

1 Article

# 2 Increase efficiency of horizontal subsurface flow 3 wetlands by means of innovating of the effluent 4 capture and evacuation device

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10

## 11 Abstract:

12 The aim of this work is to evaluate the impact of proposed modifications to conventional capturing  
13 and discharging effluents devices in Subsurface Flow Wetland (SSFW) systems. Main modifications  
14 proposed consist on extending the influence of the capture and discharge device in such a way that  
15 the SSFW width and height are fully covered. Additionally an SSFW prototype was built as pilot  
16 including the proposed modifications and the impact of the innovative device is measured based on  
17 the efficiency of the Chemical Oxygen Demand (COD) removal as compared to a traditionally built  
18 SSFW.

19 The results show that for the innovative device, the COD removal was 10% higher than for the  
20 conventional device.

21 **Keywords:** Artificial Wetlands, Horizontal Wetland, Subsurface Flow

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## 23 1. Introduction.

24 The aim of this work is to evaluate the impact on the SSFW behaviour by incorporating an innovative  
25 device in the capture and exit of the effluents from horizontal subsurface wetlands. Said device was  
26 installed in a pilot-scale wetland and in a real-scale wetland. As a consequence, there was an increase  
27 in the efficiency of organic matter removal from domestic wastewater, at a low cost of investment,  
28 operation and maintenance, and was complying with the water quality standards required by the  
29 current regulations of the country.

30 It is considered that the water is contaminated, when their chemical, physical and biological  
31 characteristics or composition has been altered, this is, it loses their potability for daily consumption  
32 or for its use in domestic, industrial or agricultural activities, which generates wastewater [1]. This  
33 statement applies regardless of domestic, industrial, agricultural, or rainwater origin [2].

34 Wastewater generated in human activities has a high load of organic material. Additionally, it  
35 contains toxic substances and inorganic matter in small quantities and, as a consequence, the sum of  
36 both components pollute water sources undermining the sustainability of water provision and  
37 consequently, the sustainability of the humanity itself. Therefore, treatment systems including  
38 physical, chemical and biological processes have been developed and widely implemented. The  
39 objective of such processes is to reduce the load of pollutants from wastewater and, ideally, to  
40 recover, recycle and reuse them before pouring it into bodies of surface water [3].

41 In the other hand, wastewater also contains useful products, such as water, organic matter, some salts  
42 and also some harmful products. The latter must be separated from useful products that could benefit  
43 the population [4]. Efficient treatment systems have been developed for the removal of pollutants,  
44 which are also economically, technically and socially feasible. One example of those treatments is the  
45 Artificial Wetlands of SubSuperficial Flow (WSSF), [5]. In the latter, it has been proposed that  
46 contaminants removal levels can be increased by modifying the design of the input geometry of the  
47 wetland or by modifying the form of distribution of the flow and its direction within the system [6].

48 Artificial wetlands can efficiently reduce the Biochemical Oxygen Demand (BOD), Total Suspended  
49 Solids (TSS), achieving adequate treatment levels with low energy consumption and simple and  
50 economic maintenance procedures [7]. However, the rate of organic matter biodegradation is lower,  
51 requiring typically 20 to 50 times more land area than in conventional systems [8].

52 In artificial wetlands, soluble organic compounds are biodegraded by aerobic processes where  
53 oxygen is supplied directly from the atmosphere by diffusion and mainly through the process of  
54 photosynthesis, into the water column [9]. Microorganisms that are attached to the support medium  
55 in subsurface flow systems are those that biodegrade the soluble organic compounds [10]. The  
56 degradation rate is typically 10 times faster than anaerobic processes [11]. On the other hand, aerobic  
57 processes are the main mechanism to reduce soluble BOD, and the elimination of particulate BOD  
58 occurs rapidly by sedimentation and particle filtration in the spaces between gravel and roots [12].

59 The structural factors that affect the removal of organic matter are related to the depth of the wetland,  
60 which in turn is conditioned by the plant's root depth, depending directly on the species of plant  
61 used. The most commonly used plant species are emergent macrophytes, typical of humid areas such  
62 as reed (*Phragmites* sp.), Bulrush (*Typha* sp.) or reeds (*Scirpus* sp.) [13]. Emerging plants such as  
63 cattails and reeds, roots depth will be less than 60 cm. The vegetation provides surfaces for the  
64 formation of bacterial films, facilitating the filtration and adsorption of pollutants from wastewater  
65 and controlling the growth of algae by limiting the penetration of sunlight [7].

