completed studies, paid attention to the effects of vehicle-induced vibrations on the tensile performance of early age PVA-ECC, even though it has bright prospects and it has been widely applied in bridge deck repairs. Among various vibration sources, vehicle-induced vibrations would be the most likely to cause continuous disturbance during the setting periods of PVA-ECC, and among many methods, the uniaxial tensile test might be the most convenient and effective way to identify the strain-hardening characteristics of PVA-ECC bridge deck repairs. Therefore, in order to determine how much vehicle-induced vibrations affect the tensile performance of early age PVA-ECC, we conducted a laboratory test program through a self-improved vibration device that simulated vehicle-induced vibrations. A total of 324 “dog-bone” shaped PVA-ECC specimens with sizes of 330 mm × 60 mm × 15 mm in 36 groups were cast and subjected to vibration tests up to different levels of vibration frequency combined with different lengths of vibration duration at different ages ranging in time from before the initial set, to during period between the initial set and the final set, and after the final set. After being vibrated with the designed variables, the specimens were tested with a uniaxial tensile test to determine their cracking strengths, ultimate tensile strengths, and strains.

2. Materials and Methods

2.1. Materials

The main matrix materials included PO 42.5 cement, Class-I fly ash, and high-quality silica sand. The PO 42.5 cement was manufactured by Ji Dong Cement in Hohhot, China, and its basic physical indexes and chemical composition are listed in Tables 1 and 2, respectively. The Class-I fly ash with particle sizes in the range from 0.5 μm to 2.0 μm was collected from Ordos Thermal Power Plant,

China, and its chemical composition is listed in Table 3. The silica sand with particle sizes in the range from 75 μm to 135 μm was provided by Hohhot Quartz Sand Group Co., Ltd.

The matrix additives that were used to modify the properties of the matrix included 3301E modified polycarboxylate-type superplasticizer (PCSP), JXPT-1206 high-efficiency defoamer (HED), MK-100000S viscosity modifying admixture (VMA), and K-II polyvinyl alcohol (PVA) fiber. The PCSP, produced by Dalian Sika Building Materials Co., Ltd., China, had a water-reducing efficiency of about 33.0 %. The VMA, produced by Shandong Chuangyao Biotechnology Co., Ltd., China, was added to enhance the cohesiveness and water holding capacity of the matrix. The HED, produced by Beijing Jinliangbo Technology Co., Ltd., China, was used to reduce the air content in the matrix. The physical properties of the PVA fiber, provided by Kuraray in Japan, are shown in Table 4. The volume fraction of the PVA fiber was maintained at 2.0 % in this investigation. In the mixture design, a water-to-binder ratio of 0.24 was used. The mixture proportions are listed in Table 5.

Table 1. Basic physical indexes of PO 42.5R cement

Initial setting/ Final setting (h)	Water requirement of normal consistency (%)	Flexural strength (MPa)		Compressive strength (MPa)	
1.95/2.98	26.9	3.0 days 5.8	28.0 days 8.1	3.0 days 28.9	28.0 days 47.6

Table 2. Chemical composition of PO 42.5R cement (%)

Al ₂ O ₃	SiO ₂	CaO	Fe ₂ O ₃	MgO	SO ₃	Loss on Ignition
7.19	23.44	55.01	2.96	2.24	2.87	2.86

Table 3. Chemical composition of class-I fly ash (%)

SiO ₂	Al ₂ O ₃	CaO	Fe ₂ O ₃	CO ₂	MgO	SO ₃	K ₂ O	Na ₂ O	TiO ₂	SrO	Others
40.28	18.15	18.08	8.56	5.18	2.34	2.08	1.76	1.31	0.95	0.73	0.58

Table 4. Physical properties of K-II PVA fiber

Fineness (dtex)	Density (g/cm ³)	Diameter (μm)	Elongation rate (%)	Tensile strength (MPa)	Length (mm)	Elastic modulus (GPa)
15	1.3	40	6	1600	12	40

Table 5. Mixture proportions of PVA-ECC (kg/m³)

Cement	Fly ash	Water	Silicon sand	PCSP	HED	VMA	PVA fiber
254	1016	304	457	15.24	2.60	0.64	26

2.2. Methods

2.2.1. Preparation of the specimens

The mix procedure of the PVA-ECC mixture was as follows: First, the weighed dry composition including the cement, fly ash, silicon sand, and VMA were mixed for two minutes in the cement mortar mixer. Second, the weighted PCSP and HED were dissolved into the weighted water, and then the solution was mixed into the dry mixture and mixed for 6 min in the cement mortar mixer. Finally, the PVA fibers were scattered artificially slowly into the mixture while the mixer was working and then mixed for 6 min after all of the PVA fibers were added.

After mixing, the PVA-ECC slurry was poured into the test mold layer by layer with a scoop-type tool. Thereafter, the newly poured mixture was left to stand in the indoor environment, and it was cured by covering it with plastic sheeting without any disturbance to the designed setting ages. Then the mixture was subjected to the designed vibration variables.

To meet the designed vibration variables, 333 “dog-bone” shaped specimens with sizes of 330 mm × 60 mm × 15 mm were cast in 37 groups (9 in each group), including one group of control specimens that was not subjected to vibration. After being subjected to the designed vibration variables, the PVA-ECC specimens were cured in a laboratory environment, and the uniaxial tensile test was performed 28 days after the cement-water contact. To eliminate the variation caused by the individuals, all the specimens were cast by one test operator.

2.2.2. Vibration device and variables

In order to simulate vehicle-induced vibrations, a vibration device was improved based on the cement mortar vibration table, as shown in Figure 1. The driving force of the vibration device was provided by a triphasic variable speed motor. The speed of the motor could be adjusted by the frequency converter, and the vibration frequency of the device would then be converted. To ensure that the vibration model was presented as simple harmonic vibration, two rotating bearings with circular eccentricity were added for the vibration device.

