

Microstructural Modeling of Glass Cullet Reaction in Cementitious Systems

Mohammadreza Mirzahosseini¹

Abstract

Finely ground glass has the potential for pozzolanic reactivity and can serve as a supplementary cementitious material (SCM). Glass reaction kinetics depends on both temperature and glass composition. Microstructural modeling is a helpful approach to get better understanding of cement hydration and microstructure development. Mechanical and performance properties of concrete are directly related to the development of concrete microstructure, which is the consequence of progress in cement hydration. This study initially provides a comprehensive background about cement hydration process and microstructural modeling of the hydration. It then utilizes results of experimental studies, i.e. isothermal calorimetry and thermogravimetric analysis, to find kinetics equation parameters called “Avrami Constants”. For the first time, these constants were found for three main components of cement, i.e. C_3S , C_2S , and C_3A , and also for glass particles smaller than 25 μm . Although modeling of cement hydration and cementitious systems containing single glass particles showed promising results, simulations of combined glass types and sizes showed that more work on microstructural models is needed to properly model the reactivity of mixed glass particle systems.

Keywords: Microstructural modeling, cement hydration, Avrami constants, hydration kinetics, glass cullet, supplementary cementitious material (SCM)

¹ Post-Doctoral Researcher at Purdue University, West Lafayette, IN

The \$ is used in cement chemist notation to denote sulfate. The hydration of C_3S and C_2S produce calcium silicate hydrate (C-S-H) and calcium hydroxide (CH).

During these reactions, each of these four phases releases some heat, making hydration an exothermal chemical reaction [4]. Monitoring and measuring the amount of heat evolved during cement hydration can provide valuable information to investigate mechanical and performance properties of concrete. Figure 1 shows the isothermal heat of hydration (HOH) of a portland cement. There are five distinct reaction stages shown in Figure 1: dissolution, induction, acceleration, deceleration, and the transition/diffusion-controlled stage [4]. Comprehensive explanations of these stages are available in the literatures [4,5,6-28].

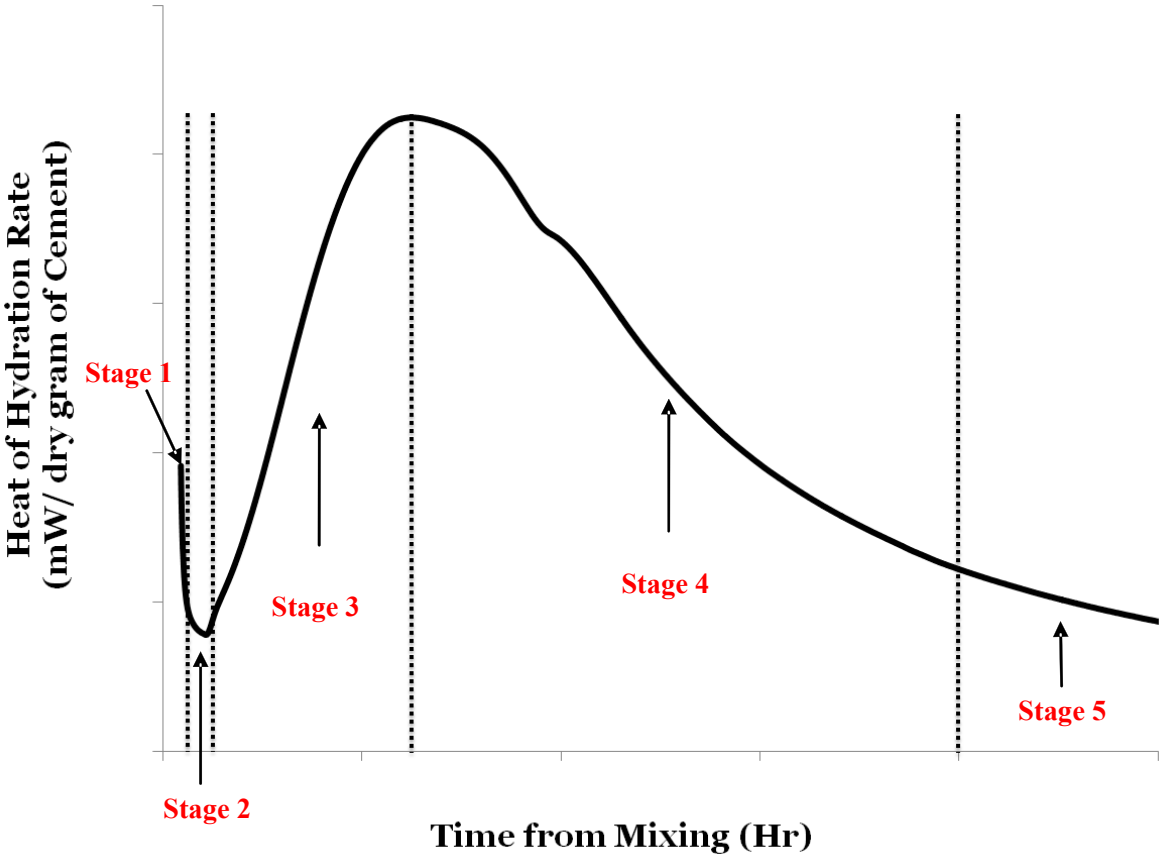


Figure 1. Hydration heat curve of portland cement paste

Factors Affecting Hydration Rate

Hydration rate varies for different cementitious materials, mixture properties, and conditions. Chemical compositions of cementitious materials, w/cm, applied pressure, particle

sizes of cementitious materials, and curing temperature can all affect the rate of hydration of cementitious materials [3]. In the case of portland cement, different amounts of C₃S, C₂S, C₃A, and C₄AF result in different hydration heat (Table 2). C₃S and C₃A have the highest rate of reaction and produce the greatest amount of heat per unit quantity in the cement [29].

Table 2. Amount of heat produced per gram of each phase

Phases	Hydration heat (J/g)
C ₃ S	500
C ₂ S	260
C ₃ A	866
C ₄ AF	420

Another important factor is w/cm ratio which not only affects the hydration rate, but also influences the degree of hydration (DOH) and strength gain of concrete. Higher w/cm ratios enhance the cement rate of hydration during the acceleration period [30,31]. Particle size plays an important role in hydration rate. As general rule, smaller particle sizes of cementitious systems and finer cement can increase the rate of hydration [29,32-34]. Finer cementitious materials have higher specific surface providing more available area to water and causing higher hydration rate. Higher surface area of cementitious system has been found to produce thinner hydration product resulting in higher final degree of hydration [3]. Influence of curing temperatures of the reaction rate of cementitious materials can be surveyed from two aspects. First of all, elevated curing temperatures can enhance the rate of hydration [35,36]. However, higher curing temperatures can cause rapid formation of hydration products which means that the hydration gradually shifts to a diffusion-controlled mechanism. This phenomenon makes it hard for water to access unreacted cementitious materials and results in a lower rate of reaction at later ages and lower ultimate hydration degree [3,37].

Supplementary cementitious Materials (SCM)

Cement production is an energy-intensive process and responsible for 5-8% of global man-made CO₂ emissions [38]. One of the most effective ways to reduce greenhouse gas emission and consumed energy from the cement industry is to partially substitute cement by other siliceous and aluminosiliceous material, known as Supplementary Cementitious Materials (SCM) [5,38]. SCMs can improve the properties of hardened concrete containing SCMs through the pozzolanic reaction. The pozzolanic reaction occurs when the CH reacts with amorphous silica of SCMs and water to create more C-S-H gel. A typical form of the pozzolanic reaction can be expressed as Eq. (5):



From a mechanical point of view, SCMs can increase the concrete ultimate compressive strength. Lothenbach et al. [39] have shown that SCMs can improve concrete microstructure through changes in C-S-H composition and changes in the porosity. The changes in the C-S-H composition however depend on the SCM composition. One material which has been studied for potential use as an SCM in concrete is waste glass cullet.

Glass Cullet in Concrete

Millions of tons of glass cullet are either landfilled or recycled throughout the world every year [40]. However, landfilling has economic and environmental issues such as limited capacity [41]. Additionally, recycling has also drawbacks such as not being able to recycle mixed color glass [42]. As the economic and environmental consequences of landfilling and recycling rise, the incentive to reuse glass cullet has grown. The concrete industry is one of the potential ways of reusing millions of tons of glass cullet per year either as aggregate or SCM [43]. Several studies have shown that glass behaves pozzolanically if ground finely enough, with a surface area of more than 300 m²/kg [44-50]. Most studies on the effect of glass cullet on cementitious mixtures as an

SCM focused on mechanical and durability properties [51-54]. There are few studies which have aimed to connect the microstructural properties of cementitious mixture containing glass powder to performance characteristics of the glass mixtures. Federico [55] performed an extensive study on the influence of glass powder on reaction kinetics and performance properties of cementitious mixtures.

Introduction to Microstructural Modeling of Hydration Process

Microstructural modeling is a helpful tool to obtain better understanding of cement hydration and microstructure development [6]. Mechanical and performance properties of concrete are directly related to the development of concrete microstructure, which is the consequence of progress in cement hydration [6,56]. Cement hydration is a complicated system making hydration difficult to model [57]. This complexity is the main reason for which there has not been developed a complete theory explaining cement hydration and chemical reaction, despite almost 200 years having passed from invention of cement [58]. Nevertheless, many efforts have been made during the past 40 years to microstructurally model hydration of cement and various cementitious materials such as fly ash, slag, and SF [57-63]. For the first time, this study has used a recently-developed modeling platform called “*μic the model*” to model kinetics reactions of three main phases of cement, i.e. C_3S , C_2S , and C_3A , and to simulated pozzolanic reactivity of single type and fraction size and combined types and particle sizes of finely ground glass cullet.