66 These plants show great adaptation to saturated environments, fast growth, strength and resistance  
67 to climatic changes, and also they do not constitute a source of food for animals [14]. One criterion  
68 for plants selection is the adaptability to the environmental conditions where a wetland is planned to  
69 be built, for this reason, local flora species are preferred [13].

70 Hence, in wetlands feeding is continuous and the water cross horizontally a filtering substrate  
71 composed by gravel, following its course by the effect of gravity, given the smooth slope on the  
72 bottom towards the exit of the wetland. This arrangement allows the contact between the residual  
73 water, the substrate and the plant's roots with the hydraulic retention time ranging from 2 to 5 days.  
74 An impermeable barrier is considered in order to confine the residual water and avoid groundwater  
75 contamination. This barrier is required to be resistant, smooth and protected against puncturing by  
76 sharp gravel [15]. The most used waterproofing material is high-density polyethylene. Regarding the  
77 filtering substrate, it is recommended to use gravel of ASTM 11/4" to ASTM 3/4", the diameter effect  
78 over the system can be summarized as follows: larger diameters increase water speed, whereas small  
79 diameter, reduce the speed causing possible floods and preferential flows [16].

80 The ratio (length: width) must be greater than (3: 1) to approximate a piston-type flow, which is  
81 directly related to the slope used at the bottom of the wetland bed, which determines the flow speed  
82 [17]. The most common range for the slope goes from 0.5 to 1%. [18].

83 In terms of modeling the system dynamics, the Basic Model of Organic Matter Removal is applied in  
 84 piston flow reactors [19]. This model has been validated [20] and relates the contaminants removal  
 85 capacity and the hydraulic residence time.

86 Eq. 1 
$$\frac{dCa}{dt} = K_T * C$$

87 Eq. 2 
$$\frac{C_e}{C_o} = \exp(-K_T * HRT)$$

88 Eq. 3 
$$HRT = \frac{V}{Q} = \frac{A_s * h * n}{Q}$$

89 Eq. 4 
$$A_s = Q * \frac{\ln \frac{C_o}{C_e}}{K_T * \gamma * n}$$

90 Table 1 Parameters of design

Co	Concentration of BOD in influent, mg / l
Ce	Concentration of BOD in effluent, mg / l
HRT	Hydraulic residence time, day
As	Surface area of the wetland, m <sup>2</sup>
N	Porosity of the wetland
Y	Depth of water in the wetland, m
Q	Average flow rate of the wetland, m <sup>3</sup> / day
V	Volume of the wetland, m <sup>3</sup>
K <sub>T</sub> , (1/d)	Constant dependent on temperature, = K <sub>20</sub> * 1,06 <sup>T-20</sup>
D	day

91 K<sub>20</sub> = 1.104 d<sup>-1</sup> Constant kinetics of organic matter removal at 20°C.

## 92 2. Materials and Methods

93 Both types of capture and exit devices were installed in two different wetlands, the first a pilot-scale  
 94 wetland, located in dependencies of the University of Bío Bío, Campus Concepción, in the city of  
 95 Concepción, and the other, in the subsurface flow wetland of Recreational Center Ainahue, located  
 96 in Hualqui, province of Concepción, whose coordinates are U.T.M. 686393.79 m E; 5905081.35 m S  
 97 (Figure 1).

### 98 Pilot Wetland:

99 Two horizontal subsurface flow wetlands of dimensions 2.0 x 0.6 m (Table 2) were built, one of them  
 100 using the proposed modifications in the capture and evacuation effluent device and the other using  
 101 the conventional device. Both were connected to the same pond, which provided the synthetic  
 102 wastewater.

### 103 Real-scale Wetland:

104 In the constructed wetland of Recreational Center Ainahue, conventional and innovative device were  
 105 used alternately, to analyze the behavior of the wetland, based on them.  
 106

107 The samples were taken during a period of three weeks approximately, while using the innovative  
 108 device. Then we proceeded to use the traditional device. During the first seven days of operation of  
 109 the device no samples were taken, so that the wetland would adapt to the hydrodynamics change.  
 110 After this pause, sampling was started for the conventional device, which was also during a period  
 111 of three weeks.  
 112



113

114 **Figure 1.** Wetlands of horizontal subsurface flow. (Left) Pilot wetlands, located in dependencies of  
 115 the University of Bío Bío, Concepción. (Right) Real-scale wetland, located in the Recreational Center  
 116 Ainahue, Hualqui.

117

Table 2. Dimensions of Wetland Sub-surface Horizontal Flow.