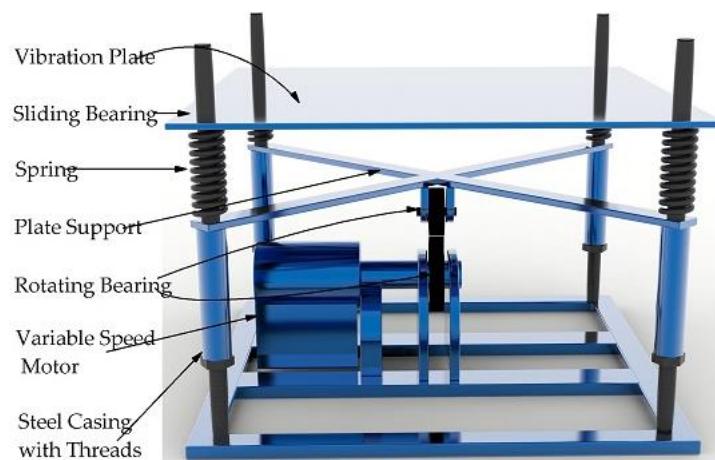


Figure 1. Self-improved vibration device based on cement mortar vibration table

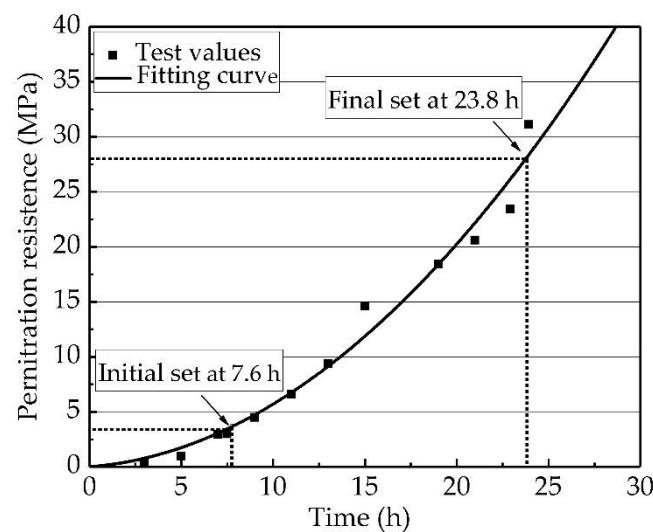


Figure 2. Results from the penetration test showing times of the initial set and the final set.

Before determining the vibration variables, the penetration test needed to be performed to determine the setting time of the PVA-ECC. The setting time of the PVA-ECC was determined based on the penetration resistance test according to JTGE30-2005. The results are shown in Figure 2. From the fitting curve of the time-penetration resistance of the PVA-ECC, the times of the initial set and the

final set were obtained, which were 7.6 h (penetration resistance \leq 3.5 MPa) and 23.8 h (penetration resistance \leq 28.0 MPa), respectively.

Three variables were considered to be of the greatest interest for the experimental program: age when vibrated, vibration frequency, and vibration duration.

The age or time when vibrated is a certain amount of setting time after water-cement contact. This variable was selected to determine the effects of vehicle-induced vibrations on the tensile performance of the PVA-ECC at different ages ranging in time from before the initial set to during the initial set and the final set and after the final set. The ages when vibrated that we chose in this investigation were 2.0 h, 8.0 h, 15.0 h, 23.0 h, 36.0 h, and 48.0 h. These six ages ensured that the PVA-ECC would be vibrated throughout the three setting periods.

The vibration frequency was selected to determine the effects of vehicle-induced vibrations on the tensile performance of the PVA-ECC under different levels of dynamics. The vibration frequencies we chose in this investigation were 2.0 Hz, 3.0 Hz, 4.0 Hz, and 5.0 Hz, and the corresponding amplitude was maintained at 5.0 mm.

The vibration duration is the time of maintaining continuous and uninterrupted vibration under the disturbance of one of the four levels of frequency. This variable was selected to explore whether the length of vibration affected the tensile performance of the PVA-ECC. The vibration durations we chose in this investigation were 2.0 h, 5.0 h, 8.0 h, and 11.0 h.

2.2.3. Uniaxial tensile test

The uniaxial tensile test was performed using an electronic universal testing machine with a range of 10 KN. The loading rate was maintained as 0.05 mm/s during loading, and the experimental data were collected by a static test analysis system.

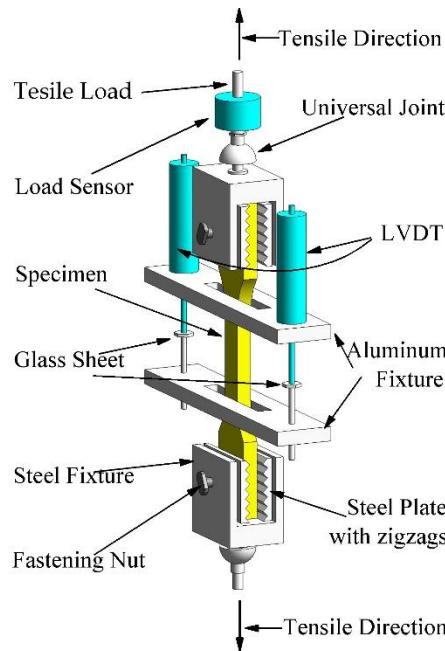


Figure 3. Schematic diagram of the loading device

The loading device was carefully designed, as shown in Figure 3. To prevent failure occurring at the ends of the specimens caused by uneven extrusion, two steel fixtures were used to clamp the ends of the specimen to produce the uniform transmission of the loads. To increase the contact area between the steel fixtures and the sides of the specimen at the ends, four steel plates with zigzag grooves were placed at the contact positions and fastened by four nuts. In order to ensure a uniaxial load and to eliminate eccentric stress for the specimen, two universal spherical hinges were installed at the top of the steel fixture, so that the geometric center line of the steel fixtures could coincide with

the axis of the specimen. Before loading, two LVDTs were fixed on the sides of the tested specimen with an aluminum fixture, and the measuring distance was maintained at 90 mm in the middle of specimen.

2.2.4. Statistical analysis

In order to minimize the discreteness of the tested data, for the 36 groups of vibrated specimens and the one group of control specimens, the Winsorized mean of the tested stress and strain values of the nine specimens were taken to represent the stress and strain values of the corresponding group. If a certain specimen among the nine specimens had an ultimate tensile strain value that was the closest to the Winsorized mean, then its stress-strain curve was taken to represent the stress-strain curve of the corresponding group.

3. Results and Discussions

3.1 Effects of age when vibrated on the tensile performance of PVA-ECC

The strain- stress curves of the twenty-four groups of vibrated specimens and the one group of control specimens at different ages when vibrated for 1.5 h, 8.0 h, 15.0 h, 23.0 h, 36.0 h, and 48.0h, subjected to the combination of a constant duration of 5.0 h and different levels of vibration frequency of 2.0 Hz, 3. 0 Hz, 40 Hz, and 5.0Hz, are shown in Figure 4. The rates of cracking strength (σ_{cc}), ultimate tensile strength (σ_{cu}), and strain (ε_{cu}) of the variated specimens over the cracking strength (σ_{vc}), ultimate tensile strength (σ_{vu}), and strain (ε_{vu}) of the control specimens with the increase of ages when vibrated are shown as Figure 5.