Background of Microstructural Modeling

Single Particle Model

The Single Particle Model was developed based on growing hydration products in layers on single spherical particles by Kondo and Kodama in 1967 [64]. This model suggested that the

first layer of hydration products creates a protective layer, making dissolution harder and ending the induction period. This layer is then consumed and acceleration period begins. Clifton et al. [65] proposed a diffusion-based single particle model for C_3S which has similar fundamentals to the Kondo and Kodama's model [64]; but has stronger mathematical bases. The strong point of this model is its ability to account for the continuous integrity of products layers through the boundaries. In addition to the mathematical models, some single-particle-based empirical models have been developed. Parrot and Killoh [66] performed an X-ray diffraction (XRD) analysis to extract a dissolution model considering cement types and sizes, w/cm, and relative humidity (RH). Tomosawa [67] proposed an empirical model which is similar to Parro and Killoh's [66], and takes into account the effect of fineness of cement particles and w/cm on cement reaction kinetics. Both of these empirical models are effective with spherical shapes and easy to be executed. However, these empirical models are just valid for the property ranges used to develop the models and need to be calibrated for other materials properties. The major drawbacks of the single particle model are that the models are not able to consider interaction between particles and cannot evaluate total cement reaction kinetic for different size ranges.

Nucleation and Growth Models

C-S-H nucleation and growth are consider and modeled as one process using Nucleation and Growth models, despite they are two different mechanisms. The first type of Nucleation and Growth model is *Early Nucleation and Growth* considering two main cases: site saturation and continuous nucleation [64]. Site saturation happens when nucleation is very quick at the beginning of hydration but suddenly stops. Continuous nucleation occurs when nucleation sites are not fully consumed. The simplest and the most widely used nucleation and growth model is *Johnson and Mehl, Avrami, and Kolmogorov (JMAK)* model typically used for C_3S modeling. This model utilizes mathematical rules to explain hydration products overlapping. The JMAK model however,

is not capable of providing physically meaningful parameters, is just valid in isothermal conditions, and is not able to take into account the impact of cement surface area as an important criterion of particle sizes on reaction rate. Regardless of these limitations, many researchers have implemented the JMAK model to study different aspects of cement hydration. The first application of the JMAK model is dated back to 40 years ago when Tenatousse and de Donder [68] used the model to find out that the nucleation and growth process is not limited to the acceleration period and can be considered as a contributing process during the deceleration period. Models proposed by Brown et al. [69] and Gartner and Gaidis [70] are the other examples of using JMAK model. The model by Brown et al. did not show conclusive results. The model by Gartner and Gaidis tried to cast doubt on spatial nucleation hypothesis in the JMAK model but it was not accepted. One of the other approaches in nucleation and growth modeling is the *Mathematical Boundary Nucleation and Growth (BNG)* model originally developed by Chan in 1956 [71]. This model assumes that C-S-H nucleation occurs merely on arbitrarily oriented and dispersed planar borders. A recent study [72] showed that BNG models can deliver more significant and realistic results compared to the JMAK models. On the other hand, the BNG model is just an estimation which means that the exact boundary condition would not be evaluated and hydration of C_3S is only accounted for. Additionally, the BNG model is developed for a fixed surface area which is not true in real world.

Hydration Simulation Model

It should be noted that this type of models has a significant difference with those mentioned above. The single particle and nucleation and growth models are mathematical models based on scientific theory, whereas simulation models are the visualized applications of those principals. Currently, advances in computer technologies have paved the way for researchers to study complicated hydration of cementitious materials accurately and in more details [64]. The first simulation model was developed by Frohnsdorff et al. [73]. Although this model did not broad

169 application until next 20 years, it could be fairly successful in simulating hydration kinetics and
170 formation of microstructure. The first published simulation model was in 1986 called the *Jennings*
171 *and Johnson Microstructure Simulation* model. They developed a 3D platform which utilized an
172 off-Lattice (Vector) approach to simulated cement hydration. Off-Lattice is a method of presenting
173 different shapes using their properties. Cement particles were simulated by spherical particles
174 randomly distributed in the paste cube. Hydration was also simulated through the decrease in
175 radius of reactant particles as hydration progresses, and an increase in thickness of hydration
176 products on the reactants' surface. This proposed simulation model was capable of taking into
177 account many complicated mechanisms such as different particle sizes, overlapping phenomenon,
178 and position and quantity of CH crystals. However, the model had restricted computational
179 abilities making it not broadly advanced and implemented. Another simulation model developed
180 is the *HymoStruc (HYdration, Morphology, and STRUCtural development)* model developed by
181 Van Breugel [74]. This model utilizes a 3D platform for modeling, is traceable from computational
182 point of view, and uses the same principal as Jennings and Johnson's for cement hydration. Though
183 the model had many shortcomings such as the model was able to simulated just one product, did
184 not explain the influence of pore solution, did not calculate overlapping phenomenon, and the
185 reaction rate was a function of particle size only. One of the fairly successful simulation models
186 was *CEMHYD3D Digital Hydration* model developed by Bentz and Garboczi [75]. This model
187 uses a 3D lattice-based platform on digital images. The discrete element approach was
188 implemented in this model. The model operates quickly, and is able to simulate non-spherical
189 cement particles. Additionally, the simulation model incorporates a broad range of phenomena
190 such as hydration heat, porosity, chemical shrinkage, setting time, and the effect of environmental
191 conditions on microstructural development. Not having a physical time scale and thermodynamic
192 information, as well as necessity of calibration of time scale and not being numerically convergent
193 are of the foremost drawbacks of the CEMHYD3D model. In order to solve some of drawbacks of

previous simulation models like restrictions of kinetics, limited implementation of different materials, and deficiency of CEMHYD3D regarding convergence a series of probabilistic rules were used by Bullard [76] to develop a stochastic simulation model known as *HydratiCA Simulation* model. This model is capable of simulating dissolution, nucleation, growth, and diffusion processes, as well as complicated reactions that happen in pore solutions. The two biggest advantages of this model are: the ability of the model to deliver an accurate prediction of hydration kinetics based on chemistry of solutions and temperatures, and user does not need to make any modifications in parameters during simulation. The main drawback of this model is that the model is cumbersome and computationally expensive, as several required inputs are needed to be specified at the beginning of simulation. The last simulation model discussed in this study is the multi-scale finite element-based model, called *DuCom Hydration* model which was developed by University of Tokyo. This semi-empirical model was used to predict structures' durability. This model was a constructional model rather than a microstructural one. The main disadvantages of this model were its dependency on merely empirical relations and using single particle approach to simulate hydration [6]. Although many researches have carried out on microstructural modeling of cement hydration, more work is still needed to obtain an accurate and comprehensive model which is able to evaluate field performance of concrete, address material-related problems, and simulate new cementitious materials.

µic the Model

The principles of the µic (reads mike) were obtained from the approach outlined by Navi and Pignat [77]. µic has been designed in a way that it can be improved as our knowledge of cement progresses. µic is a customizable modeling platform that enables users to model new cementitious materials and reaction algorithm, to extensively develop in the future, to easily interact with a friendly environment, to simulate a wide range of particles sizes as an influential

factor in cement hydration, and to use the model in regular computers. μic utilizes a fast and resolution-free approach called “Vector Approach”. Vector approach, versus discrete approach, is a widely accepted method using locations and sizes of objects to define objects’ geometry and to simulate multi-scale materials like cement. However, since vector approach is expensive from computational aspect some simplifying suppositions have been taken into account like spherical approximation, statistical homogeneity, and reduced particle size distribution. Among these three suppositions, spherical approximation has been executed for μic to make the model faster, as the sphere is the most regular shape and has fastest computation time. Object-oriented programming in Java also has been utilized in μic , as the most effective method for cement hydration to make μic operate faster. This is achieved by storing information in diverse assemblies without noticeable increases in required memory. μic simulates cement grains as spherical particles with determined radius and initial coordination in a virtual computational cube. Cement hydration is then simulated through decrease in radius of reacting particles, and simultaneous formation of hydration products in different layers on available surface of unreacted phases or in porosities. In addition to nucleation and growth of hydration products and by-products, overlapping of hydration products is also included.

Experimental Program

Materials

Cement and Water

An ASTM C150 [78] Type I/II ordinary portland cement (OPC) was used in this study. Table 3 shows the cement chemical composition as determined by X-ray fluorescence (XRF) analysis. Potential primary cement components used in this study, calculated by Bogue equations

[78] and Rietveld analysis of XRD are summarized in Table 4. Distilled water was also used as mixing water for this study.

Table 03. Chemical components of cementitious materials

Cementitious Materials	Chemical Components							
	SiO ₂ (%)	Al ₂ O ₃ (%)	CaO (%)	Na ₂ O (%)	K ₂ O (%)	Cr ₂ O ₃ (%)	Fe ₂ O ₃ (%)	CaCO ₃ (%)
OPC	19.66	4.71	62.74	0.12	0.56	-	3.26	2.2
Clear Glass	73.50	0.06	9.02	12.65	0.02	0.02	0.28	-
Green Glass	73.10	1.65	10.55	12.34	0.58	0.24	0.44	-

Table 0. Potential composition of cement based on Bogue equations and Rietveld analysis

Bogue equations				Rietveld Analysis						
C ₃ S (%)	C ₂ S (%)	C ₃ A (%)	C ₄ AF (%)	Alite (%)	Belite (%)	Aluminate (%)	Ferrite (%)	Lime (%)	Calcite (%)	Gypsum (%)
58	11	7	10	64.1	14.6	4.36	10.01	0.40	2.54	4.03

Glass Cullet

Clear and green glass was used in this study because previous research results showed that clear glass is the most commonly available type of glass and green glass has the highest pozzolanic reaction. Small impurities are added as coloring agent in glass production. These coloring agents change the glass composition and structure. The source of clear glass was waste window glass collected from recycling company at Kansas City, KS, and the source of green glass was bottle glass from the same bottle manufacturer and bottle type collected from recycling center in Manhattan, KS. The glass was washed and dried to remove any residues before crushing. After crushing to smaller than 1.18 mm (No. 16), glass particles were milled in a laboratory ball mill. After ball milling, the glass was wet-sieved using a sieve with 25 µm openings and isopropanol. In addition to single glass types and particle sizes, one combination of the two glass types, green and glass, and one narrow size ranges, 0-25 µm, were used in this study. The chemical compositions of the glass powders used for this study are shown in Table 3, while Table 5 shows

glass powder density and Blaine fineness. Glass powder and cement particle size distribution is shown in Figure 2, demonstrating that the gradation of glass powder is very similar to the cement gradation. Particle shape and texture of cement grain and glass particles was investigated by scanning electronic microscopy (SEM), as shown in Figure 3.