Parameter	Symbol	Pilot Wetland Characteristics	Real Wetland Characteristics
Flow (m <sup>3</sup> /day)	Q	2	48
Length (m)	L	2	45
Width (m)	W	0.6	13
Length / width ratio	L/W	3.33	3.46
Depth (m)	Y	0.55	0.6
Porosity Dry gravel (%)	N	0.42	0.38
Slope (m/m)	S	0.02*	0.05
Surface Area (m <sup>2</sup> )	As	1.2	585
Transverse Area (m <sup>2</sup> )	Ac	0.033	7.8
Hydraulic Gradient (m/m)	G	0.02	0.05
Vegetation		Typha	Typha

118 *Physical-Chemical Parameters and Analytical Methods*

119 • Chemical Oxygen Demand (COD)

120 The potassium dichromate method was used to evaluate COD levels. This method is a variation  
 121 of the standard method [21], however, it maintains the basis of it. The variation used has the  
 122 advantage that it requires a smaller sample and reagents. The sample is chemically oxidized through

123 the action of potassium dichromate at 150 °C for two hours. Silver sulfate is used as a catalyst and  
124 mercury sulfate to avoid possible interferences with chloride. Afterwards, determination by  
125 spectrophotometry at 600 nm is performed. Equipment and instruments were used to determine the  
126 various parameters to characterize the wastewater.

### 127 *Determination of Chemical Oxygen Demand (COD)-Substrate Relationships*

128 Samples composed by mixtures of water and substrates prepared at different concentrations,  
129 and their respective COD was estimated. This test is performed in order to produce a calibration  
130 curve and establish the ratio substrate concentration/COD.

### 131 *Experimental Methodology*

#### 132 Pilot Wetland:

##### 133 a. Feed Preparation

134 This pilot wetland was initially fed with synthetic wastewater prepared in the laboratory  
135 according to the typical characteristics of urban wastewater [22]. This wastewater has an  
136 approximate COD of 200-300 mg/L, with the corresponding proportions of nitrogen and  
137 phosphorus, in a relation of COD:N:P = 100:5:1. Approximately 200-300 mg of saccharose, 10-15  
138 mg of phosphate hydrogen of potassium, and 50-75 mg of ammonium chloride were added per  
139 liter of water.

##### 140 b. Operation Mode

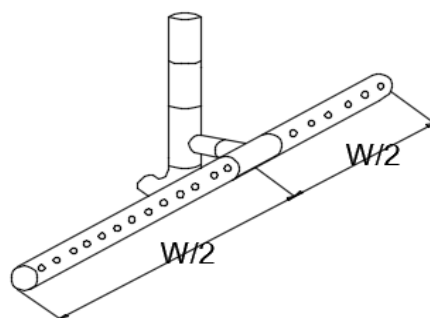
141 The synthetic wastewater was poured into a storage pond of almost 1000 L, Process effluent is  
142 collected in a 30 L volume tank, where the samples are taken to be processed. The flow of synthetic  
143 wastewater is 2 m<sup>3</sup>/day.

144

### 145 **Description of conventional and innovative output devices.**

#### 146 Conventional Exit Device

147 The conventional device consists of a PVC pipe 90 mm in diameter and 13 m in length with  
148 perforations of approximately 10 mm along its length, for the capture of the effluent (Figure 2). It is  
149 located approximately 0.2 m from the bottom of the wetland. The collection of the effluent water is  
150 done with a perforated pipe settled on the bottom of the wetland. Then, it is directed towards the exit  
151 by means of a syphon, which allows to maintaining the water level inside the wetland.



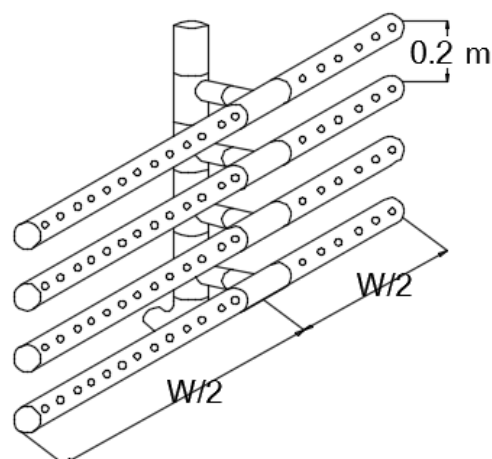
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153

**Figure 2.** Conventional outlet device for the effluent of the subsurface flow wetland.

154 Description of the Innovative device

155 The innovative exit device of the artificial wetland, consists of 4 sanitary PVC pipes 90 mm in  
 156 diameter and 13 m long, located at different heights, in climbing form at 0.15 m and 0.2 m from the  
 157 bottom of the wetland, with 10 mm perforations in diameter (Figure 3).