3.1.1. Effects of age when vibrated on the ultimate tensile strain of the PVA-ECC

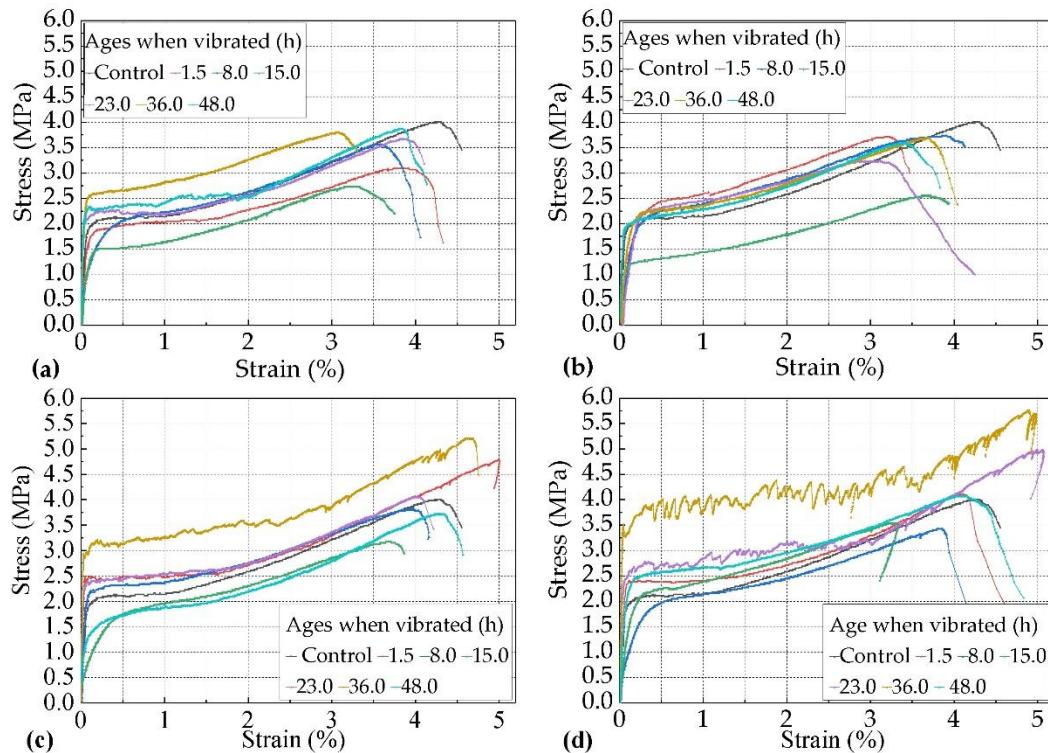


Figure 4. The strain- stress curves of the vibrated PVA-ECC groups and the control group at the ages of 1.5 h, 8.0 h, 15.0 h, 23.0 h, 36.0 h when vibrated subjected to the combination of a constant duration of 5.0 h and different levels of frequency of (a) 2.0 Hz, (b) 3. 0 Hz, (c) 4.0 Hz, and (d) 5.0Hz.

Figure 4 shows that every group of PVA-ECC specimens described in this section exhibited remarkable strain-hardening characteristics and super-high toughness. Their ultimate tensile strain

could stay steady above 3.0%, indicating that vehicle-induced vibration had no substantial effects on the strain-hardening characteristics and super-high toughness of early age PVA-ECC for this condition. Even so, according to the statistics of the twenty-four groups of vibrated specimens and one group of the control specimen, the probability of ε_{vu} being lower than ε_{cu} was approximately greater than 70 %. This indicates that the trend of vehicle-induced vibration on the ultimate tensile strain of early age PVA-ECC was negative overall.

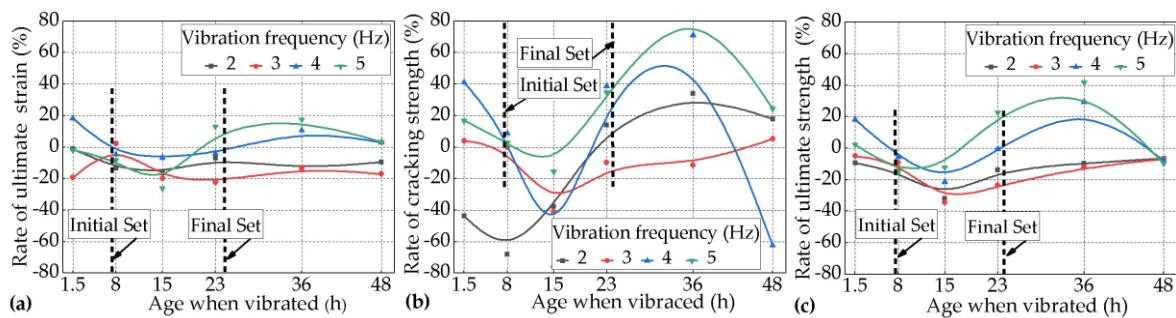


Figure 5. Rates of (a) the ultimate tensile strain, (b) the cracking strength, and (c) the ultimate tensile strength for the vibrated groups over the control group with the increase of the age when vibrated subjected to the combination of different levels of frequency and a constant duration of 5.0 h.

It can be seen from Figure 5(a) that for the PVA-ECC specimens subjected to the frequency level of 2.0 Hz, their ε_{vu} values were lower than ε_{cu} throughout the three setting periods, and the most negative effect occurred at the age of 15.0 h when the specimens were vibrated, where ε_{vu} changed by -16.55 % over ε_{cu} .

For the PVA-ECC specimens subjected to the frequency level of 3.0 Hz, their ε_{vu} changed slightly by 2.10 % over ε_{cu} at the age of 8.0 h when the specimens were vibrated, but at the rest of ages when vibrated, the corresponding ε_{vu} changed by -13.29 % to -22.38 %, and the most negative effect occurred at the age of 23.0 h when the specimens were vibrated, where ε_{vu} changed by -22.3 8% over ε_{cu} .

For the PVA-ECC specimens subjected to the frequency level of 4.0 Hz, their ε_{vu} increased by 18.18% and 10.13% over ε_{cu} at the ages of 1.5 h and 36.0 h when the specimens were vibrated, and their ε_{vu} changed by -3.96% to -6.67% during the period between ages of 1.5 h and 36.0 h when the specimens were vibrated. This indicates that the effects of vehicle-induced vibrations on the ultimate tensile strain of PVA-ECC tended to be slightly negative during the period between the initial set and the final set, and there was some extent of positive effect before the initial set or after the final set.

For the PVA-ECC specimens subjected to the frequency level of 5.0 Hz, their ε_{vu} changed approximately linearly from -1.86% to -26.11% over ε_{cu} at relatively lower ages when vibrated (1.5 h to 15.0 h), and the most negative effect occurred at the age of 15.0 h when the specimens were vibrated, where ε_{vu} changed by -26.11% over ε_{cu} . By contrast, their ε_{vu} increased within 20% at relatively greater ages (above 23.0 h). These results indicate that the effects of the vehicle-induced vibrations on the ultimate tensile strain of PVA-ECC tended to be negative at relatively earlier ages when vibrated, and it tended to be positive at relatively later ages when vibrated under the frequency level of 5.0 Hz.

To summarize, the effects of vehicle-induced vibrations on the ultimate tensile strain of almost all of the vibrated groups tended to be negative before the final set, while the effects tended to be positive to a certain extent (within 20.0 %) for some of the vibrated groups after the final set. In particular, for all of the vibrated groups, the most negative effects occurred during the period between the initial set and the final set if based on the rate of ultimate tensile strain. For these results, the following explanations can be made.

