

Article

Evaluation of the Cooling and Pavement Performance of Basic Oxygen Furnaces Slag Used in Asphalt Mixture

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Abstract: The basic oxygen furnace slag (BOF) was wide used in road construction, but there was a lack of characteristics in different asphalt mixtures. This study investigates the properties of hot-mixed asphalt (HMA) containing stone mastic asphalt (SMA), porous asphalt (PA) and dense-graded BOF as a partial substitution for natural aggregates. The purpose of this study is to evaluate various BOF slag contents in the asphalt mixtures would affect the cooling behavior after compaction. Asphalt mixture specimens contained 0%, 20%, 40% and 60% BOF slag, respectively, as coarse aggregate. Test results showed that BOF slag has a lipophilic property, so it can be adsorbed by asphalt cement, thereby reducing the cost of asphalt. The stability value of all asphalt mixtures increases with the proportion of BOF slag replacement. In addition, the voids in the mineral aggregate (VMA) value variable exhibited significant differences among asphalt mixtures, and could determine the deviation of the cooling trend of asphalt mixtures. Furthermore; it was found that the cooling procedure of the BOF slag used in dense-graded asphalt mixture takes about 100 min, and that the temperature tends to be moderate; however, it took about 120 min of cooling the SMA and PA mixture with BOF slag. In addition, the voids distribution of dense asphalt mixture was not uniform. It would result in various locations of thermal energy temperature on asphalt mixtures that were inconsistent.

Keywords: asphalt mixture; cooling; basic oxygen furnaces (BOF) slag

1. Introduction

Basic Oxygen Furnace Slag (BOF) is a steelmaking by-product. China's annual output of BOF slag reaches 2 million tons [1], and most BOF slag treatment methods involve conveyance to a disposal site for landfill which consumes space; as well, water osmosis from BOF slag causes environmental pollution. There have been several studies on the feasibility of employing other applications. Rockliff & Moffett (2002) pointed out that BOF slag has similar properties to natural aggregate, and has no any compromising effects on asphalt cement [2]. Xue et al. (2006) investigated the applications of BOF for coarse aggregate asphalt mixture, which showed better construction performance than natural aggregate. In addition, BOF slag can be applied to road construction. BOF slag has a coarser surface texture than that of the conventional aggregate; the adhesive force between BOF slag and asphalt binder improved the asphalt mixture [3]. Xie et al. (2012) used a freeze-thaw experiment to compare reused BOF asphalt mixture and found that it still had better resistance to water sensitivity compared to basalt aggregate [4].

In order to improve the performance of asphalt pavement, various laboratory tests have been used to evaluate the feasibility at different conditions. Stone Mastic Asphalt (SMA) has a high stone content, which forms a gap-graded skeleton-like stone structure. It provides deformation resistance and durability, making it suitable for heavily trafficked roads. The voids of the mixture's structure are filled with a high asphalt mastic and filler. The stone skeleton of SMA has a high angularity that improves interlocking packing and stone-on-stone contact among aggregate [5,6]. Porous Asphalt (PA) has crushed coarse aggregate without a significant proportion of fine aggregate, and it has

high drainage and noise-abrasion properties. The performance of PA mixtures regarding rutting resistance and moisture susceptibility is not as efficient as that of dense graded mixture [7,8]. In addition, reclaimed asphalt pavement (RAP) is a useful alternative to virgin materials because it reduces the need to use virgin aggregate [9]. However, Praticò (2004) showed that the hardness of the total asphalt cement in the mixture increases with the RAP content, while rejuvenating agents were added to the mixture in order to improve recovered binder performance [10].

Based on the excellent performance of SMA and PA mixtures, many studies used BOF slag as aggregate in SMA and PA mixtures. Shen et al. (2009) added BOF into a porous asphalt (PA) mixture, and found that BOF could improve the skid resistance, water sensitivity, rutting resistance and sound absorptivity [11]. When used for BOF slag stone mastic asphalt (SMA) mixtures, BOF slag presents high rutting resistance, high-temperature stability and resistance of moisture damage to improve the stability of high-temperature permanent deformation and low-temperature brittleness, effectively increasing the stability of mixtures [12,13]. Wu et al. (2007) indicated that BOF slag could be characterized as a porous material and could improve the absorption of the asphalt binder in asphalt mixture. The permanent deformation produced at elevated temperatures for asphalt pavement was reduced [14].

In addition, a newly rehabilitated asphalt pavement has been compacted, and a period of time is required for the mixture to gain strength. The pavement could open to traffic only after it has been cooled and achieved adequate strength [15,16]. Criteria for opening pavements to traffic have generally been based on time or strength. Huang & Lin (2011) investigated the thermal temperature effect of BOF slag substituted in reclaimed asphalt mixture, and found that BOF slag could decrease the cooling effect; which means that BOF slag mixture has a temperature preservation effect [17].

However, few studies have been conducted on the cooling effect of different aggregate gradation on asphalt mixtures, and substantiating the 'open to traffic' criterion. Thus, this study evaluated BOF slag applications to different aggregate gradations of asphalt mixtures, including the dense, SMA and PA graded. This study also analyzed the basic properties and cooling properties of BOF slag used in asphalt mixtures, and evaluated BOF slag applications regarding the effectiveness of stabilization and 'open to traffic' time of different types of aggregate gradation asphalt mixtures. The objectives of this research are listed as follows:

- Estimating the cooling effect of BOF slag substitution used in dense, SMA and PA mixtures.
- Comparing the cooling interval of different aggregate gradations on asphalt mixtures.
- Evaluating the reasonable 'open to traffic' temperature of BOF slag substitution used in different asphalt mixtures.
- Observing the voids distribution of BOF slag substitution used in different asphalt mixtures.