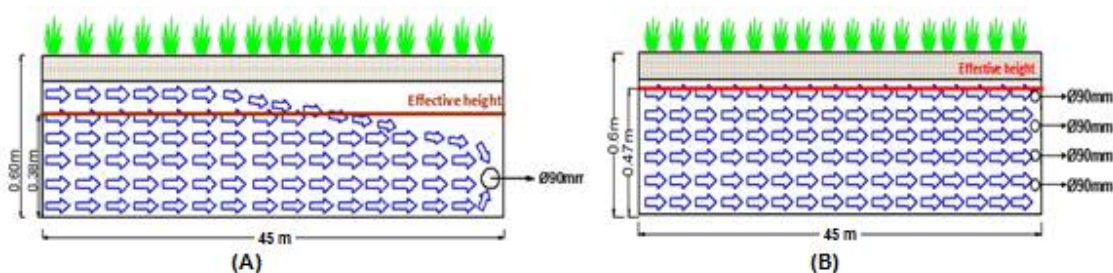
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159 **Figure 3.** Innovative outlet device for the effluent of the subsurface flow wetland, Patent  
 160 Registration Number: 503, INAPI, Chile, February 2018 [23].

161 Given the structure of the conventional device, the capture of the effluent occurs in the bottom (figure  
 162 4a), unlike the innovative device that the effluent flows in the entire water column (figure 4b). The  
 163 innovative device has an effective height higher than that of the conventional device, since it has a  
 164 greater effective volume, due to the fact that it has a smaller lost volume associated with the  
 165 generation of preferential flows.

166

167

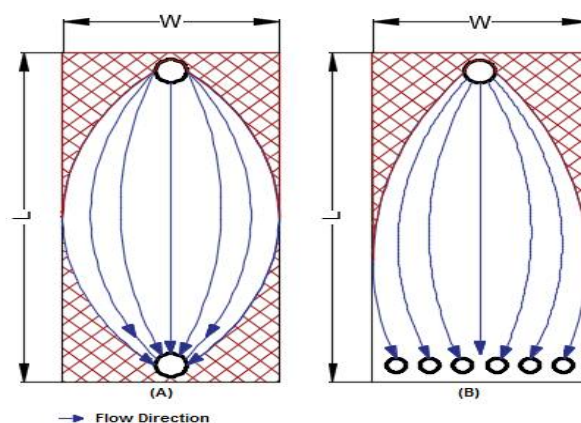


168

169 **Figure 4.** Effective height with (A) the conventional device and (B) the innovative device.

170 Something similar happens with the occupation of the wetland area, for the innovative device, the  
 171 effluent is collected throughout the width of the wetland, which minimizes the area lost (Figure 5).





172  
173  
174

Figure 5. Effective area with (A) the conventional device and (B) the innovative device.

175 Sampling and operation of the constructed wetland

176 Effluents samples from the artificial wetland, as shown in Figure 6, were sent periodically to  
177 laboratory analysis to measure the chemical oxygen demand (COD) and total suspended solids (TSS),  
178 using the standardized method. In parallel, the flow was estimated.



179  
180

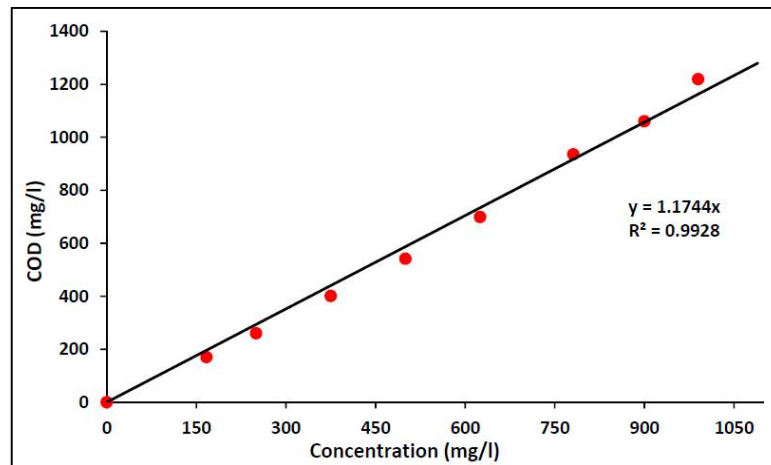
Figure 6. Effluent and affluent sample

### 181 3. Results

#### 182 3.1 COD-Substrate Relationships

183 From the experimental results, a straight line regression with a slope of 1.17 is obtained, as  
184 shown in Figure 7, from which it can be stated that the saccharose has one COD per gram, which is  
185 above other organic substances [24]. The model obtained is:  $Y = 1.1744X$ .