Before the initial set, vibrations would cause a certain degree of bleeding of the matrix according to normal concrete, and a certain degree of floatation of the PVA fibers that had a hydrophilic property, and this resulted in a certain extent of reduction of the ultimate tensile strain. After the final set, the hydration process of cement was controlled by the diffusion process. The vibrations promoted

the transportation of free water from the fiber/cement interface to the surface of the anhydrous cement grains, which might enhance the strength of the matrix to some extent and at the same time decreased the bond strength of the fiber/cement interface, therefore improving the tensile deformation capacity of the PVA-ECC. However, if vibrated during the period between the initial set and the final set, a series of negative effects might be caused: Inhibiting the agglomeration of the C-S-H particles, which was the crucial process to the formation of high-density C-S-H gel, damaging the bonds of C-S-H gels or obstructing them to form a solid-skeleton, and promoting free water aggregation and the convenient hydroscopic expansion of AFT around the interface of the fibers/matrix. These negative effects induced by vibrations might resulted in a reduction of the bond strength of the interface of fibers/matrix. Therefore, the most negative age when vibrated occurred during the period between the initial set and the final set if based on the rate of ultimate tensile strain.

3.1.2. Effects of the age when vibrated on the tensile strength of the PVA-ECC

It can be seen from Figure 5(a)-(c) that the impact trends of the vibrations on both the cracking strength and the ultimate tensile strength of vibrated PVA-ECC groups were approximately equal to that of ultimate tensile strain. The differences of the impact degree of vibrations on the strengths were more significant than that of ultimate tensile strain.

It can be seen from Figure 5(b) that for the vibrated PVA-ECC group subjected to the frequency level of 2.0 Hz, the most negative age when vibrated occurred at age of 8.0 h when vibrated for cracking strength. Except for this, it can be seen from Figure 5(b) and (c) that for all of the vibrated groups subjected to the frequency levels of 3.0 Hz to 5.0 Hz, the most negative ages when vibrated occurred at 15.0 h for cracking or ultimate tensile strength. This result indicates that for all of the twenty-four vibrated groups in this section, the most negative age when vibrated for the strength of the PVA-ECC occurred at the periods during the initial set and the final set.

Furthermore, it could be calculated that, for all of the vibrated PVA-ECC groups subjected to the frequency levels of 2.0 Hz to 5.0 Hz, at the most negative ages when vibrated, the cracking strength changed by -68.2 %, -41.04 %, -75.14 %, and -15.61 % over the control average, and the ultimate tensile strength changed by -31.84 %, -34.58 %, -21.39 %, and -14.93 % over the control average. This result indicates that the impacts degree of the vibrations on the cracking strength of the PVA-ECC were more significant for the combination of different levels of vibration frequency and a constant duration length of 5.0 h at the most negative age when vibrated compared with that of the ultimate tensile strength. Additionally, Figure 5 also show that the cracking strength was the most sensitive to the variables in this section, and then it followed the ultimate tensile strength and strain.

3.2. Effects of the vibration duration on the tensile performance of the PVA-ECC

The results in section 3.1 show that the most negative ages when vibrated for the tensile performance of PVA-ECC occurred during the period between the initial set and the final set. Furthermore, it was necessary to study the effects of the lengths of vibration on the tensile performance of the PVA-ECC at this stage. Therefore, the effects of the vibration duration on the lengths of 2.0 h, 5.0 h, 8.0 h, and 11.0 h on the tensile performance of the PVA-ECC under the combination of different levels of vibration frequency that were within the scope of 2.0-5.0 Hz and the age of 8.0 h when vibrated are investigated in this section. It should be noted that according to the results in section 3.1, for most of the vibrated groups, the most negative age when vibrated occurred at 15.0 h. Here, the age at 8.0 h when vibrated was selected because the corresponding specimens were vibrated only for the period during the initial set and the final set. The strain-stress curves of the sixteen groups of the vibrated PVA-ECC groups and the control group for the variables in this section are shown in Figure 6. The rates of the cracking strength, ultimate tensile strength, and strain with the increase of the length of the vibration duration under different levels of vibration frequency at 8.0 h when vibrated over the corresponding control averages are shown in Figure 7.

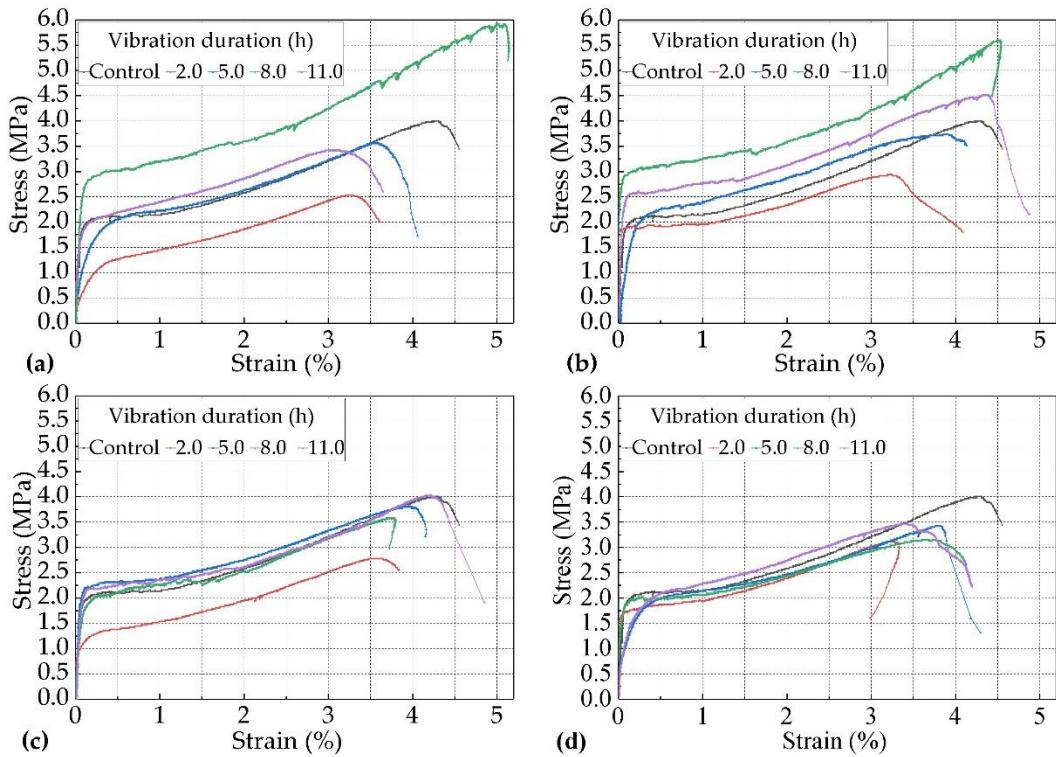


Figure 6. Strain-stress curves of the vibrated PVA-ECC groups subjected to different lengths of vibration duration under the combination of different levels of vibration frequency and the age of 8.0 h when vibrated and, (a) 2.0 Hz; (b) 3.0 Hz; (c) 4.0 Hz; (d) 5.0 Hz

3.2.1. Effects of vibration duration on the ultimate tensile strain of the PVA-ECC

Figure 6 shows that every group of PVA-ECC specimens described in this section exhibited remarkable strain-hardening characteristics and super-high toughness. Their ultimate tensile strain could stay steady above 3.0% even if the vibrations occurred during the period between the initial set and the final set with different lengths of vibration duration ranged from 2.0 h-11.0 h. Combined with the results in section 3.1, it can be further concluded that vehicle induced-vibrations had no substantial effects on the inherent tensile properties of the PVA-ECC within this investigation.

