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# Application of an automated discharge imaging system and LSPIV during typhoon events in Taiwan

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**Abstract:** An automated discharge imaging system (ADIS), a non-intrusive and safe approach, was developed for measuring river flows during flash flood events. ADIS consists of dual cameras to capture complete surface images in the near and far fields. Surface velocities are accurately measured using the Large Scale Particle Image Velocimetry (LSPIV) technique. The stream discharges are then obtained from the depth-averaged velocity (based upon an empirical velocity-index relationship) and cross-section area. The ADIS was deployed at the Yu-Feng gauging station in Shimen Reservoir upper catchment, northern Taiwan. For a rigorous validation, surface velocity measurements were conducted using ADIS/LSPIV and other instruments. In terms of the averaged surface velocity, all measured results were in good agreement with small differences, i.e., 0.004 to 0.39 m/s and 0.023 to 0.345 m/s when compared to those from acoustic Doppler current profiler (ADCP) and surface velocity radar (SVR), respectively. The ADIS/LSPIV was further applied to measure surface velocities and discharges during typhoon events (i.e., Chan-Hom, Soudelor, Goni, and Dujuan) in 2015. The measured water level and surface velocity both showed rapid increases due to flash floods. The estimated discharges from ADIS/LSPIV and ADCP were compared, presenting good consistency with correlation coefficient  $R = 0.996$  and normalized root mean square error  $\text{NRMSE} = 7.96\%$ . The results of sensitivity analysis indicate that components till ( $\tau$ ) and roll ( $\theta$ ) of the camera are most sensitive parameter to affect the surface velocity using ADIS/LSPIV. Overall, the ADIS based upon LSPIV technique effectively measures surface velocities for reliable estimations of river discharges during typhoon events.

**Keywords:** ADIS; LSPIV; surface velocity; discharge measurement; flash flood; typhoon event

## 1. Introduction

Accurate measurement of stream flows which play a critical role in hydrological processes is highly desirable for many practical applications (like flood management and reservoir operation) especially during typhoon events [1]. Typically, some gauging stations are established at upstream rivers to provide essential information for earlier warning. However, the surrounding environments can be dangerous and hardly accessible. Meanwhile, most flow measurements in current operation practice are intrusive and involve complex user-assisted procedures. Thus, flood discharge measurements are still rare and challenging.

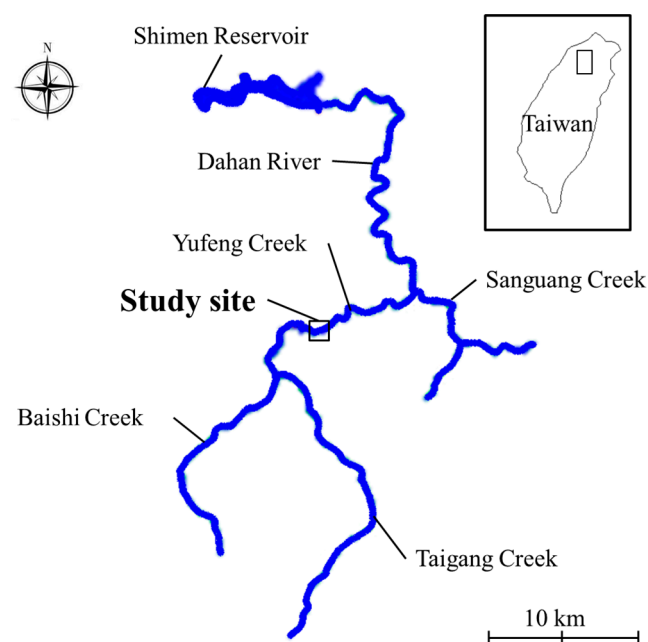
To date, both (i) intrusive and (ii) non-intrusive approaches have been developed to measure stream flows. Conventionally, river discharge measurement is conducted using current meters, dye concentrations, or acoustic methods (e.g., acoustic Doppler current profiler, ADCP). In combination with recorded water levels, a stage-discharge relationship (known as the rating curve) can be established [2]. Besides, recourse will be made to maintain the reliable rating curve for a natural river

(which often presents changing characteristics due to channel encroachment, dredging, weed growth, or other reasons). However, these intrusive approaches would hinder direct measurements under extreme weather, posing the concern of great uncertainties and questionable accuracy in extrapolated high flow rates [3].

In recent years, non-intrusive approaches based on radar (e.g., [4]) and/or imaging systems (e.g., [5-15]) have been increasingly applied, providing a safer alternative for measuring flood discharge. Without the needs of human-operated physical sampling and data acquisition, surface velocities are characterized from continuous images of observed (seeded fluid) flows mainly using two types of algorithms, i.e., particle tracers and image analyses (e.g., [16]). In the former method, the particle tracking velocimetry (PTV) returns one vector for each particle and is therefore more suitable for low seeding cases. In contrast, particle image velocimetry (PIV) used in the later one can track the average motion of a particle group within an image.

The large scale particle image velocimetry (LSPIV) originated from the lab-scale techniques is a promising tool for safely monitoring natural flows in different regimes. Recently, LSPIV has been successfully applied in various environmental studies, e.g., estimations of hydrodynamic characteristics in riverine environment [6,17] and flow patterns in limnological ecosystems [18]. The LSPIV also provides a novel hydrometric tool for gauged stations and ungauged sites in watershed. At a gauging station, LSPIV enables discharge measurement over a broader range of flow conditions than those traditional methods. Uncertainties of the extrapolated portion in a rating curve can be alleviated. On the other hand, at a ungauged site (without any hydrological data and rating curve), flow discharges can be obtained with reasonable uncertainties based upon hydraulic assumptions from in-situ characteristics [19].

The purpose of this study is to develop an automated discharge imaging system (ADIS) for estimating flash floods in a upper watershed based upon the dual-camera framework and LSPIV technique. With measured surface velocities, stream discharges are calculated from the depth-averaged velocities (using an empirical velocity-index relationship) and cross-section area. The ADIS was deployed at the Yu-Feng gauging station in the Shimen Reservoir catchment of northern Taiwan. Surface velocities and discharges during four typhoon events (i.e., Chan-Hom, Soudelor, Goni, and Dujuan) in 2015 were measured. To validate the newly developed ADIS, several conventional measurement were also conducted using the acoustic Doppler current profiler (ADCP) and surface velocity radar (SVR).