186



187

188

Figure 7. COD-saccharose relationship.

189 3.2 COD concentration of the artificial wetland

190

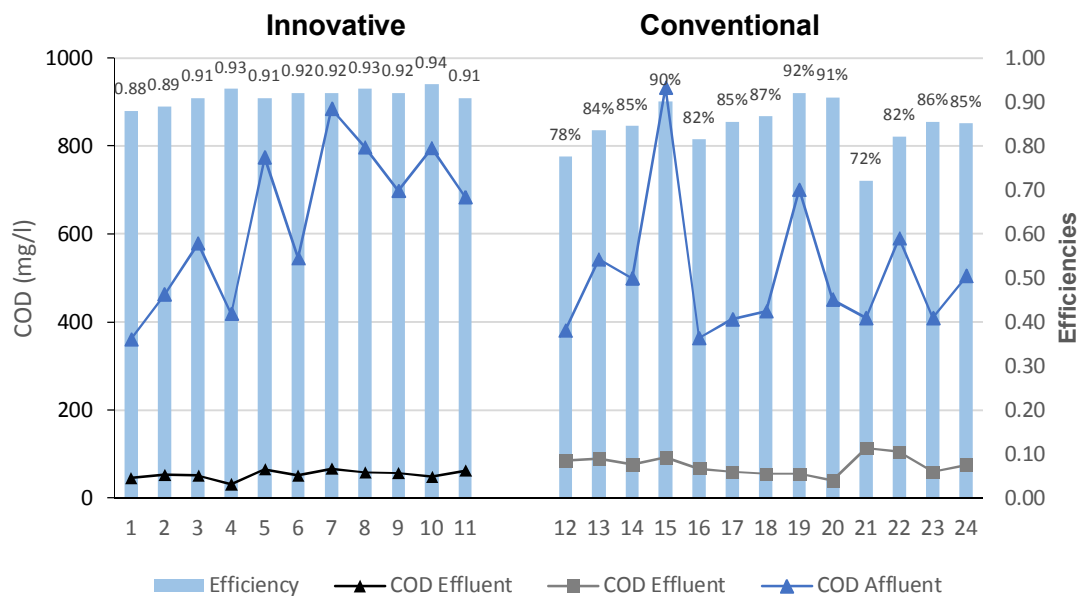
191 Figure 8 and Figure 9 shows the input and output concentrations of organic matter, using  
 192 the conventional and innovative effluent capture and evacuation device. Using the COD as  
 193 an assay, we estimate the abatement efficiency that is reached in the wetland, obtaining  
 194 average efficiencies for the innovative and conventional devices of 92% and 84%  
 195 respectively in the full-scale wetland. For the case of pilot wetlands, the efficiencies obtained  
 196 were 69% and 63% respectively. Therefore, a better performance is demonstrated for the use of the  
 197 innovative device, the percentage increase is 10% for both cases.