Table 5. Density and Blaine surface area of cementitious materials

Materials	Density (Kg/m ³)	Blaine Surface Area (m ² /Kg)
OPC	3150	395
Clear Glass 0-25 µm	2477	433
Green Glass 0-25 µm	2501	476
Green 0-25 µm + Clear 0-25 µm (Mix 2)	2492	454.5

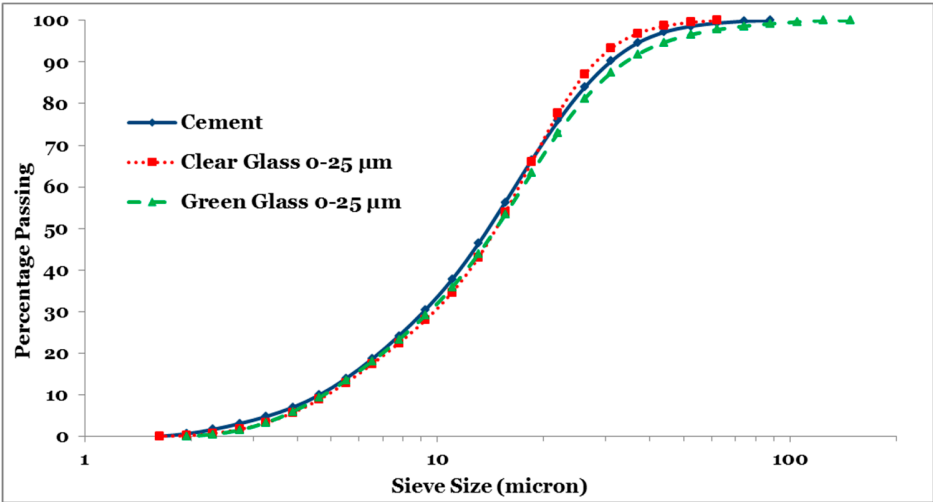


Figure 2. Gradation of cementitious materials

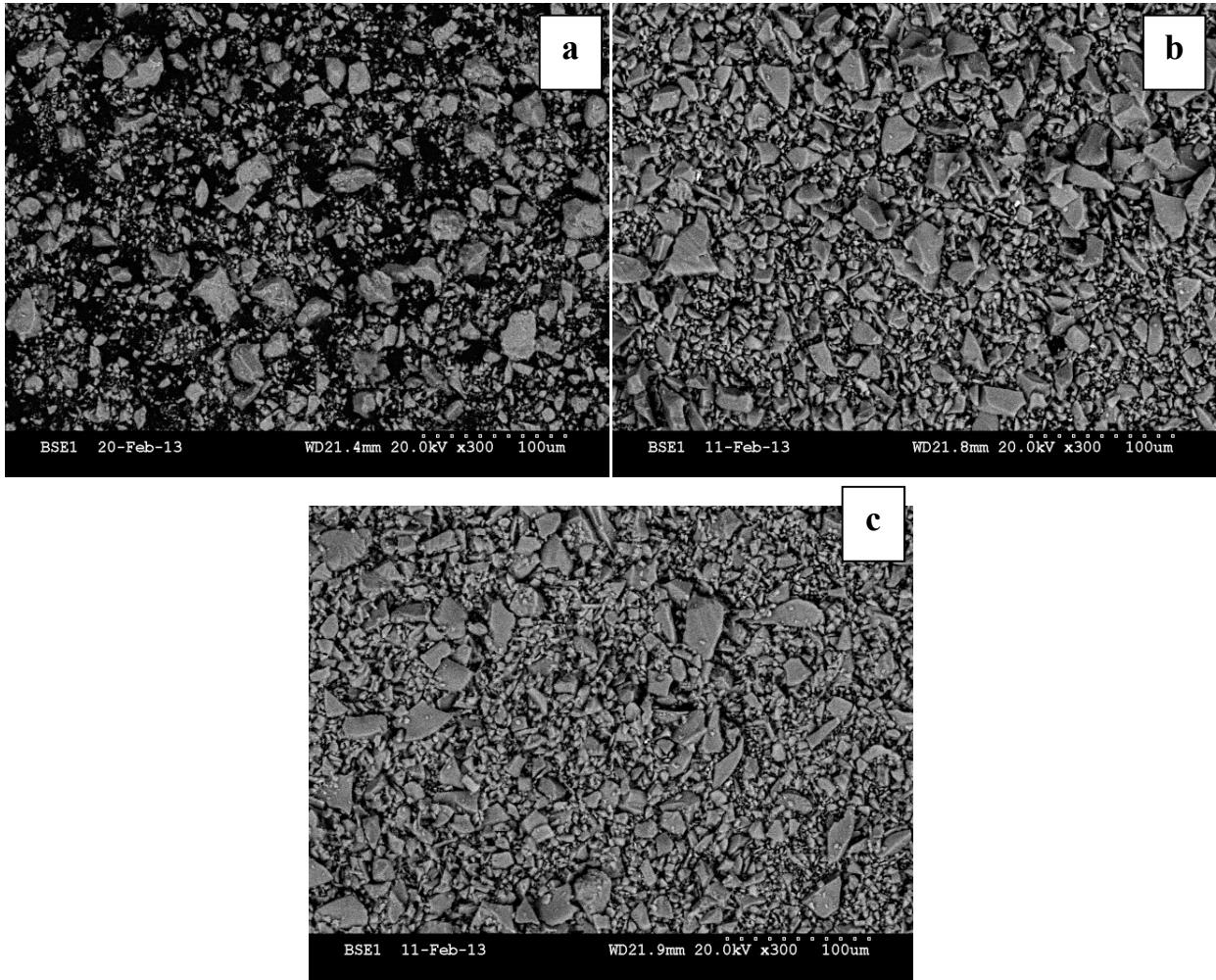


Figure 3. SEM Images; a. Cement grain, b. Clear glass 0-25 μm, c. Green glass 0-25 μm

Methodology

Cement Paste Preparation

Cement paste samples were made with a water-cementitious materials ratio (w/cm) of 0.35, using a 25% replacement by mass of portland cement with individual clear or green glass powder as well as combined types and sizes of glass cullet, as recommended by other studies [49]. Samples were cured at 50°C in order to show hydration behavior at elevated temperature. Water and cementitious materials were stirred at slow speed (500 rpm) for three minutes, followed by two minutes rest, and then high speed (2000 rpm) for two minutes. Samples were pre-conditioned to

the desired curing temperature before mixing. After mixing, paste samples for thermogravimetric analysis (TGA) were casted in a polystyrene vials with diameter of 17 mm and height of 50 mm. Paste samples with or without the glass powder were wet-cured beginning at six hours after casting at three curing temperatures in a temperature-controlled chamber.

Isothermal Calorimetry

The hydration rate and total heat of hydration of cement were measured using an eight-channel isothermal calorimeter. Approximately 30 g of paste samples were placed in the sample containers, weighed, and placed into the calorimeter. The time between initial contact of water and cementitious materials and placing samples into the calorimeter was less than 15 min in all cases. The influence of glass type and curing temperature on hydration kinetics could be observed as the change in heat of hydration when the calorimetry results were normalized by the mass of dry portland cement used in the paste. This change in heat of hydration is likely from the dilution effect providing additional space for hydration product formation and glass powder providing nucleation platform for calcium silicate hydrate (C-S-H) and glass hydration [79,80].

Thermogravimetric Analysis (TGA)

To study the pozzolanic reactivity of glass powder, the calcium hydroxide (CH) content of cement paste samples was measured by the thermogravimetric analysis (TGA) using the approach outlined by Marsh [81]. Paste samples with or without the single particle and combined glass powder were wet-cured starting at six hours after casting at three curing temperatures. At 1, 7, 28, and 91 days after casting, hydration was stopped by solvent exchange with isopropanol. Paste samples were cut from the 17-mm diameter samples into 2 mm thick discs and placed in isopropanol for seven days. The samples were then dried in a vacuum for at least four days. To minimize the risk of carbonation during casting, drying, and testing, high level of care was taken.

Finally, 30-50 mg of dried paste samples were ground and heated at a rate of 20°C/min to 1,000°C in nitrogen atmosphere.

Hydration Modeling using μic

In this section, two single particles and two types, as well as one blended glass particles and type of very finely ground glass are simulated by means of μic .

Modeling of Cement Hydration

In order to simulate glass cullet reactivity, the cement hydration needs to be modeled by means of μic . Modeling cement hydration means to fit reaction kinetics equation parameters. For this study, the reaction kinetics equation used for cement hydration as well as glass reactivity is the Avrami equation. The Avrami equation is a nucleation and growth model which was initially developed for metallic crystals. However, its S-shape is similar to the typical shape of cement hydration. Its simplicity also helps make it one of the most popular reaction kinetics equations used for modeling cement hydration [6]. The Avrami equation can be expressed as Eq. (6) [82]:

$$-\ln(1-\alpha) = ktn \quad \text{Eq. (6)}$$

where α is cement degree of hydration, t is elapsed time from initial contact of water and cement, and k and n are Avrami parameters which depend on reaction rate and how crystals grow, respectively. The Avrami constant n is a function of three additional parameters as shown in Eq. (7):

$$n = (P/S) + Q \quad \text{Eq. (7)}$$

where P is related to dimensions of products growths and can be 1, 2, or 3 for one-, two-, or three-dimensional growth, respectively. Parameter S is 1 for interface controlled and 2 for

diffusion controlled mechanisms. Q is a function of rate of nucleation and can be 1 for continuous nucleation and 0 for only initial nucleation [69]. Respectively selecting 3, 1, and 1 for P , S , and Q , the value of n will be 4 for this study. Thus, the objective of modeling cement hydration is to find the Avrami parameter k by fitting degree of hydration results obtained from μic to those obtained from isothermal calorimetry. These values were used as fixed inputs for the next step of the modeling process: modeling glass cullet.

The modeling in this study is done for three main compounds of cement, namely C_3S , C_2S , and C_3A , to attain more descriptive results. Having very complicated hydration products and known to be rather slow reacting, C_4AF has not been modeled in this study. Additionally, cement hydration and glass reactivity are simulated only at $50^\circ C$ because previous experimental results showed that reaction rate of cement and pozzolanic reaction of glass cullet are more pronounced at $50^\circ C$ compared to $10^\circ C$ and $23^\circ C$ [83].