2. Materials and Method

2.1 Basic Properties of Asphalt and Aggregate

The BOF slag used in this study originated from the steelmaking furnace areas of the China Steel Corporation (Taiwan); the virgin aggregates (granite) were provided from a local asphalt plant. The BOF slag was used as the substitute of coarse aggregate. The physical properties of BOF slag and virgin aggregate are listed in Table 1. The specific gravity of natural aggregate and BOF slag is shown in 2.6 and 3.3, respectively. BOF slag is a by-product generated during the steelmaking process; therefore, it contains a higher amount of iron, resulting in higher specific gravity and improved load-bearing capacity. In addition, the test results indicated that BOF slag has higher water absorption, but lower L.A. abrasion and Flat and Elongated (F&E) ratio than natural aggregate.

The flatness ratios of BOF slag and natural aggregate are 0.91 and 6.88, respectively. The flatness ratio is defined as the ratio between the flat and elongated sides of coarse aggregate particles, and as the indicator of load-bearing capacity for asphalt mixture. The larger flatness ratio means that the aggregate may break and result in decreased loading resistance. These characteristics would provide more aggregate interlocking and increase the resistance to aggregate breakdown by

heavy vehicles. AC-20 grade cement is used for the asphalt binder. The basic properties of asphalt cement are shown in Table 2.

Table 1. The BOF slag and Natural aggregate properties.

	BOF slag	Natural coarse aggregate	Criteria	Specification
Specific gravity	3.17	2.60	–	ASTM C127-07 ^a [18]
Water absorption (%)	2.60	1.69	3 ^e	ASTM C127-07 ^a
Los Angeles abrasion test (%)	11.30	28.89	40 ^e	AASHTO T96 ^b [19]
Fracture($\leq 3\%$)	2.64	22.24	–	ASTM D5821-01 ^c [20]
Surface($\geq 3\%$)	96.30	78.80	–	ASTM D5821-01 ^c
Flat and elongated ratio (F&E) ($\geq 1:3$ %)	0.91	6.88	7 ^e	ASTM D4791-10 ^d [21]

Table 2. The asphalt binder properties.

Test	Results	Criteria	Specification
Specific gravity	1.037	–	AASHTO T228 ^a [22]
Softening point (°C)	57	50 ⁱ	AASHTO T36 ^b [23]
Ductility (25°C, 5 cm/min, cm)	100+	50 ⁱ	AASHTO T51 ^c [24]
Flash point (°C)	230+	232 ⁱ	AASHTO T48 ^d [25]
Viscosity (60°C, poise)	1665	2000 \pm 400	AASHTO M226 ^e [26]
Solubility (%)	99.5	99 ⁱ	AASHTO T44 ^f [27]
Mix temperature (170 \pm 20cSt, °C)	155	–	ASTM D2493 ^g [28]
Compaction temperature (280 \pm 30cSt, °C)	143	–	ASTM D2493 ^g
Penetration (25°C, 100 g, 5 s 0.1 mm)	68.7	60 ⁱ	AASHTO M20 ^h [29]

2.2 Asphalt Specimen Preparation

The asphalt mix design was based on the Marshall Mix design method to incorporate the use of natural and BOF slag aggregates. The optimum asphalt contents in various asphalt mixtures (dense, SMA, PA) were determined in accordance to the Asphalt Institute method, MS-2. Three samples were prepared for each of the mixtures, resulting in a total of 15 samples. In addition, the viscosity blending chart was utilized to determine the binding and compaction temperature of the asphalt mixtures.

The 15 samples were then heated to 120°C to simulate the asphalt mix temperature during asphalt paving. In this study, the viscosity blending chart was employed to determine the optimum blend and compaction temperature of the new asphalt binder. Asphalt samples were also prepared for Marshall Mix testing at blend temperature of 120°C and compaction temperature of 115°C.

The purpose of using the Marshall Mix design is to ensure that the proportion distribution of each volume of aggregate in the asphalt mixture can comply with the standards specifications of designed gradation. Marshall Mix Design mainly measures different property parameters of asphalt content, such as asphalt mixture stability value, flow value, unit weight, air void (VA), VFA (voids filled with asphalt) and VMA (voids in the mineral aggregate), to determine the optimum asphalt content for all mixtures.

The BOF slag was used to substitute portions of the coarse aggregate, while virgin fine aggregate was applied. The percentages of the BOF slag used were 0%, 20%, 40% and 60% by volume of coarse aggregate, which corresponded to 0%, 12%, 27% and 42% of total aggregate by volume, correspondingly.

This study adopted AC-20 grade asphalt cement as the binder and mixed BOF slag with dense SMA and PA-graded, respectively; as well, the additive volume of coarse aggregate was set in the following ratios: 20%, 40% and 60%, and the test results of sieve analysis are shown in Tables 3-5. The nominal maximum size for each aggregate is 19 mm. In order to effectively compare every property of the asphalt mixture, all aggregate gradation distributions are centralized within the range of one times the standard deviation for the intermediate interval.

Since BOF slag is a substitute for coarse aggregate, and with the increased proportion of BOF slag, the passing percentage of coarse aggregate for every sieve will be decreased for the dense-graded mixtures. SMA and PA gradation would be formed to the jumping gradation (gap gradient) by increasing the coarse aggregate and replacement; therefore, with the increased proportion of BOF slag, the passing percentage of coarse aggregate for every sieve and replacement (#200) will be reduced for both the SMA and PA mixtures.

Table 3. Sieve Distribution (dense-graded).

Sieve (mm)	BOF replacement (%)				criteria
	0	20	40	60	
25	100	100	100	100	100
19	98.1	98.2	99.0	99.0	80-100
12.5	79.4	82.8	94.2	93.8	-
9.5	73.9	74.1	77.9	75.7	60-80
4.75	58.1	63.4	60.9	48.4	48-65
2.36	41.0	50.0	49.7	35.5	35-50
1.18	31.6	39.3	39.4	27.8	-
0.6	23.7	29.3	29.5	21.2	19-30
0.3	16.4	20.0	20.2	14.9	13-23
0.15	9.8	11.5	11.6	9	7-15
0.075	6.0	6.6	6.6	5.3	0-8

Table 4. Sieve Distribution (SMA-graded).