198 Real scale =  $(92-84)/84 = 0.1$

199 Pilot Scale =  $(69-63)/63 = 0.1$

200

201



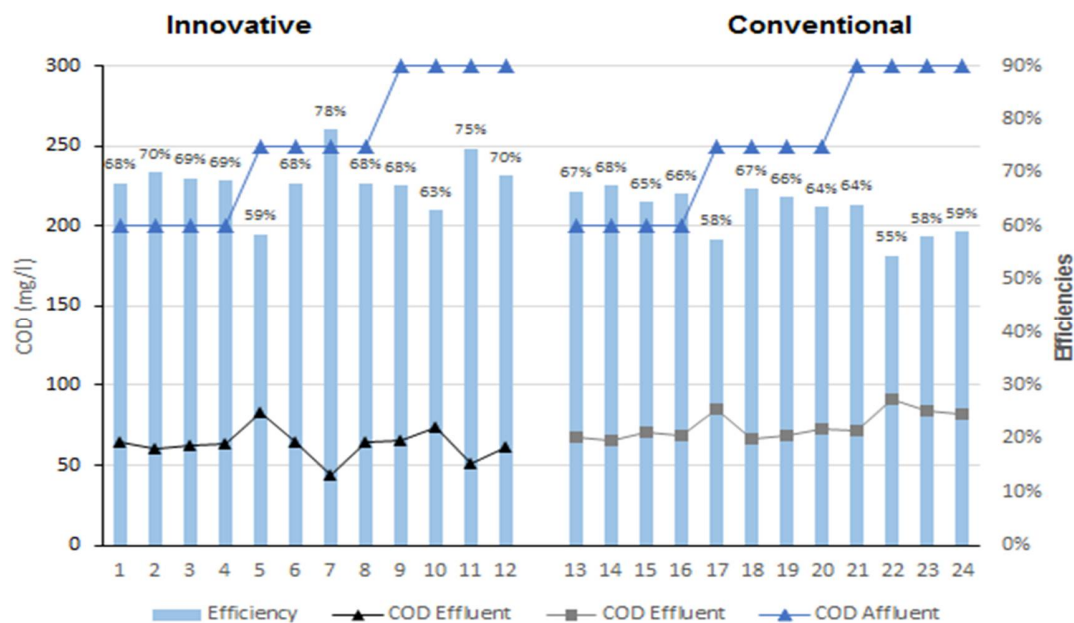
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**Figure 8.** COD concentration in effluent and effluent in a Real-scale wetland.

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207

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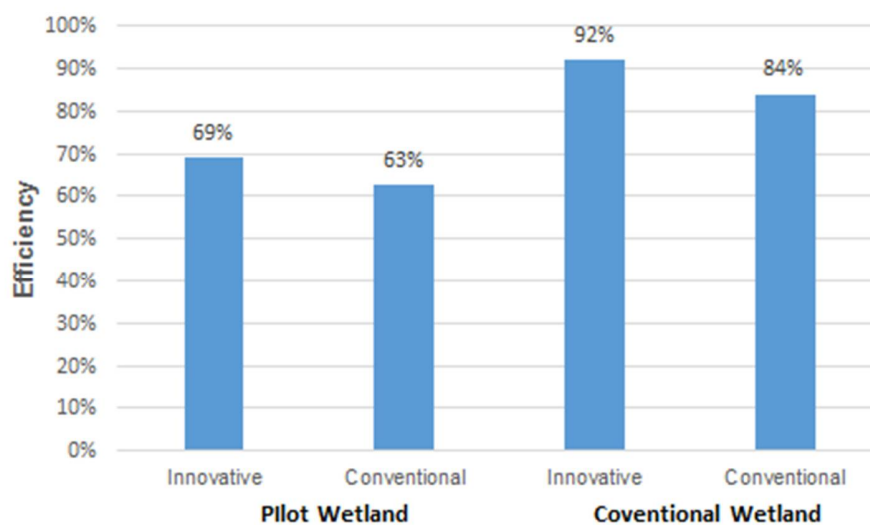
**Figure 9.** COD concentration in affluent and effluent in a Pilot-scale wetland.

## 3.3 Yields of the wetland with both devices.

210

Figure 10 shows the yields of the removal of organic matter in the horizontal subsurface flow wetland during the start-up period, with the innovative and conventional device.

212



213

214

**Figure 10.** Average efficiencies of both devices in the removal of organic matter.

215 The previous graph show that in both cases, real and pilot wetland, when the innovative device is  
 216 used, efficiencies of COD with better performance are obtained, exceeding the conventional device  
 217 performance by 6% and 8% respectively.

218 The yields with the innovative device, allow obtaining better quality effluents with shorter residence  
 219 times, therefore it is possible to reduce the extension of a wetland for the same treatment horizon,  
 220 which is based on the efficiency of the innovative device of a 92%, greater than 84% of the  
 221 conventional.

222 The lower efficiency of the conventional device is attributed to the uniqueness and location, which  
 223 causes the occurrence of preferential flows, leaving a volume with very little water movement,  
 224 generating a decrease in both the height and effective volume of the wetland.

225  
 226 On the other hand, with the innovative device, having 4 equidistant catchment outlet pipes, it tends  
 227 to generate a uniform flow that integrally occupies the cross-sectional area, using an effective height  
 228 closer to the design height of the wetland.

### 229 *Student's t-test analysis*

230 A Student's test analysis was made to compare the efficiency results between conventional and  
 231 innovative device and verify if there are significant differences between them, both for pilot and real  
 232 wetland.