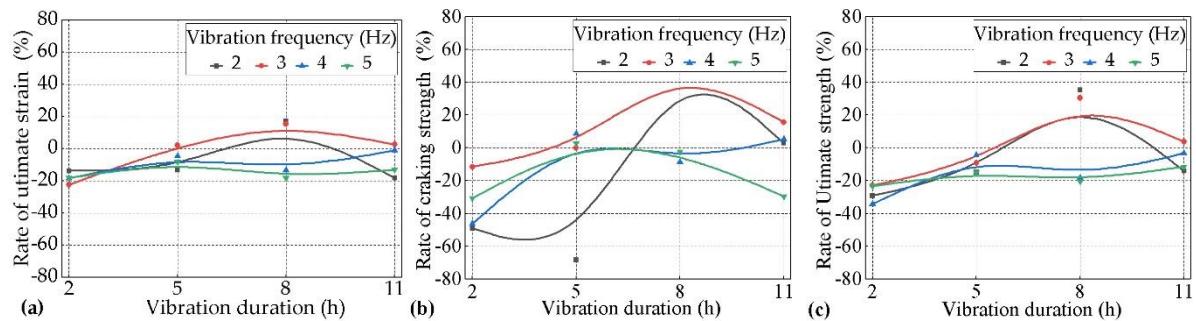


Figure 7. Rates of (a) the ultimate tensile strain, (b) cracking strength, and (c) ultimate tensile strength for the vibrated groups over the control group with the increase of the length of vibration duration under different levels of vibration frequency during the period between the initial set and the final set.

According to Figure 7 (a), for the vibrated PVA-ECC groups subjected to the frequency level of 2.0 Hz, the rate of the ultimate tensile strain over the control group first increased and then decreased with the increasing of the length of vibration duration. Figure 7 (a) also show that when the vibration duration was 5.0 h, their ε_{vu} decreased by 13.29% over ε_{cu} , and when the vibration durations were 2.0

h and 11.0 h, their ε_{vu} decreased by 13.75% and 18.18% over ε_{cu} , respectively. However, when the vibration duration was 8.0 h, their ε_{vu} increased by 17.25% over ε_{cu} .

For the vibrated PVA-ECC groups subjected to the frequency level of 3.0 Hz, the rate of the ultimate tensile strain over the control group also presented the same trend as that for 2.0 Hz. When the vibration duration was 5.0 h, their ε_{vu} slightly increased by 2.10% over ε_{cu} , and when the vibration durations were 8.0 h and 11.0 h, their ε_{vu} increased by 15.38 % and 2.56 % over ε_{cu} , respectively. However, when the vibration duration was 2.0 h, their ε_{vu} decreased by 22.38 % over ε_{cu} .

When the vibration frequency was 4.0 h or 5.0 h, the curves of the rates of the ultimate tensile strain-vibration durations were relatively smooth for 2.0 Hz or 3.0 Hz, and both of them were below the zero-axis. This result indicates that the effects of the vehicle-induced vibrations on the ultimate tensile strain of the PVA-ECC tended to be negative, but they were not obvious during the period between the initial set and the final set when subjected to relatively higher levels of vibration frequency (4.0 Hz or 5.0 Hz).

To summarize, the above results indicate that the effects of the vibration duration on the ultimate tensile strain of the PVA-ECC tended to be negative overall during the period between the initial set and the final set, but the impact trend and the degree varied for the corresponding lengths of vibration duration and levels of vibration frequency.

3.2.2. Effects of the vibration duration on the tensile strength of the PVA-ECC

It can be seen from Figure 7 that the impact trend of the vibration duration on the cracking and ultimate tensile strength of the PVA-ECC tended to be coincident with that of the ultimate tensile strain for the period during the initial set and the final set under different levels of frequency ranging from 2.0 Hz to 5.0 Hz, so this trend will not be repeated here.

The difference was that except for the individual variation group, the impact degree of the vibration duration on the strengths, including the cracking strength and the ultimate tensile strength, was greater than that of the ultimate tensile stain for the same vibration variables overall. The impact degree of the vibration duration on the cracking strength was greater than that of the ultimate tensile strength. If the defined value of the impact degree was the vibration sensitivity, then combined with section 3.1, it can be concluded that the cracking strength was the most sensitive to the variables of both the age when vibrated and the vibration duration, and then it followed the ultimate tensile strength and the ultimate tensile strain.

It can also be seen from Figure 7(b) and (c) that for all of the vibrated groups of the PVA-ECC, if they were subjected to relatively higher levels of vibration frequency (4.0 Hz or 5.0 Hz), the effects of the vehicle-induced vibrations on their strengths, including the cracking strength and the ultimate tensile strength, tended to be negative but not obvious when they were subjected to different lengths of vibration duration during the period between the initial set and the final set compared to the relatively lower levels of vibration frequency (2.0 Hz or 3.0 Hz). In addition, a similar trend was obtained to that of the ultimate tensile strain.

3.3. Effects of the vibration frequency on the tensile performance of the PVA-ECC

The results in sections 3.1 and 3.2 show that the most negative age when vibrated occurred during the period between the initial set and the final set, and at this period, the effects of the vibration duration on the tensile performance of the PVA-ECC were related to the corresponding levels of the frequency. Therefore, it was necessary to further study the effects of the vibration frequency on the tensile performance of the PVA-ECC.

The effects of the vibration frequency on the tensile performance of the PVA-ECC were studied under two kinds of variables as follows: (1) At 1.5 h, 15.0 h, 23.0 h, and 36.0 h, when vibrated with a duration of 5.0 h under different levels of vibration frequency ranging from 2.0 Hz to 5.0 Hz, referred to as Var. 1; (2) With durations of 2.0 h, 5.0 h, 8.0 h, and 11.0 h at 8.0 h when vibrated under different levels of vibration frequency ranging from 2.0 Hz to 5.0 Hz, referred to as Var. 2. The results for Var. 1 and Var. 2 are shown in Figures 8 and 9, respectively.