Sieve(mm)	BOF replacement (%)				criteria
	0	20	40	60	
25	100	100	100	100	100
19	97.9	93.9	99.7	99.0	90-100
12.5	77.0	44.4	95.7	94.4	-
9.5	72.0	38.0	78.3	79.6	56-80
4.75	58.1	27.4	54.2	51.2	35-65
2.36	39.2	20.1	33.0	36.1	23-49
1.18	29.5	17.5	24.2	28.2	-
0.6	21.9	15.5	18.4	21.5	-
0.3	15.0	13.7	13.3	15.1	5-19
0.15	8.6	12.0	8.7	9.2	-
0.075	5.0	10.6	5.8	5.4	2-8

Table 5. Sieve Distribution (PA-graded).

Sieve(mm)	BOF replacement (%)				criteria
	0	20	40	60	
25	100	100	100	100	100
19	97.2	97.3	97.6	97.8	95-100
12.5	68.4	69.2	71.1	73.4	64-84
9.5	56.9	54.7	51.1	53.4	-
4.75	24.7	25.1	23.3	25.0	10-31
2.36	13.1	14.2	14.4	15.3	10-20
1.18	10.8	11.2	11.7	12.4	-
0.6	8.9	8.9	9.8	10.5	-
0.3	7.2	6.7	8.1	8.8	-
0.15	5.6	4.7	6.4	7.1	-
0.075	4.7	3.5	5.2	5.7	3-7

2.3 Laboratory test program

2.3.1 Cooling Measuring Test of Thermal Effect

The cooling test was performed to evaluate the variable temperatures on asphalt mixtures with different BOF slag contents and graded during the cooling process. Before measuring the cooling gradient of all specimens, it is necessary to prepare the simulated rutting specimen of asphalt mixtures, as shown in Fig. 1. The purpose of this process is to simulate the actual pavement that has been completed after the rolling operation, and to assess the cooling behavior. The wheel track test device was a British 77-B0350 type tester. The conducted mold size was 30 cm in length \times 30 cm in breadth \times 5 cm in height; it was prepared and compacted in a steel mold. The size of the rubber block is 30 \times 30 \times 3.5 cm. The stiffness value of the rubber block is close to subgrade, and could simulate the reaction of the bottom layer by the traffic loading [30].

The production process of the cooling test specimen involves putting the mixture into the mold, and rolling by wheel tracking device testing. This device is commonly used for investigating the permanent performance of asphalt mixtures under conditions that simulate the effect of traffic and pavement temperature. Testing of samples within the wheel tracking device testing generally consisting of loading 0.5MPa were designed. Test specimens are tracked back and forth under the applied stationary loading. Testing is typically accomplished for a total of 8,000 loading cycles (one cycle is defined as the backward and forward movement over samples by the wheel tracking device) [31]. All samples were then allowed to cool and the rate of cooling was measured and recorded.

2.3.2. Thermal Imaging Measuring

An Infrared Thermal Imaging Camera was simultaneously employed to measure the temperature pictures of the asphalt mixtures. When an asphalt specimen has completed the compaction procedure, the side direction of the specimen mold will be removed. An Infrared Thermal imaging camera and thermal Image Transmission Software were used to conduct the temperature monitoring and analyze the variation diagrams of temperature distribution. The measurement equipment is shown in Fig. 2. The temperature was recorded once every 5 min for a total of 180 min, along with recording the temperature variation at the center and surrounding points of the asphalt mixtures.

An Infrared Thermal Imaging Camera was employed to measure the temperature of the asphalt mixtures, together with thermal image analysis system application software. Based on a test scheme,

continuous measurements of the temperature distribution and cooling gradient for designated points of different specimens are done at regular time intervals. Continuously measurements are made and recorded until the cooling temperature reaches room temperature. In addition, the procedure is also combined with the void structure distribution assessment of different asphalt specimens to observe the void structure, aggregate property, and temperature effect. Moreover, measurement temperature data were used to understand the heat preservation mechanism of BOF slag mixtures.



Figure 1. Rutting Specimens for Asphalt Mixture.

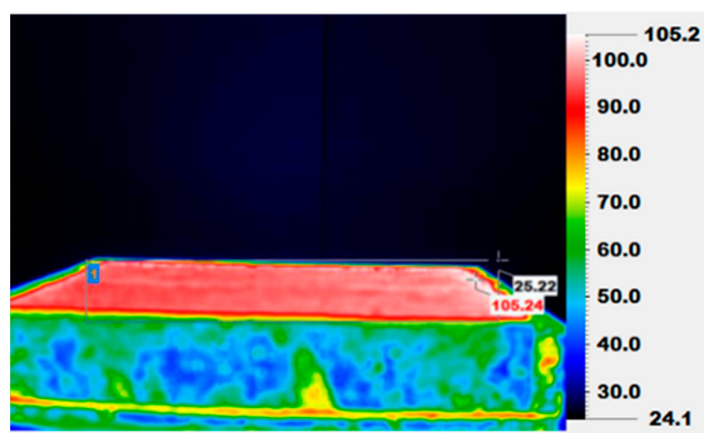


Figure 2. Thermal Image of Asphalt Mixture.

3. Test Results and Discussions

This study used the substitution ratio of BOF slag at 0%, 12%, 27% and 42% as coarse aggregate to substitute different aggregate gradation asphalt mixtures, with the time of dry mixing set at 15 s and 60 s for the wet mixing. Conducted with the Marshall Mix design, respectively, after calculating their optimum asphalt content, and creating the stability, VA, VFA and VMA, the test results are shown in Table 6.

3.1 Optimum Asphalt Content

The optimum asphalt content of asphalt mixture should provide a mix with sufficient asphalt film thickness to provide good durability and avoid excessive asphalt drainage. The optimum asphalt content is determined from the average value of the corresponding asphalt contents to the maximum stability, the maximum unit weight and 4% void rate. Higher asphalt content might lead to a poor bonding behavior for aggregate. However, lower asphalt content of mixtures will potentially be susceptible to rutting and cracking.