233 We worked based on an alpha of 0.05 and under the following hypothesis:

234 •  $H_0 = 0$

235 It indicates that there are no significant differences between the results of the COD removal  
 236 efficiencies of the effluent from the wetland innovative with the wetland conventional.

237 •  $H_1 \neq 0$

238 It indicates that there are significant differences between the results of the COD removal efficiencies  
 239 of the effluent from the wetland innovative with the wetland conventional both for pilot and real  
 240 wetland.

241

242 By using Microsoft Excel, the t-Students analyses of the efficiency results are shown in Table 3.

243 Table 3. T-Students analysis results.

T-Test for mean of two paired samples				
Parameters	Pilot-scale Wetland		Real-scale Wetland	
	Innovative	Conventional	Innovative	Conventional
Mean	0.69	0.63	0.92	0.84
Variance	0.0026	0.0019	0.00028	0.0035
Observations	12	12	12	12
Pearson Correlation Coefficient	0.43		0.22	
Hypothetical difference of the mean	0		0	
Freedom Degrees	11		11	
Statistical t	3.87		4.43	
P(T<=t) one-tailed	0.0013		0,0051	

Critical value of t(one-tailed)	1.80		1.80	
P(T<=t) two-tailed	0.0026		0.0010	
Critical value of t (two-tailed)	<b>2.20</b>		<b>2.20</b>	

244

245 For the case of the pilot-scale wetland, as the absolute value of the statistic  $t(3.87)$  is higher than the  
 246 critical value of  $t$  two-tailed (2.20) and  $P$  is smaller than 0.025, the null hypothesis is rejected and  
 247 approves  $H_1$ , that is, there are significant differences between the results of the COD removal  
 248 efficiencies of the effluent from the wetland innovative with the wetland conventional.

249 Furthermore, for the real-scale wetland, as the absolute value of the statistic  $t(4.43)$  is higher than the  
 250 critical value of  $t$  two-tailed(2.20) and  $P$  is smaller than 0.025, the null hypothesis is rejected and  
 251 approves  $H_1$ , that is, there are significant differences between the COD results for the effluent from  
 252 the wetland innovative with the wetland conventional.

#### 253 4. Discussion

254 Artificial wetlands have been validated as an alternative wastewater treatment option to the  
 255 conventional systems, shown very good results in different experiences, for example:  
 256 [25] conducted a nine-month campaign for a horizontal subsurface flow wetland, which treats rural  
 257 wastewater in the Cova Beira region. Initially, the concentrations in the influent were 506 mg / L of  
 258 BOD AND 677 mg / L of COD and the concentrations in the effluent for BOD and COD were 87 mg  
 259 / L and 222 mg / L respectively.

260 The wetland presented a high load removal, where the average efficiencies were 83% for BOD and  
 261 68% for COD. [26] it studied the application of halophytic plants in a horizontal subsurface flow  
 262 wetland constructed for the treatment of domestic wastewater. The pilot plant located in Greece was  
 263 planted with a polycropping of halophytes (*Tamarix parviflora*, *Juncus acutus*, *Sarcocornia perrenis*  
 264 and *Limoniastrum monopetalum*). The results show that the halophytes were successfully developed  
 265 in the constructed wetland, where, the average BOD concentration of 106 mg / L in the influent was  
 266 reduced to 39 mg / L in the effluent; with an average elimination of approximately 63% it obtained  
 267 removal efficiency for COD of 58%.

268 The COD elimination efficiencies of the mentioned experiences are similar to those of the pilot  
 269 wetlands that have COD elimination efficiencies of 63% with conventional device and 69% with  
 270 innovative device, with the application of the innovative patented device, the efficiencies could be  
 271 improved respective, the Cova Beira Wetland of 68 to 75% and the wetland of Greece from 58 to 64%,  
 272 therefore there is a feasibility to improve the efficiency of COD removal of these systems, which can  
 273 be achieved by incorporating the innovative patented device.

274 On the other hand we have experiences of high efficiency of elimination of COD. [27] it studied the  
 275 percentage of removal of the organic load of wastewater from a residential building that were treated  
 276 with artificial wetlands, the sampling carried out during 25 days in the low season, the initial  
 277 concentration was 164 mg / L, and after passing through the system, it was 7 mg / L, which means a  
 278 96% removal. For the the rainy season, the initial concentration in the residual water was 306 mg / L  
 279 and at the exit of the system, 30 mg / L, achieving a 90% removal.