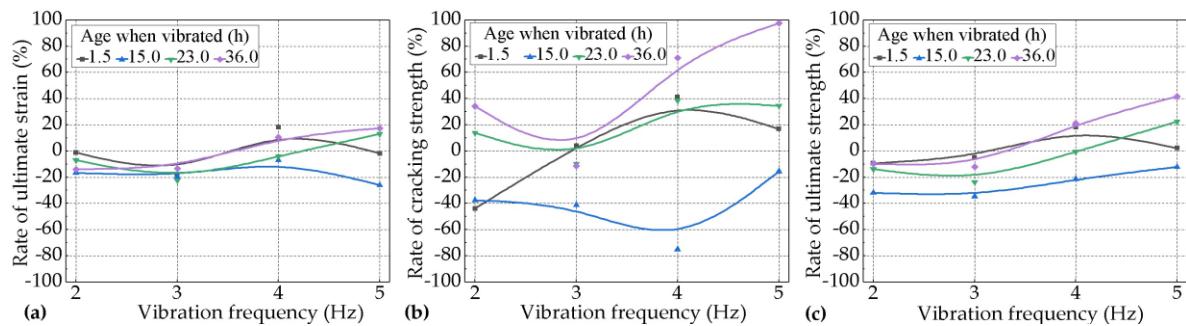


Figure 8. Rates of (a) the ultimate tensile strain, (b) cracking strength, and (c) ultimate tensile strength for the vibrated groups over the control group with the increase of the levels of vibration frequency at different ages with a duration of 5.0 h (Var. 1).

It can be seen from Figure 8 that for most of the vibrated groups of the PVA-ECC for Var. 1, the effects of vehicle-induced vibrations on the tensile performance (cracking strength, ultimate tensile strength and strain) of the PVA-ECC tended to be negative at relatively lower levels of frequency and to be positive at relatively higher levels of frequency, except for the groups that vibrated during the period between the initial set and the final set, which tended to be negative throughout the levels of frequency.

Furthermore, it can be seen from Figure 9 that for most of the vibrated groups of the PVA-ECC for Var. 2, the effects of vehicle-induced vibrations on the tensile performance of the PVA-ECC tended to be negative throughout the levels of frequency, except for the groups subjected to a duration of 8.0 h, which tended to be positive at relatively lower levels of frequency.

It can be seen from Figure 8 that for the vibrated groups of the PVA-ECC for Var. 1, the impact degrees of the vibrations on the cracking strength, ultimate tensile strength, and strain were weaker at relatively lower levels of frequency than those of relatively higher levels of frequency. While it can be seen from Figure 9 that for the vibrated groups of the PVA-ECC for Var. 2, the impact degrees of the vibrations on the cracking strength, ultimate tensile strength, and strain were greater at relatively lower levels of frequency than those of relatively higher levels of frequency.

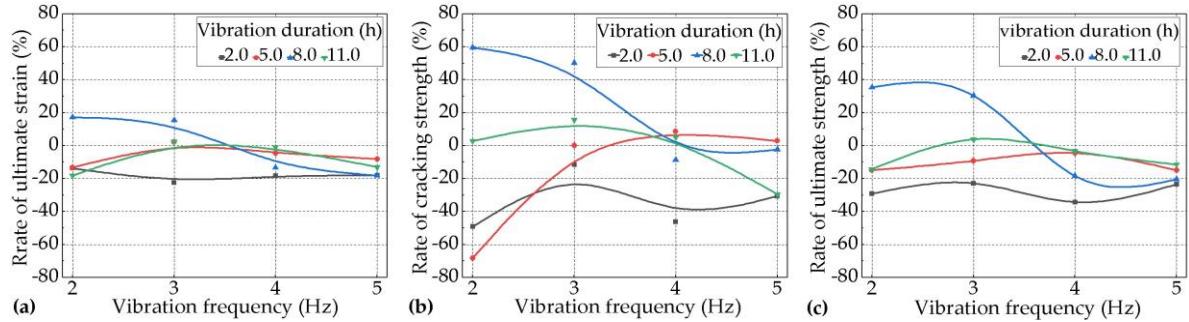


Figure 9. Rates of (a) the ultimate tensile strain, (b) cracking strength, and (c) ultimate tensile strength for the vibrated groups over the control group with the increase of the levels of vibration frequency during the period between the initial set and the final set under different lengths of durations (Var. 2).

3.4. Gray correlation analysis of the factors affecting the tensile performance of the PVA-ECC

Since Professor Deng put forward the gray system theory, gray correlation analysis has become one of the branches that is most widely used and fruitful in gray system theory, and it has been successfully applied to many areas. By comparing the geometric similarity between the curve of the comparative sequence and the reference sequence, the degree of similarity between them can be determined. A closer geometry similarity indicates a greater degree of the correlation between the comparative and the reference sequences. The main procedures are shown as follows.

First, the system behavior characteristics $X_j = \{x_j(k)\}$ were set as the reference sequences, where $k = 1, 2, \dots, n, j = 1, 2, \dots, l$. Similarly, the factors that affected the system behavior characteristics $X_i = \{x_i(k)\}$ were set as comparative sequences, where $k = 1, 2, \dots, n, i = 2, \dots, m$.

Second, the reference and comparative sequences were initialized as $x'_i(k) = x_i(k)/\bar{X}_i$ and $x'_j(k) = x_j(k)/\bar{X}_j$, respectively. Then the gray correlation coefficients $\xi_{ij}(k)$ of the reference and comparative sequences at the k moment could be expressed as

$$\xi_{ij}(k) = \frac{\min_i \min_k |x'_{0j}(k) - x'_i(k)| + \rho \max_i \max_k |x'_{0j}(k) - x'_i(k)|}{|x'_{0j}(k) - x'_i(k)| + \rho \max_i \max_k |x'_{0j}(k) - x'_i(k)|} \quad (4)$$

where $\rho \in (0,1)$ is the distinguishing coefficient, which is used to improve the significance of difference for correlation coefficients, and it is generally taken as 0.5.

Finally, the gray correlation degree $r_{ij}(k)$ of the comparative sequence X_i of the reference sequence X_j could be expressed as

$$r_{ij}(k) = \frac{1}{n} \sum_{k=1}^n \xi_{ij}(k) \quad (5)$$

For all of the 36 groups of the PVA-ECC specimens in this investigation, the corresponding cracking strength, ultimate tensile strength, and strain were taken as reference sequences and recorded successively as $X_j, j=1, 2, 3$. Three vibration factors, the age, duration, and frequency were taken as comparative sequences and recorded successively as $X_i, i=1, 2, 3$. The grey correlation degrees of the comparative sequence X_i of the reference sequence X_j were calculated as shown in Table 7.

The cracking strength, ultimate tensile strength, and strain of the control specimens were taken as reference sequences, and those of the 24 groups of vibrated specimens for Var. 1 were taken as comparative sequences. The grey correlation degrees were calculated as shown in Table 8. Similarly, the grey correlation degrees for Var. 2 were calculated as shown in Table 9.