Optimum asphalt contents of dense, SMA and PA-graded will be decreased when the BOF slag substituted proportion is increased. In BOF slag, the proportion of SiO_2 is less than in natural gravel. However, CaO occupies a higher proportion [32,33]. Since CaO is lipophilic, and BOF slag structure has higher surface area and porosity, which facilitates absorption by the asphalt cement to allow aggregate to bond with the asphalt more tightly [13], and has proven BOF slag to be lipophilic, it can be easily absorbed and bond with asphalt cement. Chen (2014) et al. undertook similar research which used the boiling water test to demonstrate, after BOF slag contacted with asphalt cement, it will form hydration products and the bonding behavior of BOF slag and asphalt cement [34]; therefore, it can decrease the asphalt content demand on asphalt mixtures.

SMA and PA-graded were designed as the jumping gradation with less proportion of fine aggregate and need to fill a large number of asphalt cement in voids among aggregates; but with the substitution of BOF slag for natural aggregate, it could lower the quantity demanded for asphalt cement, as well as reduce the cost of asphalt binder.

3.2 Marshall Test Value

The Marshall Test variable values for the three asphalt mixtures gradients containing different BOF slag contents are shown in Table 6. In Table 6, the stability of dense, SMA and PA-graded asphalt mixtures will be increased with the increasing BOF slag substitution ratio. The specific gravity of BOF slag is greater than the traditional aggregate and has a better load-bearing capacity; the asphalt film attached to the aggregate surface is also thicker.

The stability of SMA and PA-graded asphalt mixtures is greater than that of the dense-graded ones. The stability of dense, SMA and PA-graded mixture were between 16.4 to 20 kN; 16.5 to 20.2 kN and 16.4 to 20.4 kN, respectively. The stability of all the asphalt mixtures complies with the minimum requirement value of 80 kN.

The flow value of asphalt mixture decreases with the increase in the BOF slag substitution ratio; furthermore, if the flow value is too high, it means that permanent deformation may easily result for the pavement, possibly causing the phenomenon of plastic flow. However, the flow value of SMA and PA mixtures is much higher than that of the dense-graded mixture, which might indicate that the porosity of SMA and PA gradation lack the fine aggregate filled void among the aggregate. As well, the optimum amount of asphalt cement is high; thus, relative skidding may easily occur among aggregates, increasing the flow value.

The unit weight of the asphalt mixture will increase with the increased BOF slag substitution ratio. BOF aggregate contains CaO , and its specific gravity is greater than that of natural aggregate. Design coarse aggregate ratios of SMA and PA mixtures are more than that of the dense-graded mixture, and BOF slag was used to substitute for coarse aggregate; therefore, the unit weight of the asphalt mixture is obviously higher than that of the dense-graded one.

The void ratio is the ratio occupied by the void after mixing aggregate with an asphalt binder, and the specification is set at 3%~5%. The results showed that the void ratio of asphalt mixture decreased with the increased BOF slag substitution ratio. Because SMA and PA mixtures lack fine aggregate to fill the void among aggregate particles, mixtures need more asphalt binders to achieve void replacement, so as to comply with the specification set at 3%-5% for the asphalt mixture void ratio. Nevertheless, since the specific gravity between BOF slag and natural aggregate differs from each other, the compaction energy would cause aggregate fracture in the rolling process, making the gap-aggregate for gradation. As a result, the aggregate particles of dense-graded asphalt mixture might be rearranged, leading to the plastic flow or causing an increased void ratio for the asphalt mixture.

VMA is the occupation percentage of void in compacted aggregates without adding asphalt, and the standard specification is set at 13-14% as the minimum value. Rutting permanent may result if the value is less than the minimum value. The results showed that the asphalt mixture VMA value decreased with the increased BOF slag substitution ratio. The VMA value of the SMA-graded mixture is greater than that of the dense and PA-graded ones. BOF slag has porous properties, and asphalt cement can easily be permeated into the aggregate. Additionally, the traditional Marshall

Mix Design method for the jumping gradation can easily increase the asphalt content; thus, VMA values can be used to investigate whether or not the optimum amount of asphalt deviates from those of the others.

The VMA value of PA asphalt mixture without adding BOF slag is 17.8%, while the VMA values for BOF substitution ratios 40% and 60% are 16.8% and 15.8%, respectively. For the high-ratio mixture of BOF and aggregate, the VMA value decreased with the increased asphalt content. There might be relative skidding in SMA and PA-graded due to more asphalt binders.

Table 6. Marshall testing results of BOF mixtures.

Gradient	BOF (%)	Optimum asphalt content (%)	Unit weight	Air voids (%)	VMA	VFA	Flow (0.25mm)	Stability (kN)
Dense	0	5.25	2.362	3.6	14.6	72.2	8.75	16.43
	20	5.20	2.420	3.5	17.7	72.9	8.25	17.92
	40	5.10	2.515	3.9	17.2	67.2	7.75	19.59
	60	4.90	2.621	4.2	16.7	65.1	7.00	19.99
SMA	0	5.65	2.365	4.3	15.8	72.2	11.50	16.56
	20	5.51	2.421	4.1	15.5	75.2	12.00	16.86
	40	5.35	2.538	4.0	14.9	76.0	12.25	19.40
	60	5.30	2.657	3.8	14.7	77.9	12.75	20.24
PA	0	5.95	2.382	4.6	17.8	72.5	12.00	16.46
	20	5.82	2.430	4.3	17.5	74.1	12.50	16.95
	40	5.53	2.556	4.1	16.8	76.0	13.25	20.24
	60	5.41	2.727	3.7	15.8	81.1	13.50	20.43
Standard	-	-	-	3-5	>13	-	8-16	>80

Standard cited: ASTM D6927-15 [35]

3.3 BOF Substitute Proportional Influence on Specimen Cooling

Fig. 3 shows the temperature variations at different BOF slag substitution ratios in the specimens. Because the specimen surfaces were in contact of the air, the temperature dropped quickly. All the specimens have a maximum cooling gradient in the initial high-temperature status. As seen in Fig. 3(a), different amounts of BOF slag as dense-graded have a starting temperature around 105–112°C. The temperatures at the surface decreased quickly but stabilized after 80–90 min. At the end of the measurements at 180 min, the mixtures contained 20% of BOF slag had reached a stable condition with temperatures was 67°C. However, for BOF slag at 40% and 60%, the corresponding temperatures measured at 180 min were 69°C and 74°C, respectively.