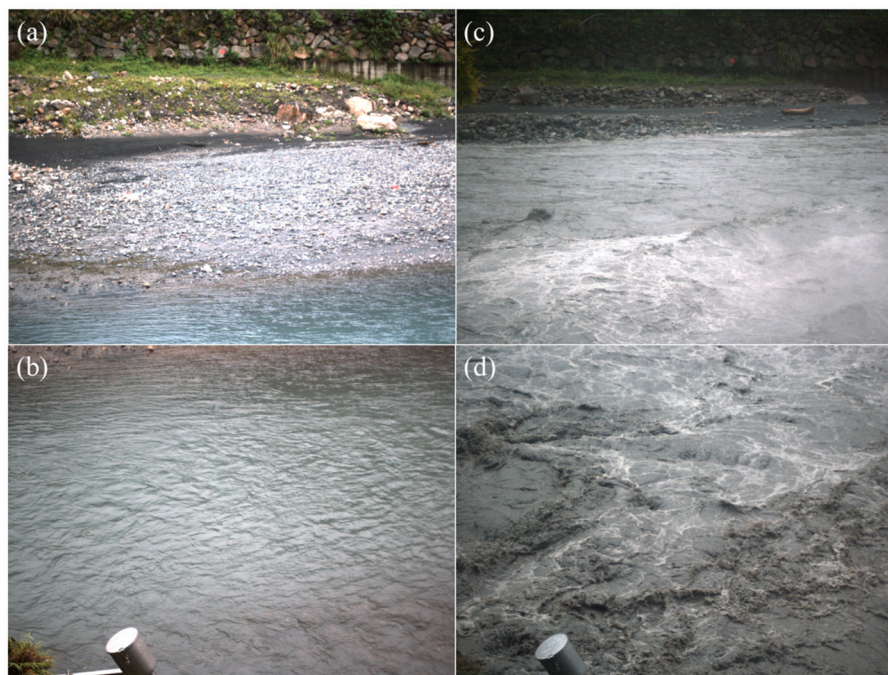


**Figure 1.** Location map of the study site. The Yufeng Creek is a upstream tributary of the Dahan River.

## 2. Materials and Methods

### 2.1. Description of the study area

The selected study site is the Yufeng gauging station with an elevation of 684 m, located at the upstream Yufeng Creek, upper catchment of Shimen Reservoir, northern Taiwan (Figure. 1). The river bed of Yufeng Creek is occupied with gravel and stone. The cross section (near the study site) has a width of 15~30 m with an average depth of 1.5 m at the normal flow condition. The Yufeng gauging station, maintained by the Northern Region Water Resources Office, Water Resources Agency, is equipped with an automated pressure sensor to measure water level. The dual-camera system (see the detailed description below) is installed at the platform of Yufeng gauging station. This platform is about at a height of 10 m above the river bed with a distance of 15~80 m to the observation area. Figure 2 displays some images obtained from the dual-camera system during normal flow condition and Typhoon Soudelor in August 2015. It can be seen that the exposed gravel area was inundated during typhoon event.



**Figure 2.** The far- and near-field photos obtained from the dual cameras: (a) and (b) for the normal flow condition; (c) and (d) for Typhoon Soudelor in August, 2015.

### 2.2. ADIS hardware system

For hardware setting, two industrial cameras (DMK 23G274 from The Imaging Source) are used in the ADIS. With progressive scanning CCD sensors, each camera can capture 1.92-megapixel (or  $1600 \times 1200$ -pixel) images at a frame rate of 20 FPS. In addition, real-time adjustments for proper light exposure and shutter speed under different conditions are controlled remotely using a server computer from the gauging station. The video image signals are directly transferred back to the Frame Grabbers (on computer) via a GigE cable connection.

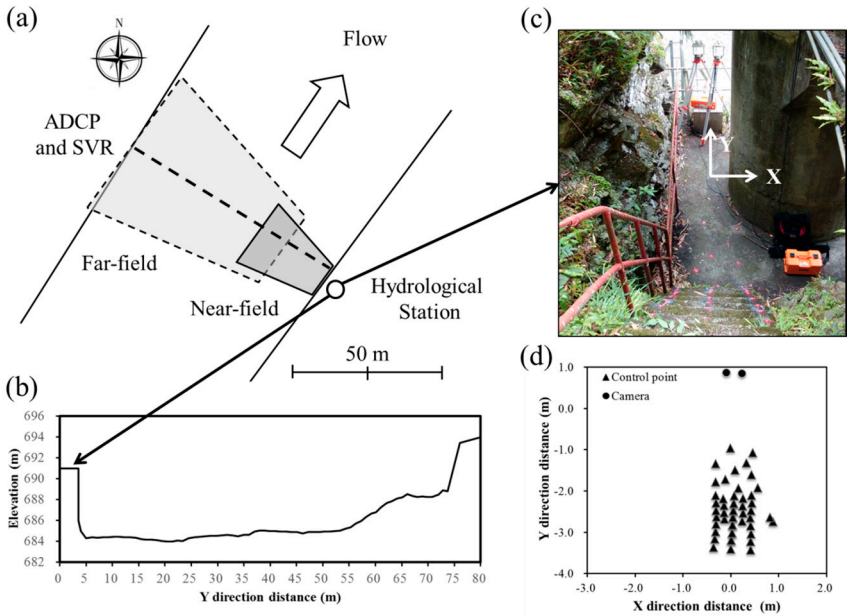
A dual-camera system (that consists of near- and far-field cameras) with 15-mm C-mount machine vision lenses was designed (see Figure 3a) to obtain high-resolution images over the entire cross section of Yufeng Creek. The near- and far-field regions are partly overlapped with areas of  $12\text{m} \times 20\text{m}$  and  $40\text{m} \times 50\text{m}$  (the flow direction  $\times$  cross-sectional direction), respectively. In the areas of interest, the image resolution is in the range of 4.35 (near-field camera) – 23.5 (far-field camera) mm/pixel.

The parameters of dual-camera system should be established and resolved using control points. Totally, 44 control points were arranged on the stairs at Yufeng station (see Figure 3b) and measured



by the total station (see the locations of control points and cameras in Figure 3c). The dual-camera system would turn horizontally between the control points and river channel (in positive and negative  $y$  directions, respectively) during the parameter calibration and flow measurement procedures.

To protect the hardware system, both cameras were enclosed in weatherproof cases which were also sealed with rubber gaskets to prevent water leaking (Figure 4). The enclosure cases were mounted on custom-made pan-tilt heads which were built on high-quality survey tribrachs. Thus, the cameras can be precisely rotated in the tilt (about vertical axis) and azimuth (about horizontal axis) directions for a desired image view (see the inset part in Figure 4).



**Figure 3.** (a) Illustration for the near-field and far-field camera view angles at Yufeng gauging station. Note that the ADCP and SVR data acquisition campaign along the river is also shown in the figure, (b) picture of the control points, (c) measured locations of control points in a horizontal plane during image calibration, and (d) cross section along the river (from right bank to left bank).