Step 1 – Initial Settings

μic reads XML files that assign the hydration rate parameters and other inputs to the modeling engine. These XML input files can be created by a graphical user interface program or developed manually. From “File” tab in the command bar of the μic interface, “Load XML File” or “Create New Reactor” is selected. In this window the name of reactor, size of virtual paste cube, hydration time step, and some other initial settings such as pixel sizes and background color are determined. For this study, the size of the virtual paste cube is set to be a $100 \times 100 \times 100$ voxel cube. Figure 4 shows a screen shot of the Reactor window.

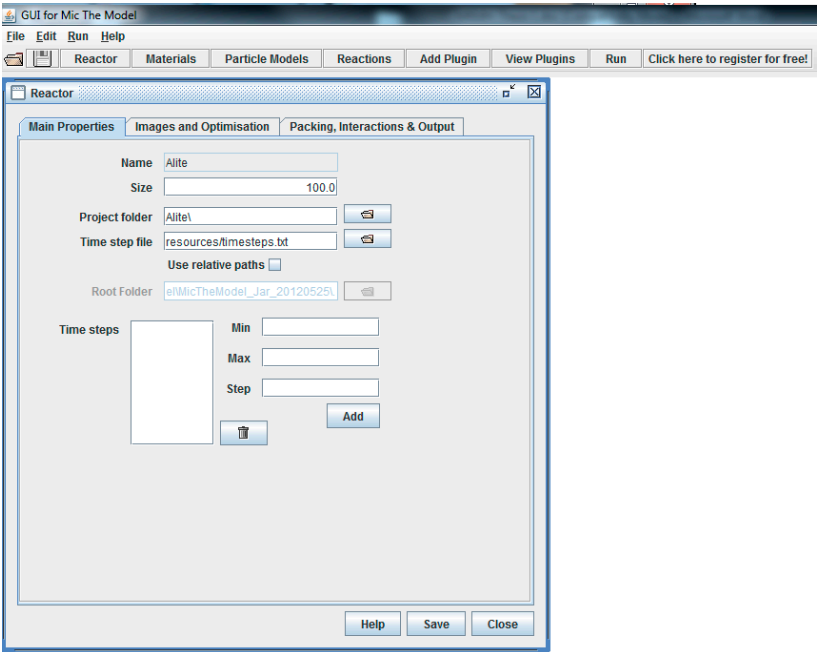


Figure 4. Reactor window in μic

Step 2 – Materials Defining

Clicking on the “Materials” tab, a new window is opened in which all constituents used for modeling are defined. The constituents and properties used for cement hydration modeling are summarized in Table 6. Initial fractions of some constituents are volume percentages of those constituents, and have been calculated through volumetric stoichiometry. Figure 5 also shows the Materials window in μic .

Table 0. Properties of all materials used for cement hydration modeling in μic

Name	Density	Initial fraction	Diffusivity	Color
Alite	3.21	0.2832	0.04	
Belite	3.28	0.0646	0.04	
Aluminate	3.03	0.0193	0.04	
C-S-H	2.00	0.0	0.04	
CH	2.20	0.0	0.04	
Ettringite	2.00	0.0	0.04	

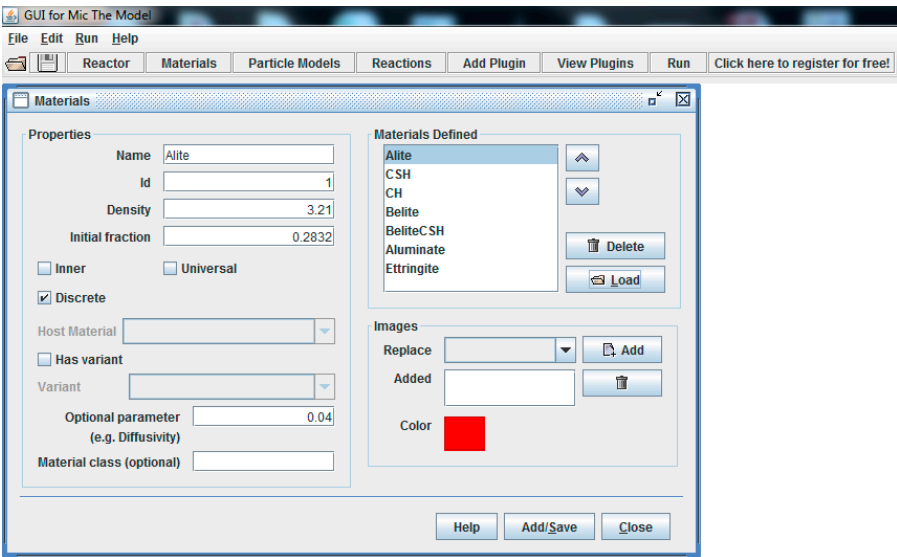


Figure 5. Materials window in μic

Step 3 – Particle Model

The “Particle Models” input section enables users to define reactant particles and their gradation, as well as layers of hydration products. Gradation results of different constituents are obtained by laser particle size distribution. For example, the layers of alite are alite (unreacted core C_3S) and C-S-H layer formed on the C_3S particles. Table 7 lists the reactants and corresponding products layers. Figure 6 shows a preview of Particle Models window.

Table 7. List of reactants and corresponding products used for modeling in μic

Reactant	Products Layer
Alite	Alite + C-S-H
Belite	Belite + C-S-H
Aluminate	Aluminate + Ettringite

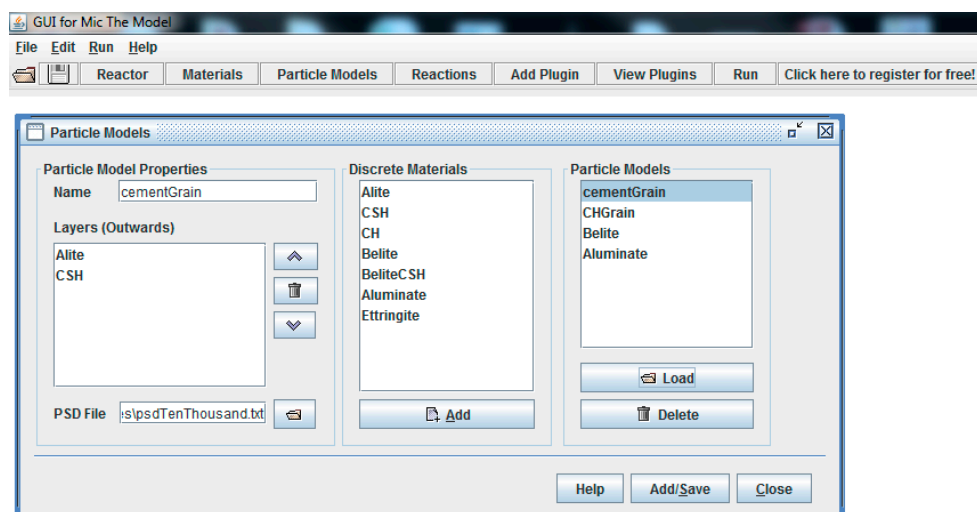
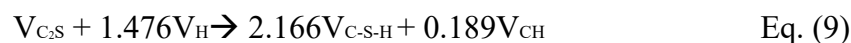
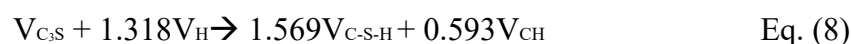


Figure 6. Particle Models window in μic

Step 4 – Reactions

A critical step in simulation in μic is to define reactions of different phases through mathematical equations. Hydrations of different phases of cement are typically expressed as mass equations, as previously shown in Eq. (1), Eq. (2), and Eq. (3). Since cement hydration is modeled in a paste cube and all fractions and calculations are volumetric-based, reactions equations should also be converted to volumetric equations. These conversions are done through stoichiometry and by assigning densities. Material densities used in this study are shown in Table 8. Equations (8), (9), and (10) show the volumetric equations for the reaction of different phases:



“Reactions” window allows users to define and customize different hydration equations. In this study, hydration equations (8), (9), and (10) are plugged into μic . As shown in Figure 7.

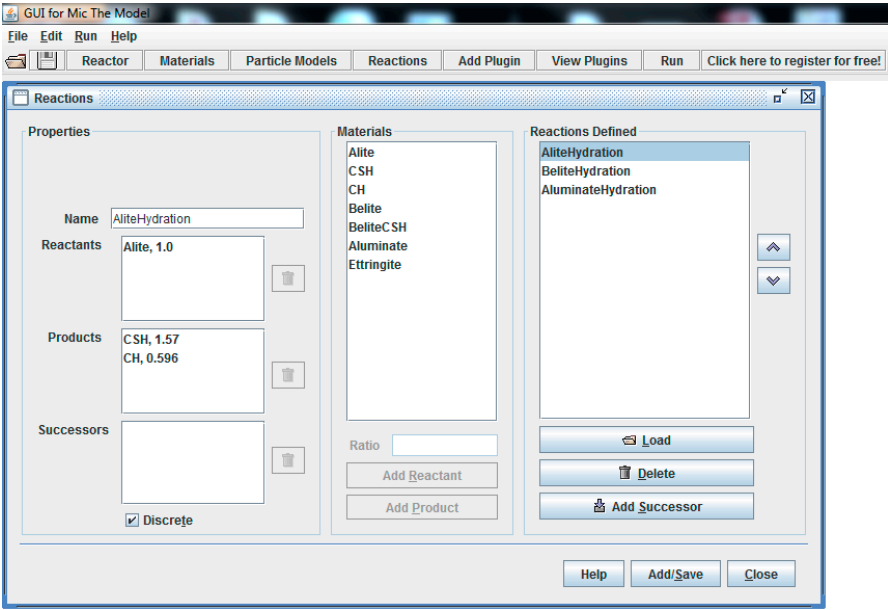


Figure 7. Particle Models window in μic

Step 5 – Plugins

“Plugins” is a list of several types of hydration model. Users are able to select desired model depending on objective of the modeling, or add new customized plugins in Java to the list. As mentioned earlier, this study uses Avrami model for cement hydration. Figure 8 shows how Avrami model is selected.