As seen in of Fig. 3(b) and (c), after SMA and PA-graded cooled for 120 min, the temperature tended to moderate, indicating that the heat releasing effect of SMA and PA-graded tended to moderate with heat preservation effectiveness. Also, the cooling rate of SMA and PA-graded decreased with the increased substitute proportion of BOF slag, which is similar to the cooling effect of adding BOF slag in dense-graded, and the cooling rate would be decreased significantly. Similar replacement proportions of 0%, 20%, 40% and 60% had the temperature compared at 180 min; where the temperatures of SMA gradation were 66°C, 69°C, 70°C and 73°C, respectively; the temperatures of PA gradation were 68°C, 69°C, 70°C and 74°C, correspondingly.

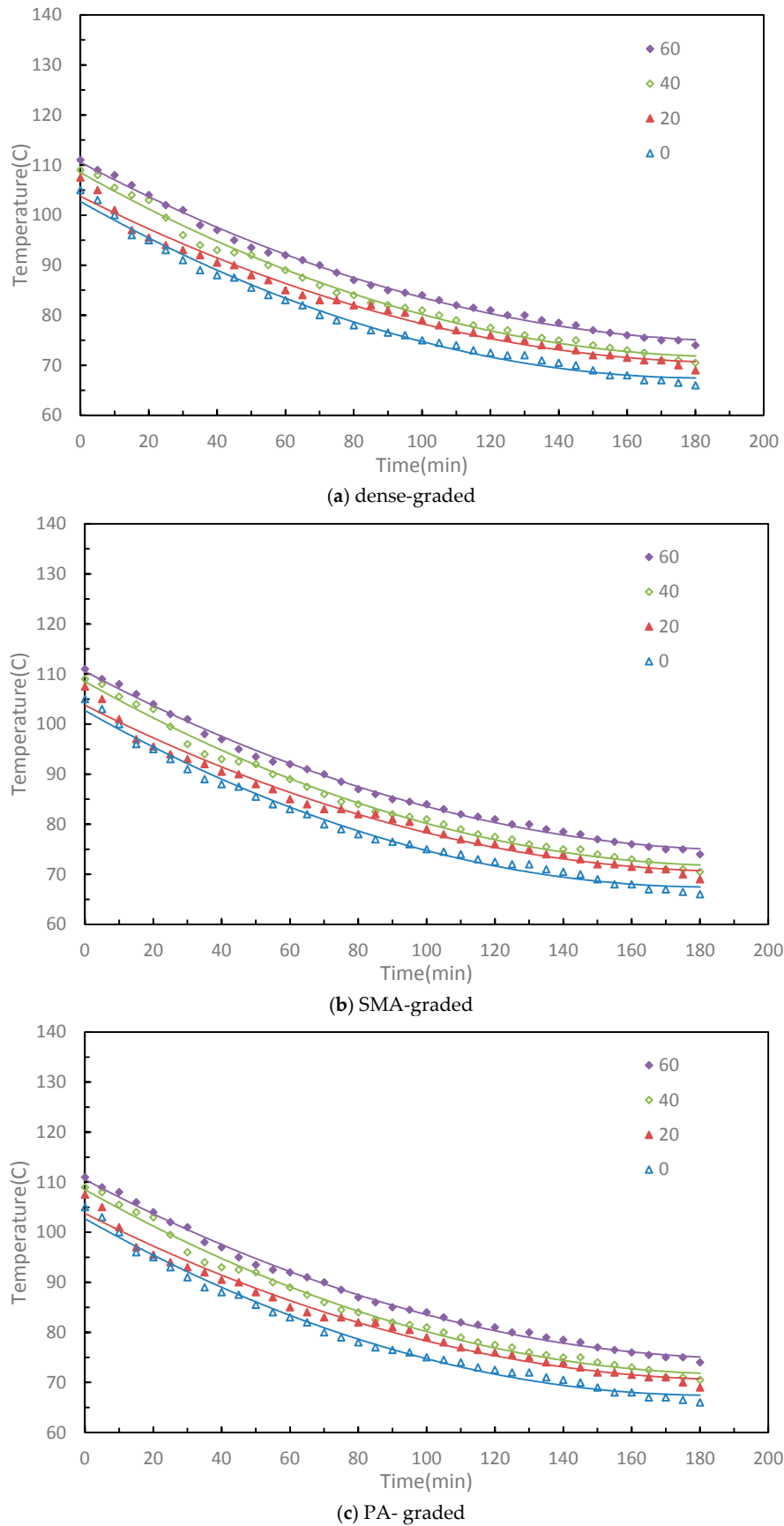


Figure 3. Temperature Gradient Variation Diagram.

After the compaction of asphalt specimens, the cooling temperature increased with the increased proportion of BOF slag, which means that besides having lipophilic qualities, BOF slag also has a heat preservation effect to maintain the temperature between aggregates, and facilitates the stable binding of asphalt with aggregate under sufficient temperature. The asphalt mixture used high-proportion BOF aggregate; it needs to extend the heat to be released completely. The asphalt mixture can have highly stable properties. Furthermore, SMA and PA-graded are designed with high-proportion coarse aggregate, and the aggregate skeleton is in direct contact between coarse aggregates to form the interlocking mechanism, as well as adding the high-proportion BOF as the coarse aggregate. Thus, the temperature can be preserved at the inter-contact status, and the cooling rate would be slow as well.

3.4 The Effects of BOF Slag Contents on Specimen Cooling Interval

Huang and Lin (2011) established the three zones by the temperature-time curves: Initial Temperature, Cessation Temperature and Equilibrium Temperature, respectively [17], as seen in Fig. 4. The Initial Temperature Zone represents the temperature changes from the time the asphalt mixtures leave the plant to the end of the compaction. The mixtures reach a more stable state at the Cessation Temperature. In addition, the Equilibrium Temperature indicated that the asphalt temperature basically could be the proper time for 'open to traffic'.