**Figure 4.** The ADIS hardware system consists of two cameras, both enclosed in the weatherproof cases.

### 2.3. ADIS software system

The ADIS software system for automatic measurement of surface velocities and discharges was developed using MATLAB language. This program mainly (i) solves camera parameters for image calibration, (ii) maps images to the physical coordinate system, (iii) measures surface velocities using LSPIV, and (iv) calculates discharge of the river by a velocity-index approach. The program of ADIS software can be used to calibrate cameras in the field and to capture images which are mapped to the physical coordinate system. The velocity field across the cross section can be extracted and calculated from images. Then the discharge is obtained according to velocity-index approach and velocity-area method. In the following, detailed LSPIV algorithms are given.

#### (i) Image calibration

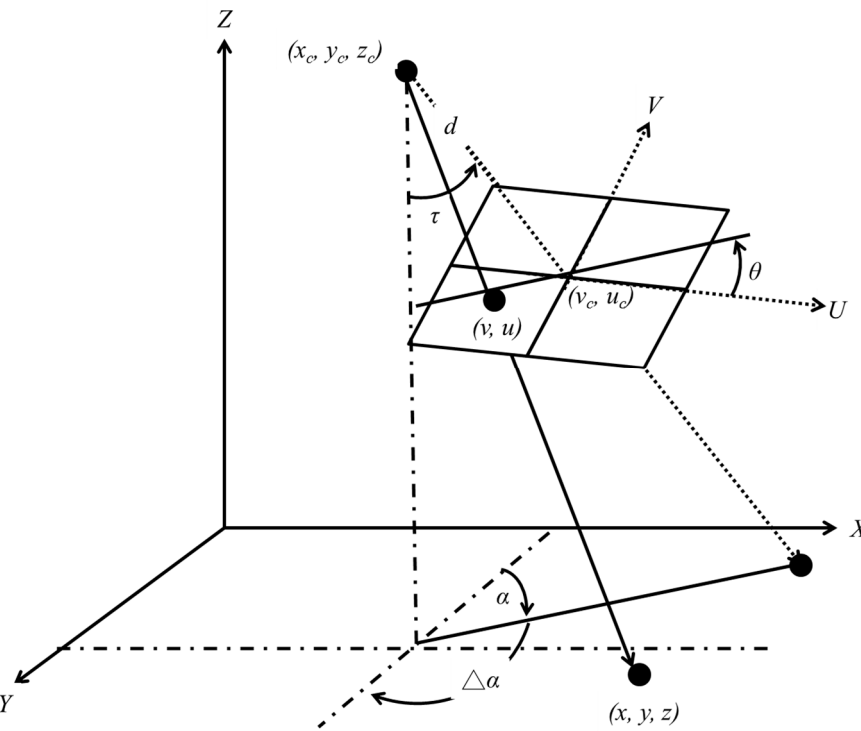
A two-step calibration procedure was applied before mapping the raw images to a physical coordinate. First, the so-called interior calibration was used to correct radial and tangential distortions (commonly appeared in the photographs from spherical-lens cameras). The distortion parameters are defined by the collinearity equations can be found in Wanek and Wu [20]. In Figure 5, the parameters,  $(u_c, v_c)$ , represent the center of the image;  $r$  is the distance from image center to an arbitrary point;  $(k_1, k_2)$  and  $(p_1, p_2)$  are the lens parameters of radial and tangential distortions, respectively.

Next, the exterior calibration based upon a pinhole camera model (see Figure 5) was adopted to determine a geometric relationship between an image point  $(u, v)$  and its physical position in space  $(x, y, z)$ . In Figure 5,  $d$  is the focal length of camera lens;  $x_c, y_c, z_c$  are the physical locations of the cameras; coefficients  $r_{ij}$  are rotational angles (i.e., components of azimuth  $\alpha$ , tilt  $\tau$ , and roll  $\theta$ ) of the camera [21]. The detailed formulas to express a geometric relationship between an image point  $(u, v)$  and physical position in space  $(x, y, z)$  can be found in Bechle et al. [12].

The procedure of interior calibration is used to obtain the parameters  $(u_c, v_c, \text{ and } d)$ , while the procedure of exterior calibration is used to yield the parameters  $(x_c, y_c, z_c, \alpha, \tau, \text{ and } \theta)$ . Based on the procedure of interior and exterior calibrations, the parameters of right and left cameras are obtained and shown in Table 1.

For robust/accurate image calibration, a large number (at least more than 15) of high-quality control points are essential [21]. In the present study, 44 control points were established and measured using a total station (see the image in Figure 3 for their distribution and physical coordinates). Also, a rotational calibration technique was developed for the in-situ conditions. The basic idea was to rotate both cameras between the observation region (river channel) and an area (stairs) where the control points existed. The rotational calibration can be conducted before/after image acquisition since our cameras always can be rotated back to the initial orientation  $\alpha$  [12]. The main reason that the control points are selected at stairs in the gauging station is to prevent the researchers who intrude the river channel region for measuring the physical coordinates using total station. The other one reason is that if the control points are set in the river channel region, they should be inundated when river banks are flooded during the typhoon events.

Note that 30 known control points were utilized to determine camera parameters and the rest 14 points were further used to examine the accuracy of image calibration. Based upon the resolved parameters from rotational calibration approach, the images of right and left cameras can adequately estimate the positions of those testing points with average errors of 0.82 and 2.2 cm, respectively.



**Figure 5.** The collinearity relationship between the object  $(x, y, z)$ , image point  $(u, v)$ , and camera  $(x_c, y_c, z_c)$ . Note that  $\Delta\alpha$  is the change of angle from the initial azimuth  $\alpha$  to the observation area.

**Table 1.** Parameters used for right and left cameras and results of parameter sensitivity analysis.

Parameter	Right camera	Left camera	Rate of change (RC) of surface velocity using LSPIV (%)	
			Parameter used at right and left cameras increases 10%	Parameter used at right and left cameras decreases 10%
$u_c$ (pixel)	800	800	-0.56	0.57
$v_c$ (pixel)	600	600	10.14	-7.67
$d$ (m)	0.0147	0.0168	-7.77	10.44
$\alpha$ (degree)	2.44	-1.98	0.00	0.00
$\tau$ (degree)	115.86	98.22	-49.67	294.75
$\theta$ (degree)	178.15	178.94	84.11	-110.67
$x_c$ (m)	0.230	-0.091	0.00	0.00
$y_c$ (m)	1.056	1.622	0.00	0.00
$z_c$ (m)	1.785	1.738	2.23	-2.23

(ii) Image mapping

A quasi three-dimensional approach was adopted to map each recorded image to the physical coordinate system. Two images (at least) were taken from different vantage points to triangulate three-dimensional coordinates of water surface [12,20,22]. Based on the camera orientation parameters, two conditions are insufficient to resolve the three unknowns  $(x, y, z)$  for image points  $(u, v)$  in either near- or far-field observation area. By the quasi three-dimensional approach in which automatic gauged water level  $z$  from the Yufeng gauging station was provided, therefore, corresponding location  $(x, y)$  of a featured object can be determined. The same approach of image mapping used by Bechle et al. [12] was employed in current study.