It can be seen from Table 7 that the gray correlation degrees of the vibration factors for the tensile performance of the PVA-ECC were greater than 0.6, indicating that the changing curves of the vibration factors were highly correlated to those of the corresponding strengths and strains of PVA-ECC. In other words, the vibration factors had closer geometry similarity to the tensile performance of PVA-ECC. The gray correlation degrees of the ages for the tensile performance of PVA-ECC were the lowest compared to those of the durations and frequencies, which means that the tensile performance of the PVA-ECC was more sensitive to ages when vibrated.

Table 7. Gray correlation degrees of the vibration factors on the tensile performance of PVA-ECC

Gray correlation degree r_{ij}	Age r_{1j}	Duration r_{2j}	Frequency r_{3j}
Cracking strength r_{i1}	0.6819	0.9512	0.9607
Ultimate tensile strength r_{i2}	0.6674	0.9810	0.9549
Ultimate tensile strain r_{i3}	0.6668	0.9800	0.9513

Table 8. Gray correlation degrees of the ages when vibrated on the tensile performance of PVA-ECC with a constant of duration of 5.0 h.

Gray correlation degree r_{ij}	1.5h r_{ij}	15.0h r_{2j}	23.0h r_{3j}	36.0h r_{4j}
Cracking strength r'_{i1}	0.6514	0.6242	0.7045	0.6539
Ultimate tensile strength r'_{i2}	0.6620	0.5764	0.5264	0.8298
Ultimate tensile strain r'_{i3}	0.6514	0.6242	0.7045	0.6539

Table 9. Gray correlation degrees of the durations of vibration on the tensile performance of PVA-ECC during the period between the initial set and the final set.

Gray correlation degree r_{ij}''	2.0h r_{ij}''	5.0h r_{2j}''	8.0h r_{3j}''	11.0h r_{4j}''
Cracking strength r_{i1}''	0.7237	0.6110	0.6463	0.8182
Ultimate tensile strength r_{i2}''	0.7861	0.8450	0.3946	0.6928
Ultimate tensile strain r_{i3}''	0.8105	0.6815	0.3676	0.5182

4. Conclusions

A self-improved vibration device was made that simulated the vehicle-induced vibrations and accounted for different vibration variables including the age when vibrated, vibration duration, and vibration frequency that early age PVA-ECC bridge repairs would be subjected to with great possibility. An experimental program was followed to investigate the effects of vehicle-induced vibrations on the tensile performance of early age PVA-ECC. A total of 324 PVA-ECC specimens in 36 groups were cast and subjected to vibration tests up to different levels of vibration frequency combined with different lengths of vibration durations at different ages when vibrated ranging in time from before the initial set, to during the period between the initial set and the final set, and after the final set. After the vibration tests, the specimens were tested with a uniaxial tensile test to determine their cracking strengths, ultimate tensile strengths, and strains. According to the results obtained, the following conclusions were drawn.

(1) The effects of vehicle-induced vibrations on the tensile performance including the cracking strength, ultimate tensile strength, and strain of almost all of the vibrated PVA-ECC groups that were subjected to a constant vibration duration of 5.0 h tended to be negative before final setting. However, the effects were positive to a certain extent for some of them after the final set. In particular, the most negative ages when vibrated occurred during the period between the initial set and the final set.

(2) During the period between the initial set and the final set, the effects of different lengths of the vibration duration on the tensile performance of the PVA-ECC tended to be negative overall, but the impact trend and the degree varied for the corresponding lengths and levels of vibration. The cracking strength, ultimate tensile strength, and strain tended to be negative if the corresponding groups were subjected to relatively greater levels of frequency and shorter lengths of duration, and they tended to be positive to a certain extent for individual groups if the corresponding groups were subjected to relatively lower levels of frequency and longer lengths of duration.

(3) The cracking strength was the most sensitive to the variables of both the age when vibrated and the vibration duration in this investigation, and then it followed the ultimate tensile strength and the ultimate tensile strain.

(4) The grey correlation analysis was applicable in analyzing the effects of vehicle-induced vibrations on the tensile performance of early age PVA-ECC.

Author Contributions: Conceptualization, C.Y., and S.L.; Methodology, X.Z., X.W., and H.W.; project administration, S.L., and H.W.; writing-original draft preparation, X.Z. Writing-review and editing, S.L., C.Y., and X.W.

Funding: This research was funded by the National Natural Science Foundation of China, grant numbers 51368041 and 51768051, the Inner Mongolia Natural Science Foundation, grant number 2017MS0505, and the Inner Mongolia Science and Technology Innovation Guidance Project, grant number KCBJ2018016.

Acknowledgments: The authors are grateful for the Mechanical Laboratory of College of Science, Inner Mongolia University of Technology, for supplying the mechanical testing system used in this work.

Conflicts of Interest: The authors declare no conflict of interest.