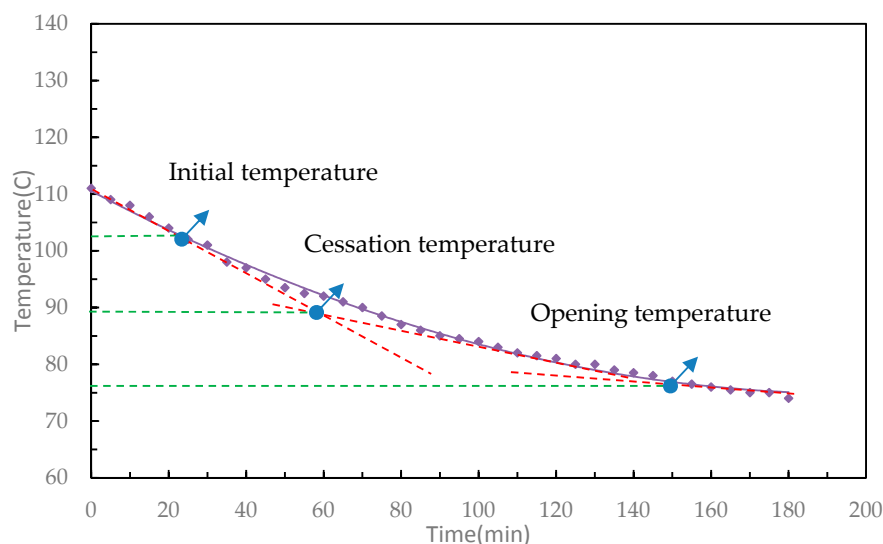


Figure 4. Cooling Interval Diagram of Asphalt Mixture.

In this study, the temperature-time curves for mixtures containing various BOF slag content are shown in Figures 5–7. Temperatures corresponding to the three inflection points of the curve were defined as the points. From Fig. 5, it can be seen that the Initial Temperature on dense mixtures decreases as the BOF slag content increases. For a dense specimen without BOF slag, the Initial Temperature was 95°C; for a specimen with 60% BOF slag, the Initial Temperatures drops to 90°C. This means that the BOF slag in the dense mixtures can store the thermal energy longer.

The Cessation and Equilibrium Temperature increases as the BOF slag content increases. Willoughby et al. (2000) defined the Cessation Temperature as the asphalt mixture temperature at which the density of asphalt pavement cannot be improved with additional compaction effort [36]. Based on this concept, for mixtures without and with 100% BOF slag, the Cessation Temperatures drop to 76°C and 86°C, respectively. The BOF slag in the dense mixtures could reach stability earlier at higher temperature than those without BOF slag specimen. For mixtures without and with 60% BOF slag, the Equilibrium Temperature drops to 70°C and 79°C, respectively. This phenomenon indicates that the use of BOF slag has a stable nature, even if the Equilibrium temperature is higher than pure dense specimen, about 9°C, and it can be 'open to traffic' on asphalt pavement.

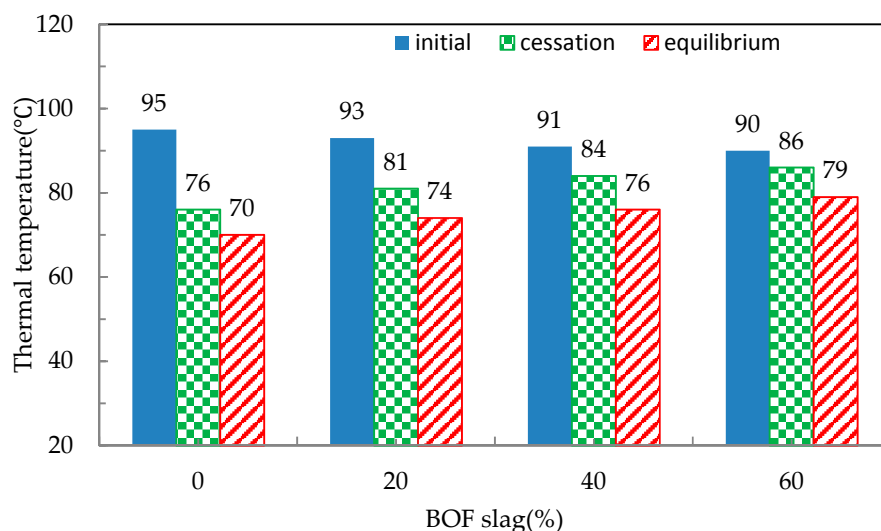


Figure 5. Relationship between the Temperature Intervals and the BOF Slag Contents (dense).

From Fig. 6, the Initial, Cessation and Equilibrium Temperature on SMA mixtures increase as the BOF slag content increases. The mixture, which is made with a high amount of the coarse particle size category, forms a coarse aggregate skeleton. This increases the interlocking of the aggregates and provides better stone to stone contact, which serves as a load carrying mechanism and hence provides better loading resistance. SMA specimen coarse aggregate particles form a stone to stone contact skeleton that can transfer thermal energy and increase all temperature zones. Moreover, Cessation and Equilibrium Temperature on SMA mixtures were not significantly different. This indicates that asphalt mixtures with BOF slag can be compacted and opening at higher temperatures as compared with those without BOF slag.

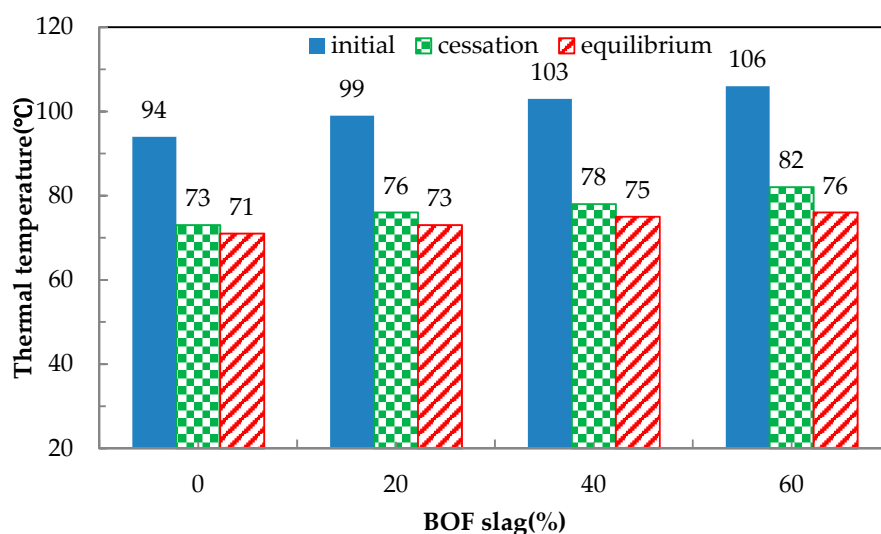


Figure 6. Relationship between the Temperature Intervals and the BOF Slag Contents (SMA).