Note that the free surface of shallow-water flow [23] was assumed to be relatively flat and parallel to the XY plane. At current study site (i.e., Yufeng gauging station), measurement results from a level meter indicated that slope of water surface was generally smaller than 0.002 in the flow direction. Along the centerline of Yufeng Creek, changes in water levels were about only  $\pm 1.5$  cm from the mean. In this study, use of mean water level merely caused a tiny error of 0.01 cm in estimated horizontal position, supporting validity of flat water surface assumption.

### (iii) Velocity measurement

Surface velocity, including its magnitude and direction, is a valuable indicator to the discharge condition of a river. In this study, the LSPIV tracking motion of features on water surface over time was used to measure surface velocity. The procedure for LSPIV analysis is described as follows. First, images are selected from those created ortho-photos (in the previous image mapping step) using a constant time interval, e.g., a time step  $\Delta t = 0.05$  s in this study. Second, two successive images (A and B) are used to analyze the velocity field. Image A is divided into a uniform grid of subarrays, known as interrogation areas. The dimension of interrogation areas ( $100 \times 100$  pixels) can be decided according to the size of flow patterns [6]. A subarray is then moved and located in Image B with an image intensity correlation method to determine the displacement of interrogation areas. The most likely displacement of the visible features on water surface is determined as the maximum cross-correlation coefficient computed between two ensembles of gray-scale pixels in interrogation areas [19]. The overlap between interrogation areas in two successive images is 50% for typhoon event and 95% for normal flow condition. Such interrogation areas ( $100 \times 100$  pixels) were adequate to capture the motion of surface water.

Ten images (i.e. 0.5 s) were used to obtain the time averaged velocity field. The image resolution is not modified for LSPIV analysis. Because the time averaged velocity field was calculated automatically in the field, all images were recorded and saved in the computer. All velocity values across the image are simply averaged. The measured data before averaging is not smoothed and processed, therefore no range for measured velocity using LSPIV is defined.

In this study, the LSPIV method was further compared with other instruments for surface velocity measurements, including Stalker Pro II surface velocity radar (SVR) and SonTek acoustic Doppler current profile (ADCP). For measurement capability of SVR (and ADCP), flow velocities up to 18.0 m/s (and 20.0 m/s) can be well captured (with errors smaller than 1%). The location of ADCP and SVR data acquisition campaign along cross-section is shown in Figure 3a.

### (iv) Discharge calculation

The river discharge was calculated using a velocity-index approach which relates the surface velocity ( $V_{surf}$ ) obtained from LSPIV to a depth-averaged one ( $V_{depth-avg}$ ), i.e.,

$$V_{depth-avg} = \frac{Q_{measured}}{A} \quad (1)$$

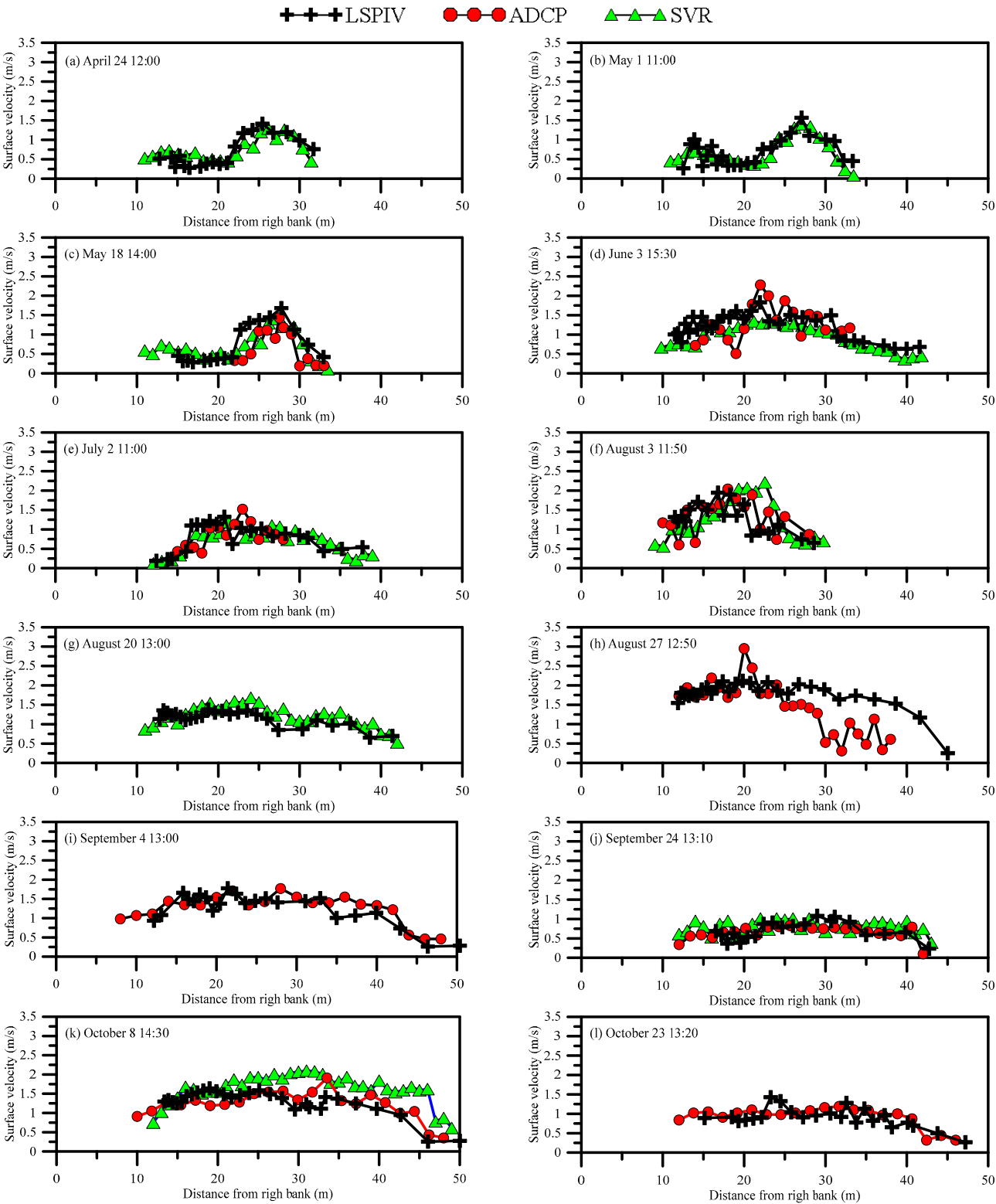
and

$$V_{depth-avg} = k \times V_{surf} \quad (2)$$

where  $Q_{measured}$  is the measured stream discharge obtained from ADCP;  $A$  is the cross-section area;  $k$  is the surface correction factor (i.e., velocity index). The depth-averaged velocity ( $V_{depth-avg}$ ) is obtained from ADCP measurement. In the laboratory, a surface correction factor based upon a parabolic vertical velocity profile was determined, i.e.,  $k = 0.95$  [24]. In the field, the correction factor  $k$  is site-specific and may show significant variability among various hydraulic/hydrologic conditions. Generally, correction factors present an increasing trend with water stages, e.g.,  $k = 0.85$  for the baseflow and  $k = 0.93$  at a high flow condition [6].