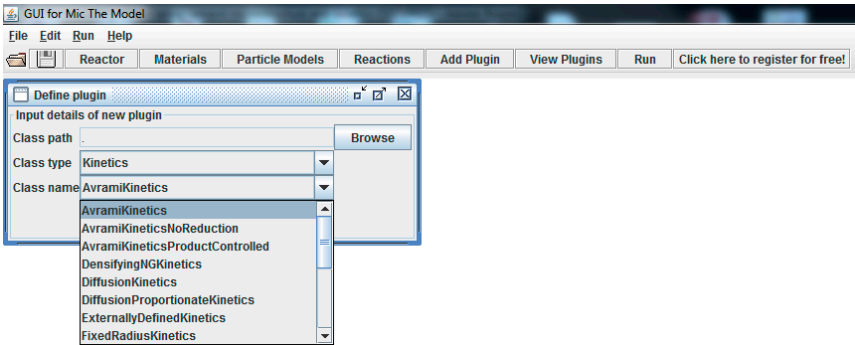


Figure 8. Selection of Avrami kinetics model

Then Avrami constants, starting time (set to zero), initial degree of hydration (set to zero), order of implementation of kinetics (set to one for all, as all reactions happen simultaneously), and types of reactions and reactants are determined. Figure 9 shows a set Avrami model for alite hydration.

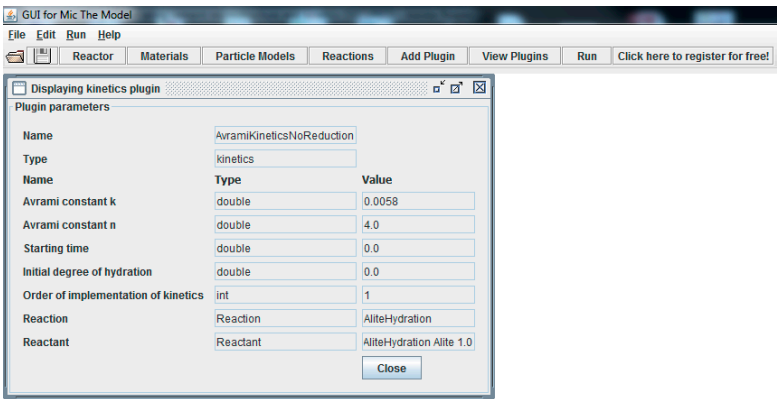


Figure 9. Avrami model set for alite hydration

Step 6 – View Plugins

This window allows users to control all determined plugins.

Step 7 - Run


After saving the project, users can run the model. Once the model runs, a folder in the name of project is automatically created which contains a cross section of simulated hydration at each time steps (Figure 10), as well as an excel file that gives the degree of hydration and changes in volume of all constituents at each time steps. The black pixels shown in Figure 10 are porosity.

Modeling of Glass Cullet Reactivity

Single type and particle size

Modeling of pozzolanic reaction of two glass types (i.e. clear and green) and one single particle size (<25 μm) of finely ground glass followed the same basic modeling steps as those explained for cement hydration modeling, with only a few alterations. Clear glass smaller than 25 μm, green glass smaller than 25 μm, and pozzolanic C-S-H were added to the previous constituents, but simulated through separate models to determine the reaction rate parameters to use for each material by itself. Table 8 shows the properties of the individual types of glass in μic.

Table 0. Properties of glass particles used for pozzolanic reaction modeling in μic

Name	Density	Initial fraction	Diffusivity	Color
Clear <25 μm	2.48	0.1455	0.0	
Green <25 μm	2.50	0.1455	0.0	
Pozzolanic C-S-H	2.00	0.0	0.04	

In the “Particle Models” window, gradations of glass particles obtained from laser particle size distribution device were defined. Reactants and products layers on glass particles were also determined. Table 9 lists the glass reactants and corresponding hydration products.

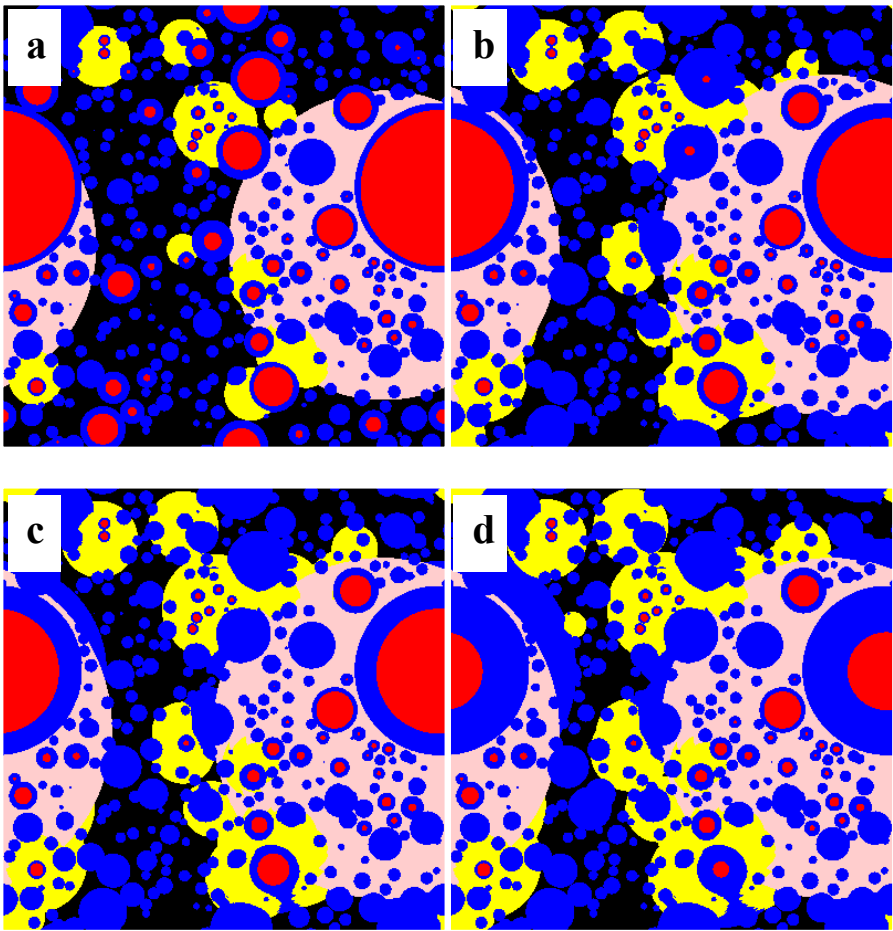
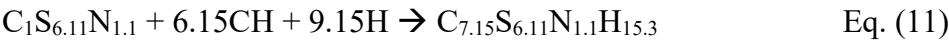


Figure 10. Cross sections of simulated cement hydration at 50°C: a. 1 day, b. 28 days, c. 91 days, and d. 365 days

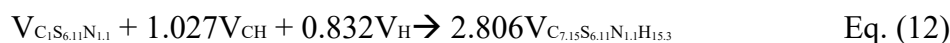
Table 0. List of glass reactants and corresponding products used for modeling in μ c

Reactant	Products Layer
Clear <25 μ m	G025 + C-S-HG025
Green <25 μ m	GG025 + C-S-HGG025

The pozzolanic reaction equation for glass cullet used in concrete was determined by Saeed et al. [43] and is shown determine in Eq. (11):



where N is shorthand for Na₂O. Using the material densities shown in Table 8 and the stoichiometric calculations from Eq. (11) give a volumetric-based equation shown in Eq. (12):



The last step for modeling the pozzolanic reactivity of glass cullet was to obtain the Avrami constants. These constants were attained through a trial-error process used in modeling to fit to the CH content calculated curve determined by TGA measurements.

Combined glass types and particle sizes

The process of the modeling Mix 2 was the same as that previously outlined for cement and single glass particles. To model combined glass types and sizes (Mix 2), the fit Avrami constants for cement, Clear glass smaller than 25 μm, and Green glass smaller than 25 μm at 50°C obtained by modeling them in separate steps were used. The only difference between modeling Mix 2 and earlier materials is that the initial fractions of both Clear glass smaller than 25 μm and Green glass smaller than 25 μm are 0.07275 instead of 0.1455 (see Table 8).

Results and Discussions

Avrami Constants for Cement

The Avrami constants found from fitting the model degree of hydration to the degree of hydration found from isothermal calorimetry for three compounds of cement are shown in Table 10. Figure 11 shows the fit obtained from the modeling to the measured data.

Table 10. Avrami constants of three phases obtained by μ ic

Name	Avrami Constants	
	k	n
Alite	5.8E-3	4.0
Belite	3.3E-3	4.0
Aluminate	7.0E-3	4.0

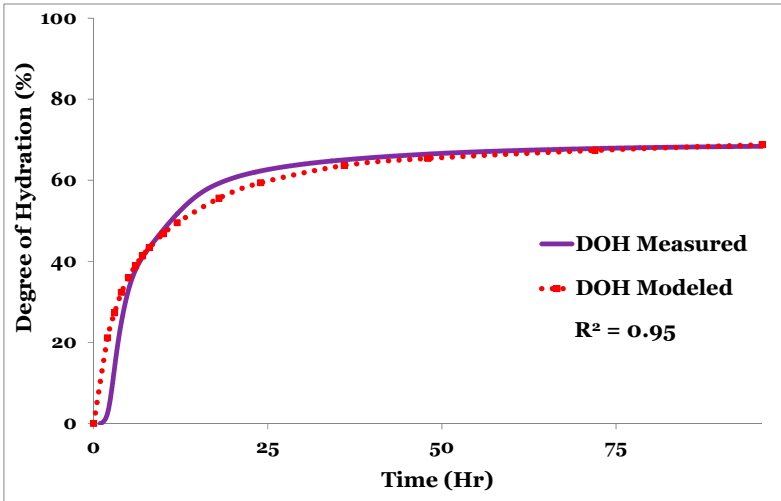


Figure 11. Fitting modeled to measured DOH results

Avrami Constants of Single Glass Cullet

Cross sections of simulated microstructures for cementitious systems containing green glass smaller than 25 μ m are shown in Figure 12. Following the same method discussed for modeling clear glass, the Avrami parameters for the green glass smaller than 25 μ m material were fit using the TGA measurements as shown in Figure 13. The Avrami parameters for the pozzolanic reactivity of Green glass smaller than 25 μ m were found to be $k = 1.0899\text{E-}5$ and $n = 1.5$.