From Fig. 7, the Initial, Cessation and Equilibrium Temperature on PA mixtures increase as the BOF slag content increases. A similar trend is also observed for the cooling interval within the SMA mixture. However, the PA mixture had lower Initial temperature interval than the SMA mixture did. As mentioned earlier, for the mixtures with 40% and 60% of BOF slag, the VMA values of all PA mixtures were higher than those of the SMA mixture, and therefore contain higher amounts of

coarse aggregate ((19, 12.5 and 4.75 mm aggregate size). Asphalt mixtures increase the porosity and contain a higher amount of coarse aggregate, which decreases the thermal conductivity, resulting in higher temperature. A similar trend is also observed in the study results of Mohajerani et al. (2017) [37].

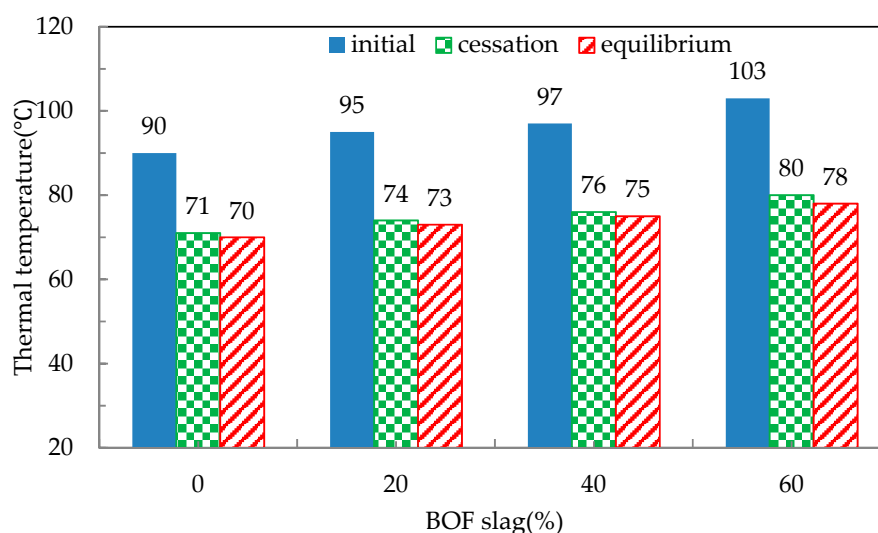


Figure 7. Relationship between the Temperature Intervals and the BOF Slag Contents (PA).

3.4 Voids Distribution of Asphalt Mixture

This study used Infrared Thermal Imaging Camera photos to prove the voids distribution of specimen by adding 40% BOF aggregate into three asphalt mixture gradations, for example, comparing the cooling time at the 60th minute. As shown in Fig. 8(a), the dense mixture surface temperature image showed the center temperature of the specimen to be 91.7°C; the temperature at the right side of the specimen is 94.8°C. Since the heat exchange process would be transferred to the area with more voids, it is determined that the right side of the specimen may have more voids.

Based on measuring the surface and internal temperatures from the sides of the dense mixture, as shown in Fig.8(b), it is found that the surface temperature of the specimen is about 92.4°C, and its internal temperature cooling rate is fast. The yellow area is about 60-70°C, which shows that after compacting the pavement in the cooling phase, the internal heat energy of asphalt mixture continuously transferred upward; therefore, the surface temperature of pavement was higher than the internal one. In measuring the test results, as expected, for dense mixture, there was a significant difference between the internal and surface temperatures. Furthermore, from the thermal image it can be determined that the distribution of voids of dense mixture is not uniform and the thermal energy temperature is inconsistent. This indicates that the non-uniform distribution of the voids on dense mixture caused the cooling process to decrease the loading resistance.

From Fig. 9(a), it can be found that the center temperature of the SMA specimen was 85.78°C; those temperatures around the sides without any significant difference were about 85.16-85.98°C, meaning that the internal void of SMA mixture was more uniform than that of the dense mixture. The characteristic of the asphalt binder exhibits viscous-plastic behavior at higher temperature. Thus, the film of the asphalt binder can be filled up with voids during the high-temperature mixing process, and it would have smaller voids and heat energy that might not be easily dissipated. Observed from the side thermal image, as shown in Fig 9(b), the SMA mixture heat can be transferred from the internal space up to the surface of the specimen. Since its internal voids are more uniform, its internal heat energy (yellow area) area occupied larger range (60°C-80°C); it showed that in the cooling phase, the uniform voids distribution can maintain the temperature with a slower heat dissipation rate than that of the dense mixture.

As shown in Fig. 10(a), when the center temperature of the PA mixture is 88.59°C, temperatures at the sides without any significant difference are about 89.12-87.83°C. A similar trend is also observed for the SMA mixture. In Fig.10(b), the internal heat energy (yellow area) area on the PA specimen is more significant (60°C-80°C), which means that after adding BOF slag into PA mixture, it increased the heat preservation effect of BOF slag with a slower cooling rate, promoting the bonding capacity for the PA mixture.