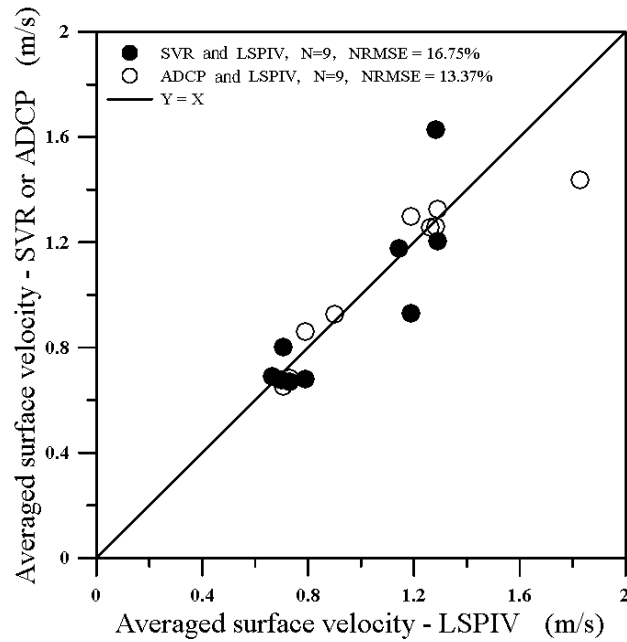
In this study, a velocity-index relationship for ADIS was established based on the depth-averaged velocity derived from the ADCP cross-sectional measurement results. Once the correction factor is obtained, river discharges can be readily estimated from the surface velocities using Eqs. (3) and (4). Note that velocity fields from concurrent surveys with a standard velocity-area method [25] were also employed to calculate total river discharge (sum of products of depth-averaged velocity and area in divided subsections) for further comparison (or validation).





**Figure 6.** Comparison of surface velocities measured by LSPIV, ADCP, and SVR on different dates under normal flow conditions: (a) April 24, (b) May 1, (c) May 18, (d) June 3, (e) July 2, (f) August 3, (g) August 20, (h) August 27, (i) September 4, (j) September 24, (k) October 8, and (l) October 23 in the year of 2015.





**Figure 7.** Scatter plot of averaged surface velocities for LSPIV against SVR (solid circles) and ADCP (open circles) measurements.

**Table 2.** Averaged surface velocities from ADIS, ADCP, and SVR measurements.

Measured date in year 2015	Averaged surface velocity (m/s)				
	LSPIV (A)	ADCP (B)	SVR (C)	$(D) =  (A) - (B) $	$(E) =  (A) - (C) $
April 24, 12:00	0.663	--	0.691	--	0.028
May 1, 11:00	0.698	--	0.675	--	0.023
May 18, 14:00	0.73	0.682	0.67	0.048	0.06
June 3, 15:30	1.189	1.3	0.929	0.111	0.26
July 2, 11:00	0.79	0.861	0.679	0.071	0.111
August 3, 11:50	1.291	1.325	1.206	0.034	0.085
August 20, 13:00	1.144	--	1.178	--	0.034
August 27, 12:50	1.827	1.437	--	0.39	--
September 4, 13:00	1.26	1.256	--	0.004	--
September 24, 13:10	0.706	0.654	0.8	0.052	0.094
October 8, 14:30	1.283	1.26	1.628	0.023	0.345
October 23, 13:20	0.9	0.928	--	0.028	--

**3. Results and Discussion**

Our ultimate goal is to promote this cost-effective ADIS for practical operations in the near future. In the present study, the aim is to examine the performance of ADIS. In this section, the measured surface velocities using different instruments (i.e., LSPIV, ADCP, and SVR) were compared during normal flow first, followed by extreme-flow applications during typhoons in 2015. The analyzing approach using ADIS for normal flow and for typhoon event is same. The normal flow condition is defined as no rainfall during the measured date (see Table 2). The ADIS measurement for typhoon events in 2015 include Typhoon Chan-Hom (July 10-12), Typhoon Soudelor (August 7-9), Typhoon Goni (August 23-25), and Typhoon Dujuan (September 28-30).

**3.1. Comparison of surface velocity from different instruments**

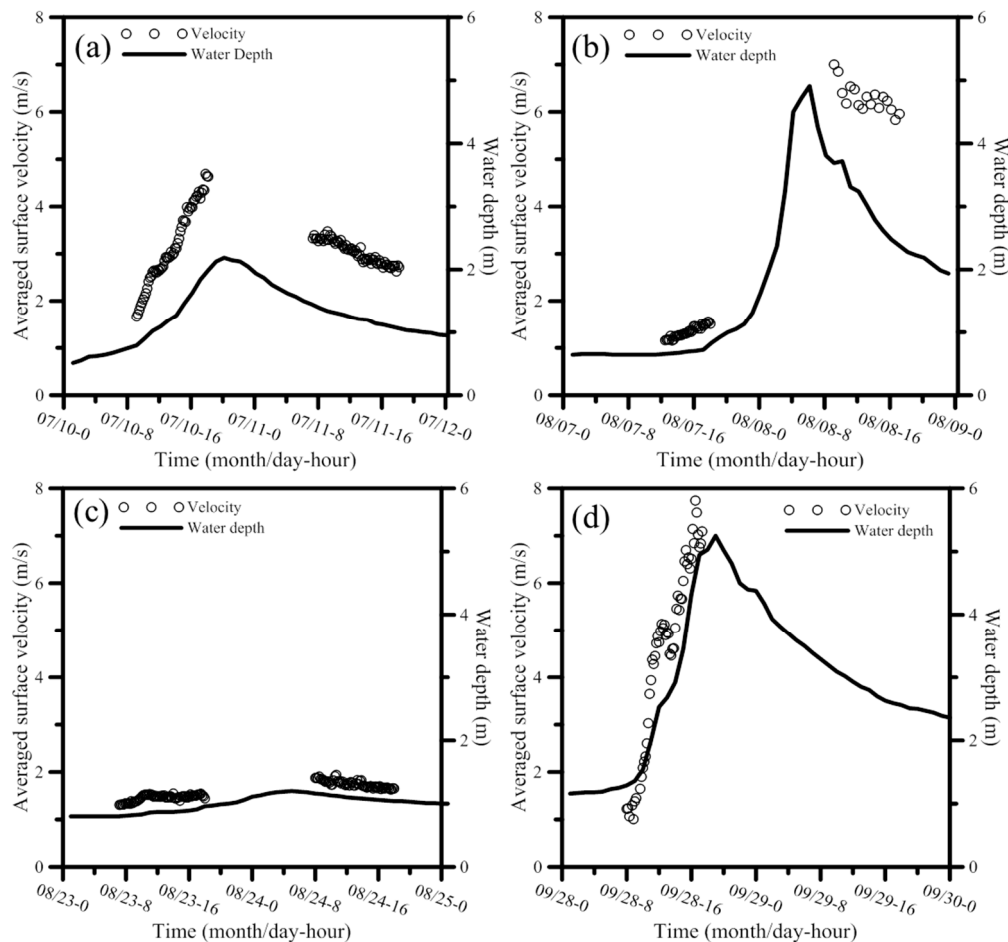
Figure 6 compares measured surface velocities along the channel cross-section in normal flow conditions. On each measured date, velocity patterns captured by LSPIV, ADCP, and SVR were quite