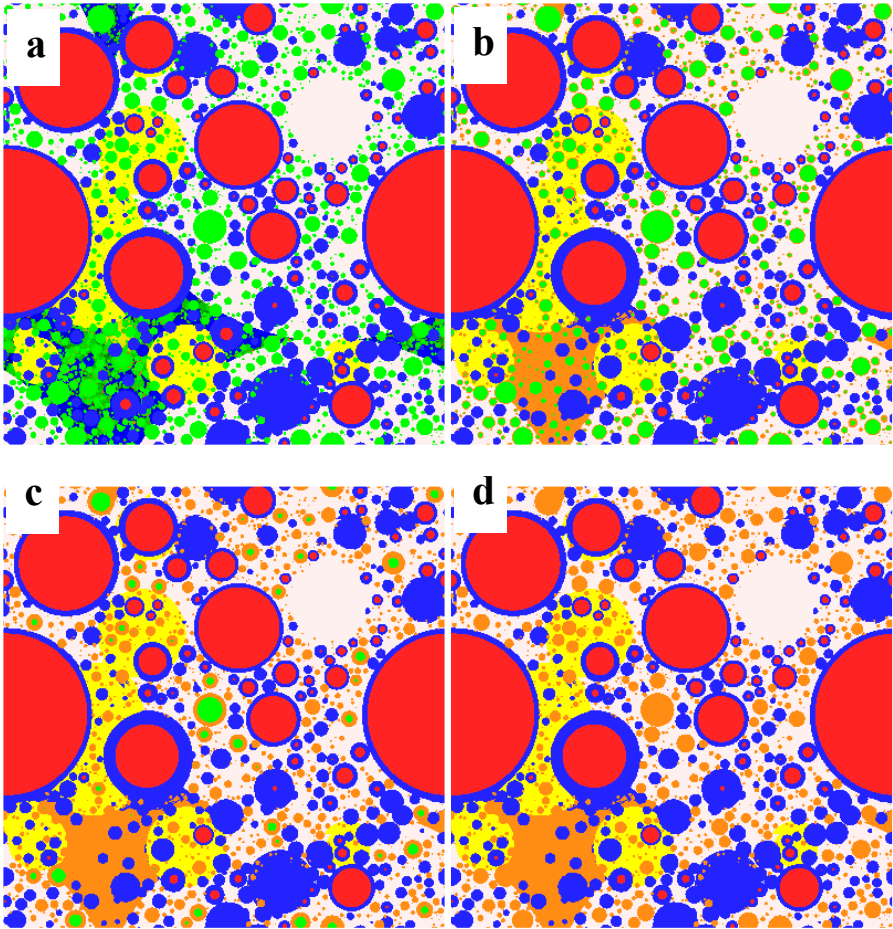


Figure 12. Cross sections of simulated pozzolanic reaction of Green glass <25 μm at 50°C: a. 1 day, b. 28 days, c. 91 days, and d. 365 days

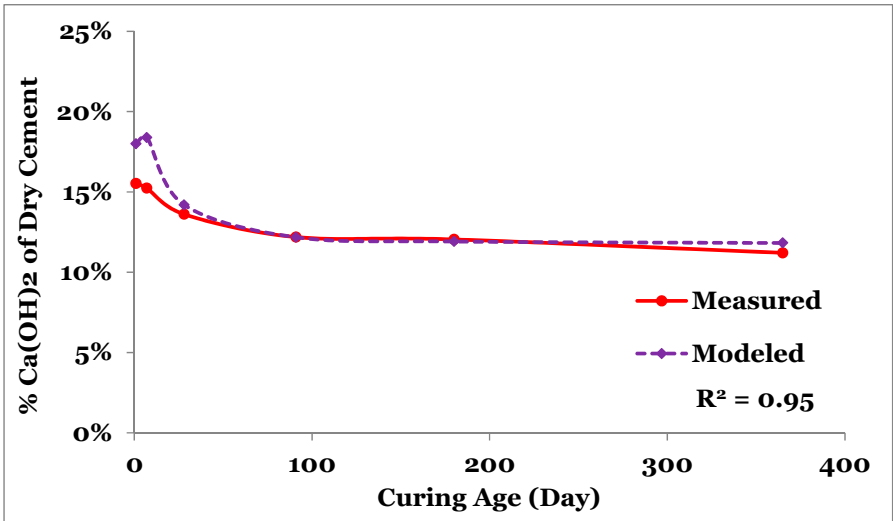


Figure 13. Fitting modeled to measured results of CH content for Green glass <25 μm at 50°C

Avrami Constants of Combined Glass Cullet

Cross sections of simulated microstructures for cementitious systems Mix 2 are shown in Figure 14.

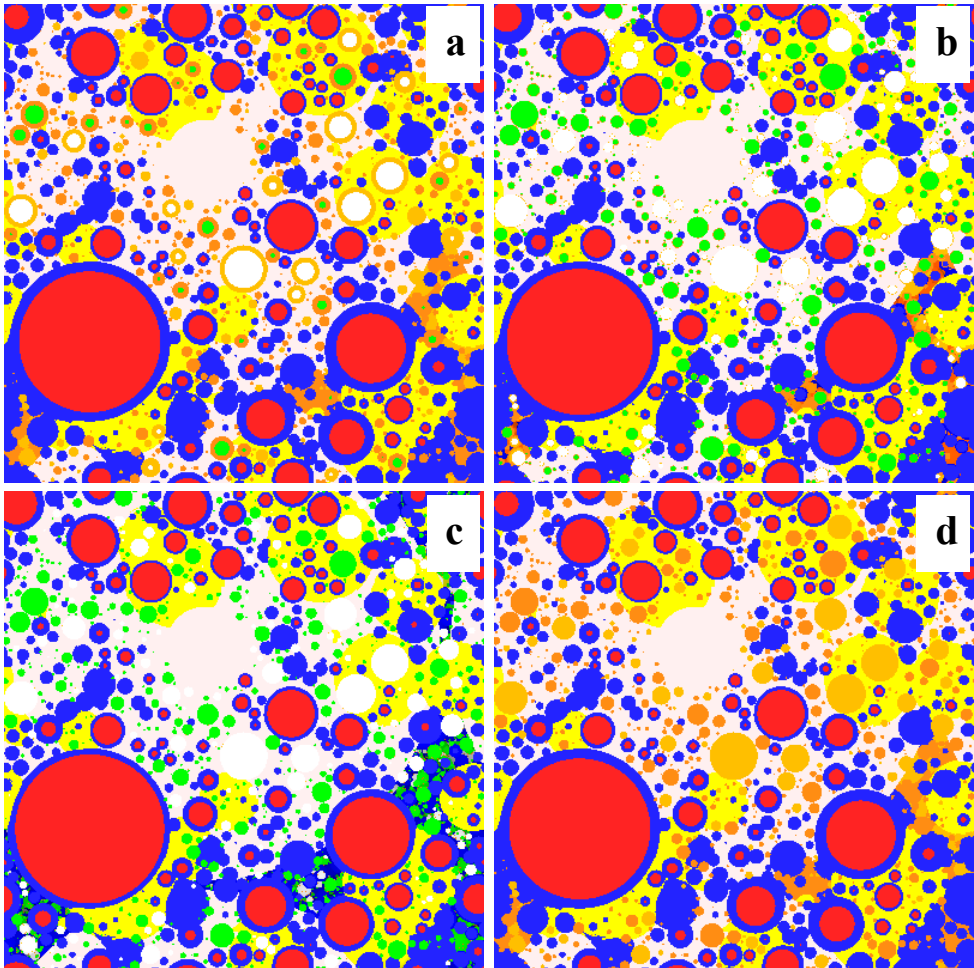


Figure 14. Cross sections of simulated pozzolanic reaction of Mix 2 at 50°C: a. 1 day, b. 28 days, c. 91 days, and d. 365 days

The first interesting point is that the modeling of Mix 2 pozzolanic reactivity was in well agreement with the calculated CH content obtained from the modeling of single particles ($R^2=0.99$) as shown in Figure 15. Results also showed that simulation for Mix 2 was not satisfactory and the differences between measured and modeled values are significant. In other words, the effect of particle sizes of on glass cullet pozzolanic reactivity could not be accounted for through linear addition as expected to be obtained by microstructural modeling. This

discrepancy might be caused by some errors in Avrami constants attained in modeling of cement and single glass particles. Another possible explanation for inaccurate modeling is the effect of elevated curing temperatures on reactivity and mechanical properties of cementitious systems containing Mix 2. As explained earlier, Mix 2 physical properties do not follow the linear-addition behavior. In other word, elevated curing temperatures is an important parameter not only in reactivity and mechanical properties of concrete containing mixed types and sizes of glass, but also in microstructural modeling.

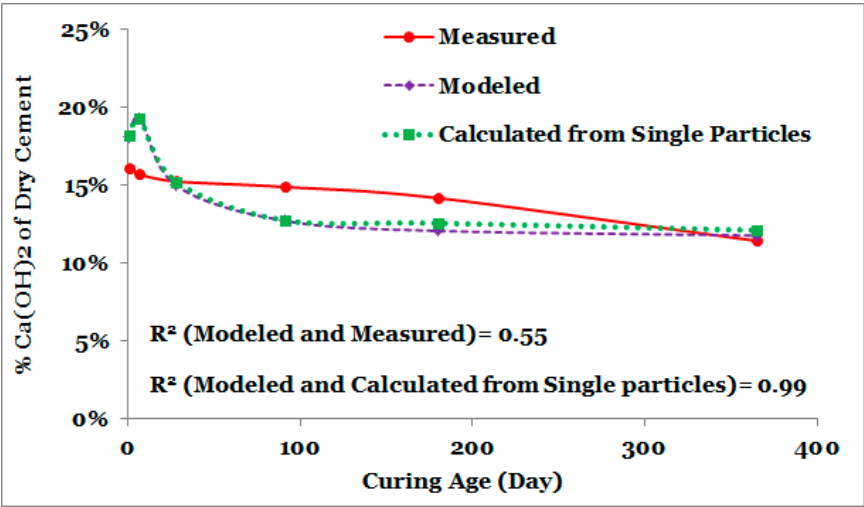


Figure 15. Fitting modeled to measured results of CH content for Mix 2 at 50°C

Conclusion

The objective of this study was to microstructurally model cement hydration and cementitious systems containing single glass types and sizes and combined glass types and sizes. In order to achieve these goals, a newly developed modeling platform called “μic” was used. Modeling outputs were fitted to the results of experimental studies. Some minor errors were seen in modeling cement and glass particles, especially at early ages. Despite acceptable fits of single type and size of glass cullet, microstructural modeling could not verify that the effect of particle size distribution on pozzolanicity (i.e. combined glass types and sizes) of glass powder is linear.