280 [28] Evaluated 18 artificial subsurface flow wetlands planting *Stipa ichu*. Six of the wetlands were  
 281 assembled without plants and twelve of them with plants, for the construction they used rectangular  
 282 plastic containers with measures of 13 cm in height, 33 cm in length and 26 cm in width, and with a  
 283 hole in the lower part that it collected the effluent. During a period of 10 days of follow-up and with  
 284 a hydraulic residence time of 35 hours, the COD removal efficiency of domestic wastewater was

285 92.43% for wetlands without plants and 95.5% for wetlands with plants. [29] evaluated two wetlands  
286 with soil biotechnology plants (SBT). The different plants were classified as Plant I and Plant II. Plant  
287 I was controlled for a period of 12 months and an average COD of 266 mg / L was observed in the  
288 influent, while the value of the effluent was reduced to 32 mg / L, indicating 87% elimination  
289 efficiency.

290 The COD elimination efficiencies of the mentioned experiences are similar to those of the real  
291 wetlands that have COD elimination efficiencies of 85% with conventional device and 92% with  
292 innovative device, with the application of the innovative patented device, the efficiencies could be  
293 improved, the Residential Building and the wetland with *Stipa ichu* approach the 100%, wetlands  
294 with soil biotechnology plants from de 87 a 95%; therefore, there is a feasibility to improve the  
295 efficiency of COD removal of these systems, which can be achieved by incorporating the innovative  
296 patented device.

297 That is why we have worked on a device that ensures reaching the efficiency values of the high  
298 elimination range, since the last experiences is all very close or over the 90%, thus the differential  
299 shown in quality treatment of constructed wetland with the innovative device indicates that with this  
300 improvements, tend to achieve in elimination efficiency of COD terms, the highest values.

301 The following experiences show the application of constructed wetlands to wastewater of different  
302 nature to sewage, such as composting leachate, landfill leachate and wastewater from the  
303 pharmaceutical industry, which are more difficult to biodegrade than a domestic wastewater and  
304 achieving reasonable elimination results on the order of 74.5% of BOD and 53.7% of COD, for its  
305 application in composting leachates. In Isfahan, organic matter was removed from the leachate  
306 produced in the composting facility. The study was carried out in two horizontal flow wetlands with  
307 the dimensions of 1.5m x 0.5m x 0.5m. One of them was planted with *Vetiveria zizanioides* and the  
308 other wetland remained as control, without planting. They were operated with a leachate flow rate  
309 of 24 L / d for more than five months. The control wetland eliminated 21.8% of BOD<sub>5</sub> and 26.2% of  
310 COD and the planted wetland eliminated 74.5% of BOD<sub>5</sub> and 53.7% of COD [30]. The removal  
311 efficiencies of two horizontal subsurface flow wetlands were also investigated by [31]. One of  
312 downflow (F1) and the other of upflow (F2), both filled with the hybrid substrate zeolite-slag for the  
313 treatment of leachates in rural landfills. The results showed that constructed wetlands were able to  
314 eliminate the following range of COD, 20.5-48.2% (F1) and 18.6-61.2% (F2). [32] They applies an  
315 artificial subsurface flow wetland for the treatment of wastewater from a cosmetic and  
316 pharmaceutical industry, using a system of rooted emergent macrophytes (*Cyperus papyrus*) for the  
317 removal of organic loads, the initial concentration of 92 mg / L of BOD<sub>5,20</sub> is reduced to a  
318 concentration of 20 mg / L. The wetland showed a high efficiency in the removal of organic load of  
319 79% of BOD<sub>5,20</sub>.

320

321 The extension of the application of wetlands to different kinds of wastewater, reinforces the need to  
322 improve the efficiency of COD elimination and therefore use of the patent innovative device, in order  
323 to guarantee treatment efficiencies of 60% for all types of wastewater.

324

## 325 5. Conclusions

326 It is observed that the wetland with innovative device presents higher yields than those obtained  
327 with the conventional device. By obtaining higher yields with the innovative device, it allows  
328 achieving effluents of better quality, which is verified in that the performance of the innovative device  
329 has a COD removal efficiency of 92% being superior to the conventional device of 84%, for the case  
330 of the full-scale wetland.

331 The innovative device has a COD removal efficiency of 69% being superior to the conventional device  
332 of 63%, for the case of the pilot-scale wetland.

333 The t-Student statistical analysis to the results obtained from the pilot and real-scale wetlands,  
334 approved the hypothesis H1, that is, there are significant differences between the COD removal  
335 efficiencies from the innovative device with respect at the conventional device.  
336 The innovative device achieves an efficiency of 10% over the conventional device in both the pilot  
337 and real wetland.

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