consistent. From the right to the left river banks (i.e., the X axis in the figure), surface velocities close to both banks were smaller and gradually increased toward the central stream. During these periods, the maximum of surface velocity could reach 3 m/s approximately (see Figure 6h). Similar characteristics were also found in [7,19,26,27,28]. In terms of averaged surface velocity (shown in Table 2), differences between the ADIS/LSPIV and ADCP (or SVR) measurements range from 0.004 m/s to 0.39 m/s (0.023 m/s to 0.345 m/s).

Figure 7 shows a scatter plot for the averaged surface velocities from LSPIV and ADCP (or SVR) measurements. Also, the normalized root mean square error *NRMSE*, a statistical indicator, was used to quantify the accuracy, i.e.,

$$NRMSE = RMSE / RMS = \sqrt{\frac{1}{N} \sum_{i=1}^N (O_i - P_i)^2} / \sqrt{\frac{1}{N} \sum_{i=1}^N O_i^2} \quad (3)$$

where *N* is the total number of data points; *O<sub>i</sub>* is the measured results from ADCP (or SVR); *P<sub>i</sub>* is the value measured with LSPIV. The *NRMSE* is 13.37% between LSPIV and ADCP measurements (or 6.75% when compared to SVR). Overall, comparisons indicated that ADIS/LSPIV is capable of measuring the surface velocities in river.



**Figure 8.** Time series data of self-recorded water levels and averaged surface velocities obtained by LSPIV at the Yufeng gauging station during (a) Typhoon Chan-Hom, (b) Typhoon Soudelor, (c) Typhoon Goni, and (d) Typhoon Dujan in the year of 2015.

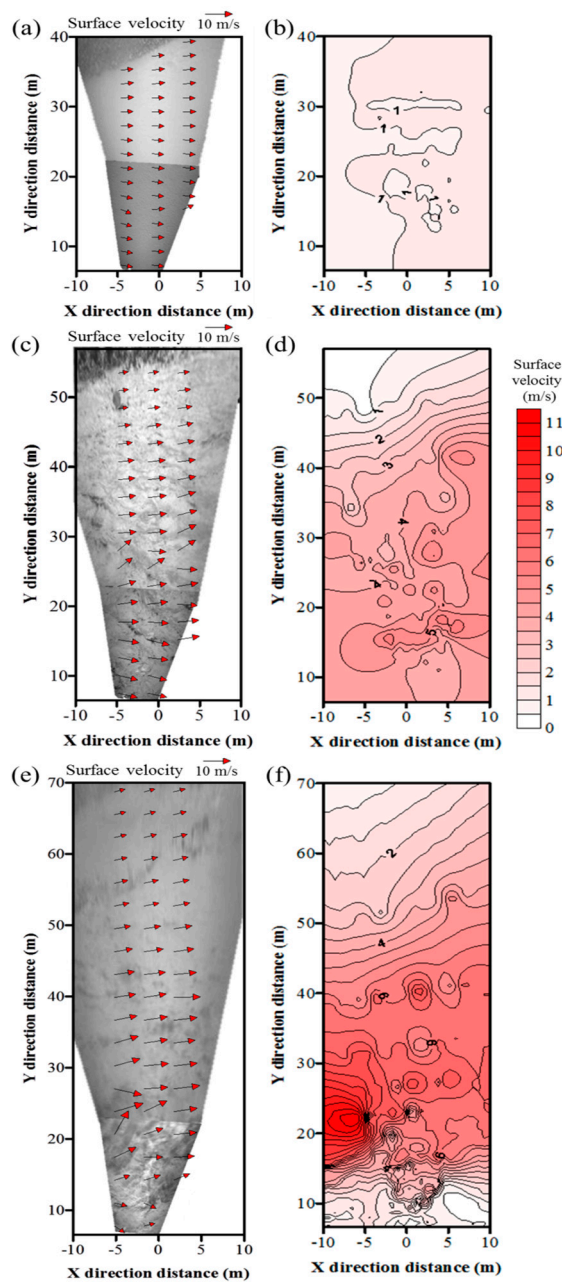
### 3.2. Comparison surface velocity and water level during typhoon event

ADIS based on LSPIV was further applied to measure surface velocities during typhoon events. Figure 8 presents the time series data for averaged surface velocities and self-recorded water depths at Yufeng gauging station during Typhoon Chan-Hom, Typhoon Soudelor, Typhoon Goni, and Typhoon Dujan. In response to flash floods, both water depths and averaged surface velocities

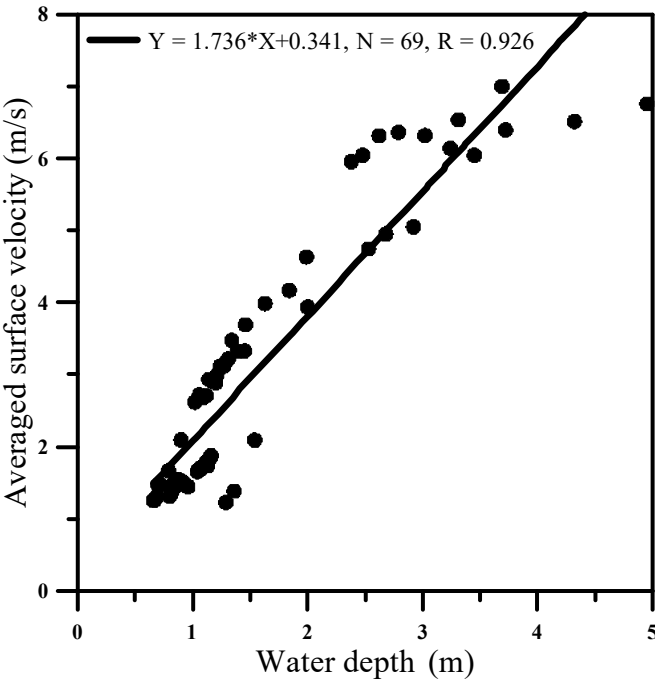
showed rapid increases. Especially in the event of Typhoon Dujuan, the peak of averaged surface velocities up to 8 m/s was captured. Noticed that no data were available in the nighttime currently since the ADIS would not work due to light limitation.