This inaccuracy can be attributed not only to some inherent limitations of microstructural modeling such as limited knowledge about mechanisms of hydration kinetics, but also to accumulation of minor errors in earlier steps of modeling, effects of some important factors such as curing temperatures and gradation, and accuracy of reaction equations. It can be recommended that a comprehensive stoichiometry study is performed on pozzolanic reaction of glass cullet to obtain a precise equation which can be used in microstructural modeling platforms like μic .

Acknowledgment

This work has been funded by the National Science Foundation (CMMI-1032636). The author would like to acknowledge Ash Grove Cement Company for performing XRF measurements and measuring surface area and density of materials, and Kyle A. Riding for his valuable advices.

References

- [1] United Nation Environmental Program (UNEP) (2010). Greening Cement Production has a Big Role to Play in Reducing Greenhouse Gas Emissions. http://na.unep.net/geas/getUNEPPageWithArticleIDScript.php?article_id=57 (downloaded on March 14, 2014)
- [2] Van Oss, H. G. (2011). USGS Mineral Program Cement Report. United States Geological Survey (USGS) Report, pp. 38-39.
- [3] Lin, F., and Meyer C. (2009). Hydration kinetics modeling of Portland cement considering the effects of curing temperature and applied pressure, Cement and Concrete Research, vol. 39, pp. 255–265.
- [4] Kirby, D. M., and Biernacki, J. J. (2012). The effect of water-to-cement ratio on the hydration kinetics of tricalcium silicate cements: Testing the two-step hydration hypothesis, Cement and Concrete Research, vol. 42, pp. 1147–1156.
- [5] Kosmatka, S. H., Kerkhoff, B., and Panarese, W. C. (2003). Design and Control of Concrete Mixtures, Portland Cement Association, 14th Edition, USA, pp. 360.
- [6] Bishnoi, S. (2008). Vector Modelling of Hydrating Cement Microstructure and Kinetics, Doctoral Thesis, Swiss Federal Institute of Technology in Lausanne, Laboratory of Materials of Construction, pp. 166.
- [7] Bullard, J. W., Jennings, H. M., Livingston, R. A., Nonat, A., Scherer, G. W., Schweitzer J. S., Scrivener, K. L., and Thomas, J. J. (2011). Mechanisms of cement hydration, Cement and Concrete Research, vol. 41, pp. 1208–1223.
- [8] Stein, H. N., and Stevels J. M. (1964). Influence of silica on the hydration of 3CaO, SiO₂, J. Appl. Chem., vol. 14, pp. 338–346.

- 525 [9] Juilland, P., Gallucci, E., Flatt, R., and Scrivener, K. (2010). Dissolution theory applied to
526 the induction period in alite hydration, *Cement and Concrete Research*, vol. 40, pp. 831–
527 844.
- 528 [10] Gartner, E. M., and Gaidis, J. M. (1989). Hydration mechanisms, I, in: J. Skalny (Ed.),
529 *Materials Science of Concrete*, Vol. 1, American Ceramic Society, Westerville, OH, pp.
530 95–125.
- 531 [11] Garrault, S., and Nonat, A. (2001). Hydrated layer formation on tricalcium and dicalcium
532 silicate surfaces: experimental study and numerical simulations, *Langmuir*, vol. 17, pp.
533 8131–8138.
- 534 [12] Garrault-Gauffinet, S., and Nonat, A. (1999). Experimental investigation of calcium
535 silicate hydrate (C–S–H) nucleation, *J. Cryst. Growth*, vol. 200, pp. 565–574.
- 536 [13] Damidot, D., Nonat, A., and Barret P. (1990). Kinetics of tricalcium silicate hydration in
537 diluted suspensions by microcalorimetric measurements, *J. Am. Ceram. Soc.*, vol. 73(11),
538 pp. 3319–3322.
- 539 [14] Arvidson, R. S., Fischer, C., and Luttge, A. (2009). Resolution of crystal dissolution and
540 growth processes at multiple scales, *Geochim. Cosmochim. Acta*, vol. 72 (12), A34.
- 541 [15] Damidot, D., Bellmann, F., Möser, B., and Svoidnich, T. (2007). Calculation of the
542 dissolution rate of tricalcium silicate in several electrolyte compositions, *Cement*
543 *WapnoBeton*, vol. 12/74 (2), pp. 57–67.
- 544 [16] Ménétrier, D., Jawed, I., Sun, T. S., and Skalny J. (1979). ESCA and SEM studies on
545 early C₃S hydration, *Cem. Concr. Res.*, vol. 9, pp. 473–482.
- 546 [17] Tadros, M. E., Skalny, J., Kalyoncu, R. S. (1976). Early hydration of tricalcium silicate,
547 *Journal of the American Ceramic Society*, vol. 59 (7–8), pp. 344–347.

- 548 [18] Young, J. F., Tong, H. S., and Berger, R. L. (1977). Compositions of solutions in contact
549 with hydrating tricalcium silicate pastes, *Journal of the American Ceramic Society*, vol. 60
550 (5–6), pp. 193–198.
- 551 [19] Damidot, D., and Nonat, A. (1994). C₃S hydration in diluted and stirred suspensions: (I)
552 study of the two kinetic steps, *Advances in Cement Research*, vol. 6 (21), pp. 27–35.
- 553 [20] Barret, P., and Ménétrier, D. (1980). Filter dissolution of C₃S as a function of lime
554 concentration in a limited amount of lime water, *Cement and Concrete Research*, vol. 10,
555 pp. 521–534.
- 556 [21] Kondo, R., and Ueda, S. (1968). Kinetics of hydration of cements, *Proceedings of the 5th*
557 international symposium on chemistry of cement, Tokyo, pp. 203–248.
- 558 [22] Pommersheim, J. M., and Clifton, J. R. (1979). Mathematical modeling of tricalcium
559 silicate hydration, *Cement and Concrete Research*, Vol. 9, pp. 765–770.
- 560 [23] Gartner, E. M., and Gaidis, J. M. (1989). Hydration mechanisms, I, in: J.P. Skalny (Ed.),
561 *The Materials Science of Concrete, I*, American Ceramic Society, pp. 95–125.
- 562 [24] Jennings, H. M., and Pratt, P. L. (1979). An experimental argument for the existence of
563 a protective membrane surrounding Portland cement during the induction period, *Cement*
564 *and Concrete Research*, vol. 9, pp. 501–506.
- 565 [25] Xie, T., and Biernacki, J. J. (2011). The origins and evolution of cement hydration
566 models, *Comp. Concr.*, vol. 8(6), pp. 647–675.
- 567 [26] Garraut, S., Behr, T., and Nonat, A. (2006). Formation of the C-S-H layer during early
568 hydration of tricalcium silicate grains with different sizes, *Journal of Physical Chemistry*,
569 Vol. 110, pp. 270–275.
- 570 [27] Bishnoi, S., and Scrivener, K. L. (2009). μ ic: a new platform for modeling the hydration
571 of cements, *Cem. Concr. Res.*, vol. 39(4), pp. 266–274.

- 572 [28] Bishnoi, S., and Scrivener, K. L. (2009). Studying nucleation and growth kinetics of alite
573 hydration using μ ic, *Cem. Concr. Res.*, vol. 39(10), pp. 849–860.
- 574 [29] Van Breugel, K. (1991). Simulation of hydration and formation of structure in hardening
575 cement-based materials, PhD Thesis, Delft University of Technology, The Netherlands.
- 576 [30] Danielson, U. (1962). Heat of hydration of cement as affected by water–cement ratio,
577 Paper IV-S7, Proceedings of the 4th International Symposium on the Chemistry of Cement,
578 Washington DC, USA, pp. 519–526.
- 579 [31] Taplin, J. H. (1969). A method for following hydration reaction in Portland cement paste,
580 *Australian Journal of Applied Sciences*, vol. 10, pp. 329–345.
- 581 [32] Knudsen, T. (1982). Modeling hydration of portland cement — the effect of particle size
582 distribution, *in*: Young, J. F. (Ed.). *Characterization and performance prediction of cement*
583 *and concrete*, United Engineering Trustees, Inc., New Hampshire, USA, pp. 125–150.
- 584 [33] Frigione, G., and Marra, S. (1976). Relationship between particle size distribution and
585 compressive strength in Portland cement, *Cement and Concrete Research*, vol. 6(1), pp.
586 113–127.
- 587 [34] Bezjak, A. (1986). An extension of the dispersion model for the hydration of Portland
588 cement, *Cement and Concrete Research*, vol. 16 (2), pp. 260–264.
- 589 [35] Lerch, W., and Ford, C. L. (1948). Long-term study of cement performance in concrete:
590 chapter 3, Chemical and physical tests of the cements, *ACI Journal*, vol. 19(8), pp. 745–
591 795.
- 592 [36] Escalante-Garcia, J. I. (2003). Nonevaporable water from neat OPC and replacement
593 materials in composite cements hydrated at different temperatures, *Cement and Concrete*
594 *Research*, vol. 33(11), pp. 1883–1888.