References

1. Mihai, I.C.; Jefferson, A.D. A micromechanics based constitutive model for fibre reinforced cementitious composites. *International Journal of Solids & Structures* **2017**, *s* 110–111, 152-169, doi:10.1016/j.ijsolstr.2017.01.032.
2. Lu, C.; Leung, C.K.Y. Effect of fiber content variation on the strength of the weakest section in Strain Hardening Cementitious Composites (SHCC). *Construction & Building Materials* **2017**, *141*, 253-258, doi:10.1016/j.conbuildmat.2017.03.020.
3. Yang, E.H.; Li, V.C. Strain-rate effects on the tensile behavior of strain-hardening cementitious composites. *Construction And Building Materials* **2014**, *52*, 96-104, doi:10.1016/j.conbuildmat.2013.11.013.
4. Li, V.C. Integrated structures and materials design. *Materials And Structures* **2007**, *40*, 387-396, doi:10.1617/s11527-006-9146-4.
5. Zhang, P.; Wittmann, F.H.; Zhang, S.L.; Muller, H.S.; Zhao, T.J. Self-healing of Cracks in Strain Hardening Cementitious Composites Under Different Environmental Conditions. *Rilem Bookser* **2018**, *15*, 600-607, doi:10.1007/978-94-024-1194-2_69.
6. Muzenski, S.; Flores-Vivian, I.; Sobolev, K. Hydrophobic engineered cementitious composites for highway applications. *Cement Concrete Comp* **2015**, *57*, 68-74, doi:10.1016/j.cemconcomp.2014.12.009.
7. Yang, G.; Yu, J.T.; Luo, Y. Development and Mechanical Performance of Fire-Resistive Engineered Cementitious Composites. *J Mater Civil Eng* **2019**, *31*, doi:10.1061/(Asce)Mt.1943-5533.0002666.
8. Lim, I.; Chern, J.C.; Liu, T.; Chan, Y.W. Effect Of Ground Granulated Blast Furnace Slag on Mechanical Behavior Of Pva-Ecc. *J Mar Sci Tech-Taiw* **2012**, *20*, 319-324.
9. Wu, C.; Pan, Y.; Ueda, T. Characterization of the abrasion resistance and the acoustic wave attenuation of the engineered cementitious composites for runway pavement. *Construction And Building Materials* **2018**, *174*, 537-546, doi:10.1016/j.conbuildmat.2018.04.152.
10. Zhang, Z.G.; Zhang, Q.; Qian, S.Z.; Li, V.C. On the use of low-cost PVA fiber to develop ECC material for pavement use. *Functional Pavement Design* **2016**, 150-150.
11. Sahmaran, M.; Yucel, H.E.; Yildirim, G.; Al-Emam, M.; Lachemi, M. Investigation of the Bond between Concrete Substrate and ECC Overlays. *J Mater Civil Eng* **2014**, *26*, 167-174, doi:10.1061/(Asce)Mt.1943-5533.0000805.
12. ChiaHwan, Y.; Wei, L. Application of Interfacial Propagation and Kinking Crack Concept to ECC/Concrete Overlay Repair System. *Adv Mater Sci Eng* **2014**, doi: 10.1155/2014/623467.
13. Hou, M.J.; Hu, K.X.; Yu, J.T.; Dong, S.W.; Xu, S.L. Experimental study on ultra-high ductility cementitious composites applied to link slabs for jointless bridge decks. *Composite Structures* **2018**, *204*, 167-177, doi:10.1016/j.compstruct.2018.07.067.
14. Lepech, M.D.; Li, V.C. Application of ECC for bridge deck link slabs. *Materials And Structures* **2009**, *42*, 1185-1195, doi:10.1617/s11527-009-9544-5.
15. Van den Heede, P.; De Belie, N.; Pittau, F.; Habert, G.; Mignon, A. Life cycle assessment of Self-Healing Engineered Cementitious Composite (SH-ECC) used for the rehabilitation of bridges. *Life-Cycle Analysis And Assessment In Civil Engineering: Towards an Integrated Vision* **2019**, 2269-2275.
16. Cao, M.L.; Li, L.; Xu, L. Relations between rheological and mechanical properties of fiber reinforced mortar. *Comput Concrete* **2017**, *20*, 449-459, doi:10.12989/cac.2017.20.4.449.
17. Li, M.; Li, V.C. Rheology, fiber dispersion, and robust properties of Engineered Cementitious Composites. *Materials And Structures* **2013**, *46*, 405-420, doi:10.1617/s11527-012-9909-z.

18. Felekoglu, B.; Tosun-Felekoglu, K.; Ranade, R.; Zhang, Q.; Li, V.C. Influence of matrix flowability, fiber mixing procedure, and curing conditions on the mechanical performance of HTPP-ECC. *Compos Part B-Eng* **2014**, *60*, 359-370, doi:10.1016/j.compositesb.2013.12.076.
19. Georgiou, A.V.; Pantazopoulou, S.J. Effect of fiber length and surface characteristics on the mechanical properties of cementitious composites. *Construction And Building Materials* **2016**, *125*, 1216-1228, doi:10.1016/j.conbuildmat.2016.09.009.
20. Wang, Z.C.; Wang, Z.C.; Xia, H.C.; Wang, H.F. Analysis of Hydration Mechanism and Microstructure of Composite Cementitious Materials for Filling Mining. *J Wuhan Univ Technol* **2017**, *32*, 910-913.
21. Chen, J.; Li, Z.J.; Jin, X.Y. Morphological effect of C-S-H gel on the elastic behaviour of early-age cement-based materials. *Mag Concrete Res* **2017**, *69*, 629-640.
22. Lu, Z.Y.; Hanif, A.; Lu, C.; Sun, G.X.; Cheng, Y.; Li, Z.J. Thermal, mechanical, and surface properties of poly(vinyl alcohol) (PVA) polymer modified cementitious composites for sustainable development. *J Appl Polym Sci* **2018**, *135*, doi: 10.1002/app.46177.
23. Salvador, R.P.; Cavalaro, S.H.P.; Segura, I.; Hernandez, M.G.; Ranz, J.; de Figueiredo, A.D. Relation between ultrasound measurements and phase evolution in accelerated cementitious matrices. *Mater Design* **2017**, *113*, 341-352.
24. Nocun-Wczelik, W.; Bochenek, A.; Migdal, M. Calorimetry and other methods in the studies of expansive cement hydrating mixtures. *J Therm Anal Calorim* **2012**, *109*, 529-535.
25. Rahimi-Aghdam, S.; Bazant, Z.P.; Qomi, M.J.A. Cement hydration from hours to centuries controlled by diffusion through barrier shells of C-S-H. *J Mech Phys Solids* **2017**, *99*, 211-224.
26. Weatherer, P.J.; Fargier-Galbadon, L.B.; Hedegaard, B.D.; Parra-Montesinos, G.J. Behavior of Longitudinal Joints in Staged Concrete Bridge Decks Subject to Displacements during Curing. *J Bridge Eng* **2019**, *24*, doi: 10.1061/(Asce)Be.1943-5592.0001433.
27. Wang, W.; Liu, S.; Wang, Q.Z.; Yuan, W.; Chen, M.Z.; Hao, X.T.; Ma, S.; Liang, X.Y. The Impact of Traffic-Induced Bridge Vibration on Rapid Repairing High-Performance Concrete for Bridge Deck Pavement Repairs. *Adv Mater Sci Eng* **2014**, doi: 10.1155/2014/632051.
28. Harsh, S.; Darwin, D. Traffic-induced vibrations on bridge deck repairs. *NCHRP Synthesis*: No.86, **1981**.
29. Hong, S.; Park, S.K. Effect of vehicle-induced vibrations on early-age concrete during bridge widening. *Construction and Building Materials* **2015**, *77*, 179-186, doi: 10.1016/j.conbuildmat.2014.12.043.
30. Fernandes, J.; Bittencourt, T.; *Concrete subjected to vibrations in early-ages*; 2011; Vol. 4, pp. 592-600.
31. Zhang, X.; Zhao, M.; Zhang, Y.J.; Lei, Z.; Zhang, Y.R. Effect of vehicle-bridge interaction vibration on young concrete. *KSCE Journal of Civil Engineering* **2015**, *19*, 151-157, doi: 10.1007/s12205-014-0509-z.
32. Kwan, A.K.H.; Ng, P.L. Effects of traffic vibration on curing concrete stitch: Part I-test method and control program. *Engineering Structures* **2007**, *29*, 2871-2880, doi: 10.1016/j.engstruct.2007.01.029.
33. Ng, P.L.; Kwan, A.K.H. Effects of traffic vibration on curing concrete stitch: Part II-cracking, debonding and strength reduction. *Engineering Structures* **2007**, *29*, 2881-2892, doi: 10.1016/j.engstruct.2007.01.033.
34. Jianjun, W.; Xing. Effect of traffic load induced bridge vibrations on concrete tensile properties. *Journal of Southeast University (Natural Science Edition)* **2010**.