Figure 9 illustrates the velocity vectors and velocity contours using LSPIV on September 28, 2015 for Typhoon Dujuan. The value zero at X axis represents the camera location. Y axis is the lateral direction which represents across the river (see Figure 3). Because the river bank was flooded at 17:00, the river width at Y direction was extended comparing to river width at 08:00 and 12:00. It also can be seen that the surface velocity at 17:00 is larger than that at 08:00 and 12:00.

Figure 10 shows the scatter plot for water depths and averaged surface velocities in these four typhoon events, revealing a strong relationship with correlation coefficient  $R = 0.926$ . Similar results can be also found in the study of Creutin et al. [6] who conducted discharge measurement in the Iowa River using PIV techniques. Based on the high correlation, surface velocity of the flow was expressed as a function of water depth.



**Figure 9.** Measured surface velocity using LSPIV on September 28, 2015 (Typhoon Dujuan) (a) velocity vector at 08:00, (b) velocity contour at 08:00, (c) velocity vector at 12:00, (d) velocity contour at 12:00, (e) velocity vector at 17:00, and (f) velocity contour at 17:00.



**Figure 10.** Scatter plot between measured water depths and averaged surface velocities for all typhoon events in 2015.

3.3. Index velocity

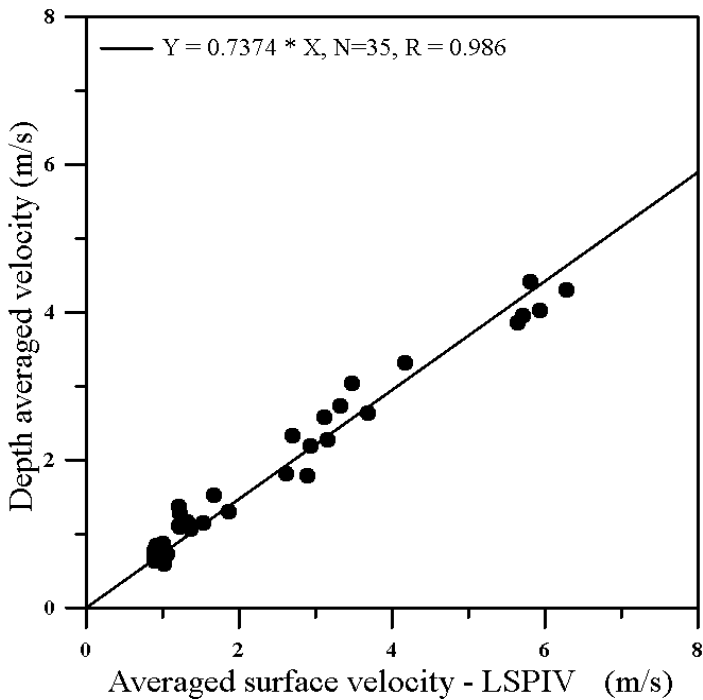
Figure 11 compares the measured surface and depth-averaged velocities obtained at different dates. The surface velocity is measured from ADIS/LSPIV, while the depth-averaged velocity is obtained from ADCP measurement. Note that the cross-section area is calculated based on the actual water depth cross the river section instead of assuming a constant averaged depth (see Figure 3d). Linear regression was used to yield the velocity-index relationship. The correction factor (velocity index) for Yu-Feng gauging station was  $k = 0.737$  with a correlation coefficient  $R = 0.986$ . In the literature, Polatel [29] found that velocity index ranges from 0.789 to 0.928 based on a series of laboratory experiments. Besides, Muste et al. [9] reported that velocity index is dependent on the vertical profile of velocity affected by flow aspect ratio, Froude and Reynolds number, micro and macro bed roughness, and relative submergence of large-scale roughness elements. Le Coz et al. [19] utilized LSPIV approach to measure flash floods, giving a velocity index of 0.90 based upon available gauging data. By contrast, Jodeau et al. [7] considered a correction factor of  $k = 0.79$  for a mountain stream under extreme flow conditions. In this study, the velocity index was 0.737, somewhat smaller but close to the value used in Jodeau et al. [7]. The reasonable explanation is that high bottom roughness (due to coverage of gravels and stones on the river bed) produces lower velocity index for the Yu-Feng Creek (at upper catchment of Shimen Reservoir).

3.4. Discharge estimation

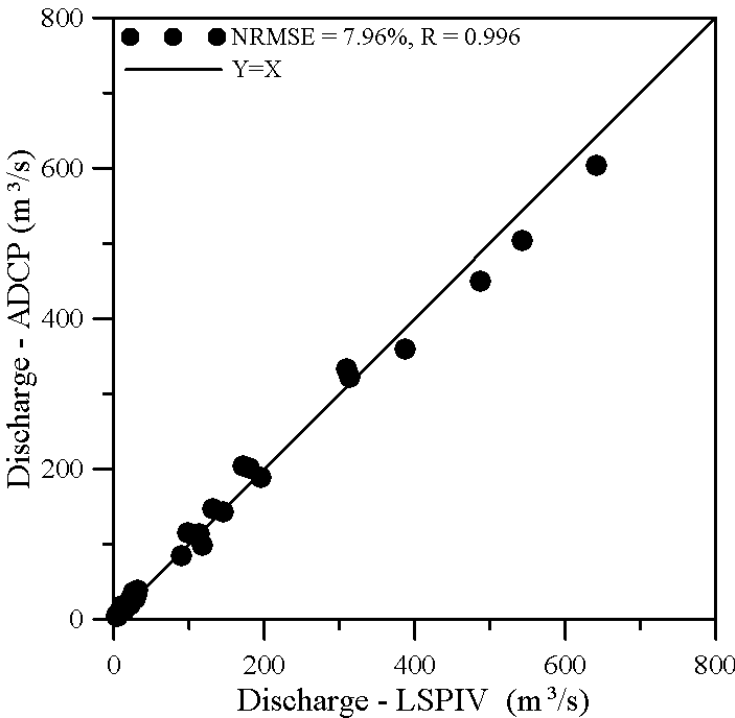
Discharge is calculated on the basis of the velocity-index and velocity-area methods. The measured surface velocity using LSPIV is transformed to depth averaged velocity according to the velocity-index (shown in Figure 11). The depth averaged velocity multiplies by cross section area to yield the discharge. Figure 12 compares the estimated river discharges from ADIS/LSPIV and ADCP measurements. Good agreement (with  $R = 0.996$  and  $\text{NRMSE} = 7.96\%$ ) indicates that the ADIS/LSPIV developed in this study provides a reliable tool for estimating flood discharge. Further, stream flows estimated using the conventional rating curve (from Water Resources Agency, Taiwan) are demonstrated in Figure 12. By comparison with the discharges from ADCP, flood estimations based on extrapolated part of the rating curve show significant credibility gaps [3] with a large statistical error of  $\text{NRMSE} = 42.2\%$ .