- [37] Bentur, A., Berger, R. L., Kung, J. H., Milestone, N. B., and Young J. F. (1979), Structural properties of calcium silicate pastes: II, effect of the curing temperature, Journal of the American Ceramic Society, vol. 62(7–8), pp. 362–366.
- [38] Worrell, E., Price, L., Martin, N., Hendriks, C., and Meida, L. O. (2001). Carbon dioxide emissions from the global cement industry, Annual Review of Energy and the Environment, vol. 26, pp. 303-329.
- [39] Lothenbach, B., Scrivener, K., Hooton, R. D. (2011). Supplementary cementitious materials, Cement and Concrete Research, vol. 41, pp. 1244–1256.
- [40] United States Environmental Protection Agency (EPA) (2010). Municipal solid waste in the United States: 2009 Facts and Figures. Report No. EPA530-R-10-012.
- [41] Shao, Y., Lefort, T., Moras, S., and Rodriguez, D. (2000). Studies on Concrete Containing Ground Waste Glass. Cement and Concrete Research, vol. 30(1), pp. 91-100.
- [42] Shi, C., and Zheng, K. (2007). A Review on the Use of Waste Glasses in the Production of Cement and Concrete. Resources, Conservation and Recycling, vol. 52(2), pp. 234-247.
- [43] Saeed, H. A., Ebead, U. A., Tagnit-Hamou, A., and Neale, K. W. (2011). Stoichiometric study of activated glass powder hydration. Advances in Cement Research, vol. 24(2), pp. 91-101.
- [44] Dyer, T. D., and Dhir, R. K. (2001). Chemical Reactions of Glass Cullet Used as Cement Component. Journal of Material of Civil Engineering, vol. 13(6), pp. 412-417.
- [45] Shayan, A., and Xu, A. (2004). Value-added Utilization of Waste Glass in Concrete. Cement and Concrete Research, vol. 34(1), pp. 81-89.
- [46] Shayan, A., and Xu, A. (2006). Performance of Glass Powder as a Pozzolanic Material in Concrete: A Field Trial on Concrete Slabs. Cement and Concrete Research, vol. 36(3), pp. 457-468.

- 619 [47] Pereira-de-Oliveira, L. A., Castro-Gomes, J. P., and Santos, P. M. S. (2012). The Potential
620 Pozzolanic Activity of Glass and Red-clay Ceramic Waste as Cement Mortars
621 Components. *Construction and Building Material*, vol. 31, pp. 197-203
- 622 [48] Schwarz, N., and Neithalath, N. (2008). Influence of a fine glass powder on cement
623 hydration: Comparison to fly ash and modeling the degree of hydration. *Cement and*
624 *Concrete Research*, vol. 38, pp. 429-436
- 625 [49] Tagnit-Hamou, A., and Bengougam, A. (2012). The Use of Glass Powder as
626 Supplementary Cementitious Material. *Concrete International*, vol. 34(3), pp. 56.
- 627 [50] Shi, C., Wu, Y., Riefler, C., and Wang, H. (2005). Characteristics and Pozzolanic
628 Reactivity of Glass Powders. *Cement and Concrete Research*, vol. 35(5), pp. 987-993.
- 629 [51] Nassar, R., and Soroushian, P. (2011). Field Investigation of Concrete Incorporating
630 Milled waste Glass, *Journal of Solid Waste Technology and Management*, vol. 37(4), pp.
631 307-319.
- 632 [52] Bajad, M. N., Modhera, C. D., and Desai, A. K. (2011). Effect of Glass on Strength of
633 Concrete Subjected to Sulphate Attack, *International Journal of Civil Engineering*
634 *Research and Development*, vol. 1(2). pp. 1-13.
- 635 [53] Meena, A., and Singh, R. (2012). Comparative Study of Waste Glass Powder as
636 Pozzolanic Material in Concrete, Bachelor Thesis, Department of Civil engineering,
637 National Institute of Technology, Rourkela, India, pp. 46.
- 638 [54] Kou, S. C., and Xing, F. (2012). The Effect of Recycled Glass Powder and Reject Fly
639 ash on the Mechanical Properties of fiber-reinforced Ultralight Performance Concrete,
640 *Advances in Material science and Engineering*, pp. 8.

- 641 [55] Federico, L. (2013). Waste Glass – A Supplementary Cementitious Material, Ph.D.
642 Dissertation, Department of Civil engineering, McMaster University, Hamilton, Ontario,
643 Canada, pp. 99.
- 644 [56] Xu, W., and Chen, H. (2012). Microstructural modelling of cement-based materials via
645 random packing of three-dimensional ellipsoidal particles. *Procedia Engineering*, vol. 27,
646 pp. 332-340.
- 647 [57] Gallucci, E., Mathur, P., and Scrivener, K. (2010). Microstructural development of early
648 age hydration shells around cement grains, *Cement and Concrete Research*, vol. 40, pp. 4–
649 13.
- 650 [58] Thomas, J. J., Biernacki, J. J., Bullard, J. W., Bishnoi, S., Dolado, J. S., Scherer, G. W.,
651 and Luttge, A. (2011). Modeling and simulation of cement hydration kinetics and
652 microstructure development. *Cement and Concrete Research*, vol. 41, pp. 1257–1278.
- 653 [59] Diamond, S. (2004). The microstructure of cement paste and concrete—a visual primer,
654 *Cement & Concrete Composites*, vol. 26, pp. 919–933.
- 655 [60] Merzouki, T., Bouasker, M., Khalifa, N. E. H., and Mounanga, P. (2013). Contribution
656 to the modeling of hydration and chemical shrinkage of slag-blended cement at early age,
657 *Construction and Building Materials*, vol. 44, pp. 368–380.
- 658 [61] Bentz, D. P. (2008). Virtual Pervious Concrete: Microstructure, Percolation, and
659 Permeability, *ACI Materials Journal*, vol. 105(3), pp. 297-301.
- 660 [62] Bentz, D. P. (2000). Influence of silica fume on diffusivity in cement-based materials II.
661 Multi-scale modeling of concrete diffusivity, *Cement and Concrete Research*, vol. 30, pp.
662 1121–1129.
- 663 [63] Roy, D. M. (1993). Concrete microstructure, Strategic Highway Research Program
664 (SHRP), National Research Council, Washington, DC 1993.

- 665 [64] Thomas, J. J., Biernacki, J. J., Bullard, J. W., Bishnoi, S., Dolado, J. S., Scherer, G. W.,
666 and Luttge, A. (2011). Modeling and simulation of cement hydration kinetics and
667 microstructure development. *Cement and Concrete Research*, vol. 41, pp. 1257–1278.
- 668 [65] Pommersheim, J. M., and Clifton, J. R. (1979). Mathematical modeling of tricalcium
669 silicate hydration, *Cem. Concr. Res*, vol. 9, pp. 765–770.
- 670 [66] Parrot in 2796: Parrot, L. J., and Killoh, D. C. (1984). Prediction of cement hydration,
671 *Br. Ceram. Proc.*, vol. 35, pp. 41–53.
- 672 [67] Tomosawa, F. (1997). Development of a Kinetic Model for Hydration of Cement, in: H.
673 Justnes (Ed.), *Proceedings of the Tenth International Congress on the Chemistry of*
674 *Cement*, Göteborg, Sweden.
- 675 [68] Tenoutasse, N., and De Donder, A. (1970). The kinetics and mechanism of hydration of
676 tricalcium silicate, *Silicates Ind.*, vol. 35, pp. 301–307.
- 677 [69] Brown, P. W., Pommersheim, J. M., and Frohnsdorff, G. (1985). A kinetic model for the
678 hydration of tricalcium silicate, *Cem. Concr. Res.*, vol. 15, pp. 35–41.
- 679 [70] Gartner in 2796: Gartner, E. M., and Gaidis, J. M. (1989). Hydration Mechanisms, I, in:
680 J.P. Skalny (Ed.), *Materials Science of Concrete*, American Ceramic Society, Westerville,
681 OH, pp. 95–125.
- 682 [71] Cahn, J. W. (1956). The kinetics of grain boundary nucleated reactions, *Acta Metall.* vol.
683 4, pp. 449–459.
- 684 [72] Thomas, J. J. (2007). A new approach to modeling the nucleation and growth kinetics of
685 tricalcium silicate hydration, *J. Am. Ceram. Soc.*, vol. 90, pp. 3282–3288.
- 686 [73] Frohnsdorff, G. J. C., Freyer, W. G., and Johnson P. D. (1968). The Mathematical
687 Simulation of Chemical, Physical and Mechanical Changes Accompanying the Hydration
688 of Cement, *5th Int. Congr. Chem. Cem.*, Tokyo, vol. 2, p. 321.

- 689 [74] Van Breugel, K. (1995). Numerical simulation of hydration and microstructural
690 development in hardening cement paste (II): applications, *Cem. Concr. Res.*, vol. 25, pp.
691 522–530.
- 692 [75] Bentz, D. P., and Garboczi E. J. (1991). A digitized simulation model for microstructural
693 development, *Ceram. Trans.*, vol. 16, pp. 211–226.
- 694 [76] Bullard, J. W. (2007). A three-dimensional microstructural model of reactions and
695 transport in aqueous mineral systems, *Modell. Simul. Mater. Sci. Eng.*, vol. 15, pp. 711–
696 738.
- 697 [77] Navi, P., and Pignat, C. (1996). Simulation of cement hydration and the connectivity of the
698 capillary pore space, *Advanced Cement Based Materials*, vol. 4, pp. 58–67.
- 699 [78] ASTM (2012). Standard Specification for Portland Cement. ASTM C150/150M, pp. 9.
- 700 [79] Mehta, P. K. (2009). Global Concrete Industry Sustainability. *Concrete International*,
701 vol. 31(2), pp. 45–48.
- 702 [80] Russ, J. C. (1986). *Practical Stereology*, Springer, pp. 196.
- 703 [81] Marsh B. K. (1984). Relationships between engineering properties and microstructure
704 characteristics of hardened cement paste containing pulverized fuel ash as a partial cement
705 replacement, PhD thesis, The Hatfield Polytechnic, UK.
- 706 [82] Farjas, J., and Roura, P. (2006). Modification of the Kolmogorov-Johnson-Mehl-Avrami
707 rate equation for non-isothermal experiments and its analytical solution, *Acta Materialia*,
708 vol. 54(20), pp. 5573–5579.
- 709 [83] Mirzahosseini, M.R., and Riding, K. A. (2014). Effect of Curing Temperature and Glass
710 Type on the Pozzolanic Reactivity of Glass Powder, *Cement and Concrete Research*, vol.
711 58, pp. 103–111.