The ADIS indirect flow measurement can be utilized to improve stage-discharge relationships and reduce uncertainties associated with flood discharges [30]. Later on, combination with hydraulic analysis (from experimental investigation or numerical simulation) will be made to achieve further improvement for the LSPIV technique, e.g., capability in assessing various velocity structures over a range of flow conditions [19]. Overall, the results (see Figures 11 and 12) indicated that the developed ADIS/LSPIV provides a promising, accurate, and robust alternative (or even a replacement to the conventional rating curve method) for estimating stream discharge over wide ranging flow conditions (especially in the mountainous cases like Taiwan) [19,31].



**Figure 11.** The velocity-index relationship between LSPIV avergaed surface velocity and depth-averaged velocity.



**Figure 12.** Comparison of estimated river discharge from the ADIS/LSPIV and ADCP measurements.

### 3.5. Sensitivity analysis

The sensitivity analysis was conducted by varying the interior and exterior parameters at right and left cameras including  $u_c$ ,  $v_c$ ,  $d$ ,  $x_c$ ,  $y_c$ ,  $z_c$ ,  $\alpha$ ,  $\tau$ , and  $\theta$ . The effect of each parameter at right and left cameras on the surface velocity using LSPIV was evaluated using two alternative cases including 10% increase and 10% decrease in each parameter. The rate of change (RC) of surface velocity using LSPIV is given by following formula:

$$RC = \frac{V_{surf, sen} - V_{surf}}{V_{surf}} \times 100\% \quad (4)$$

where  $V_{surf}$  is the surface velocity obtained from LSPIV and  $V_{surf, sen}$  is the surface velocity obtained from LSPIV according to sensitivity analysis of interior and exterior parameters at right and left cameras.

Table 1 illustrates the results of sensitivity analysis. It shows that two parameters  $\tau$  and  $\theta$  used in LSPIV technique are sensitive to the estimation of surface velocity. The increases in parameters  $\tau$  and  $\theta$  lead to a 49.67% decrease and a 84.11% increase in surface velocity, respectively. On the other hand, the decreases in parameters  $\tau$  and  $\theta$  yield a 294.75% increase and a 110.67% decrease for surface velocity, respectively. Given 10% change of surface velocity, the acceptable tolerances for parameters  $\tau$  and  $\theta$  are 0.8 and 3.5 degrees, respectively. In other words, one who measures the surface velocity using LSPIV should control the tilt ( $\tau$ ) and roll ( $\theta$ ) angles of camera within 0.8- and 3.5-degree deviation, respectively, to ensure effective measurement (i.e., less than 10% of error). We further deduced the uncertainty of measurement by specifying the parameters ( $\tau = 0.8$  degree and  $\theta = 3.5$  degree) with an increment of  $\pm 0.1$  degree. In terms of  $\tau$  (or  $\theta$ ), the resulting tolerance errors for surface velocity measurement were  $10 \pm 0.99\%$  (or  $10 \pm 3.50\%$ ) under 95% confidence level. Note that the camera parameters should be carefully adjusted and calibrated to obtain accurate results. Once the positions of left/right cameras was shifted, the parameters must be re-calculated using the known control points to eliminate errors in surface velocity measurement.

## 4. Conclusions

In this study, development of an automated discharge imaging system ADIS with LSPIV method for measuring flood discharges during typhoon events was presented. ADIS consists of dual cameras to capture complete surface images in the near and far fields. Surface velocities were accurately measured by the Large Scale Particle Image Velocimetry (LSPIV) technique. The ADIS/LSPIV was deployed at Yu-Feng gauging station in Shimen Reservoir upper catchment, northern Taiwan. For comparison (and validation), surface velocity measurements using ADIS/LSPIV and other instruments were carried out. All measured results were in good agreement with small differences in the averaged surface velocities, i.e., 0.004 to 0.39 m/s and 0.023 to 0.345 m/s when compared the ADIS/LSPIV to acoustic Doppler current profiler (ADCP) and surface velocity radar (SVR), respectively. The ADIS/LSPIV was further utilized to measure surface velocity during typhoon events in 2015. The measured water level and surface velocity were highly correlated, showing rapid changes simultaneously in response to flash floods.

The river discharge was calculated using a velocity-index approach that transfers the surface velocity obtained from LSPIV to a depth-averaged one. Due to coverage of gravels and stones on the river bed, high bottom roughness led to a small correction factor  $k = 0.737$  in the Yu-Feng Creek. The stream discharge was also estimated from ADCP measurement results using the velocity-area method. Discharges obtained from ADIS/LSPIV and ADCP were quite comparable with  $R = 0.996$  and  $NRMSE = 7.96\%$ . Conventional flood estimation based upon extrapolated parts of a rating curve yielded significant credibility gaps with a large statistical error of  $NRMSE = 42.2\%$ . The sensitivity analysis of parameter shows that components tilt ( $\tau$ ) and roll ( $\theta$ ) of the camera are most sensitive parameter to affect the surface velocity using LSPIV technique. Overall, the promising ADIS/LSPIV provides non-intrusive, safe, and reliable measurement to obtain river discharges during flood events.

The limitation of ADIS/LSPIV indeed requires preliminary field-based intrusive experimental campaign for image calibration and the ADCP measurements for estimating the depth. The ADIS/LSPIV measurement in the nighttime would be supplied with extra light source which should be investigated and resolved.

In field experiments, despite of its great performance, ADIS/LSPIV technique for stream discharge measurement may still encounter uncertainties from various factors such as orthorectification induced by camera movement, image quality and sampling, surface velocity errors affected by wind and trace quality, and complex flow structures. In the future study, field measurements will be investigated to assess uncertainties in the ADIS/LSPIV.

**Acknowledgments:** This study was supported by the Northern Region Water Resources Office, Water Resources Agency, Taiwan, under grant no. 104C10. This financial support is greatly appreciated. The authors acknowledge two anonymous reviewers provided useful comments and suggestions for improving the manuscript.

**Author Contributions:** Wen-Cheng Liu supervised the progress of the Water Resources Agency's project, served as a general editor, and prepared a draft of the manuscript. Wei-Che Huang and Chih-Chieh Young performed the experiments, data collection and analysis, and model establishment and discussed the results with Wen-Cheng Liu. All authors read and approved the final manuscript.

**Conflicts of Interest:** The authors declare no conflict of interest.

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