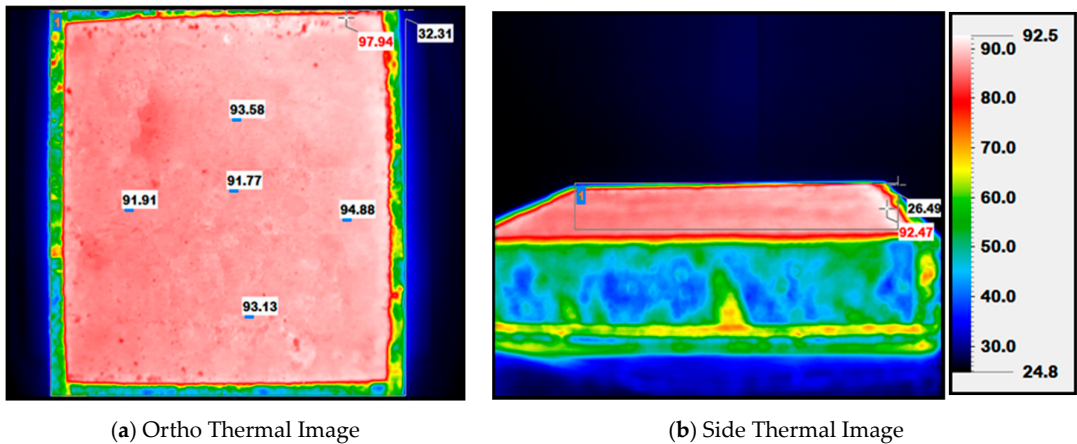


Figure 8. Dense-graded Thermal Image (°C).

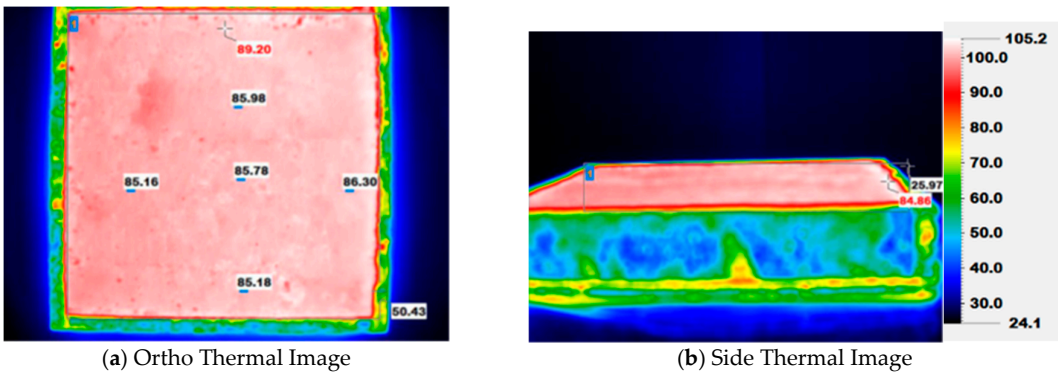


Figure 9. SMA-graded Thermal Image (°C).

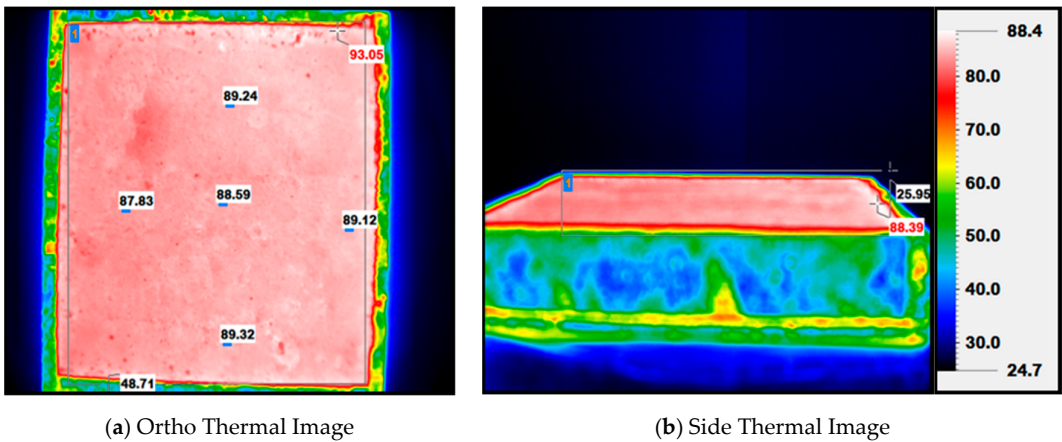


Figure 10. PA-graded Thermal Image (°C).

4. Conclusions

This study evaluated the effects of the use of BOF slag as coarse aggregate replacement on the cooling behavior of hot mix asphalt mixtures. From the study test results, the following conclusions can be drawn:

1. The optimum asphalt content of all asphalt mixtures (dense, SMA and PA) would be decreased in accordance with the increased proportion of BOF slag. BOF slag has a lipophilic property which can absorb asphalt cement, thus reducing the demanded quantity of asphalt.
2. The stability value of asphalt mixture would be increased with the increased BOF slag substitution ratio. Since the coarse aggregate ratio is high in SMA and PA-graded, and BOF slag was substituted for the coarse aggregate, the stability value of asphalt mixture also increased.
3. The VMA value of asphalt mixture would be decreased with the increased BOF slag substitution ratio; the VMA value of PA-graded mixture is lower than those of the dense and SMA-graded ones. BOF slag has porous properties, and asphalt cement can easily permeate into the aggregate and result in the decreased VMA value. Thus, the VMA value can be used to examine whether or not the optimum amount of asphalt has deviated from the others.
4. After cooling the dense mixture for about 100 min, the temperature tended to be moderated, which showed that the cooling and heat dissipation was gradually completed. However, after cooling, the SMA and PA-graded for 120 min, the temperature tended to be moderated, which showed that SMA and PA-graded have better temperature preservation effectiveness.
5. The voids distribution of the dense mixture was not uniform, and the thermal energy temperature was inconsistent as well. Voids distribution of SMA and PA-graded was more uniform than that of the dense mixture to maintain heat energy with a small heat dissipation rate in the cooling phase.
6. The temperatures increased as the BOF slag content increased at the same cooling time. The SMA and PA mixtures with higher BOF slag hold heat better than those containing less BOF slag. It would require longer time for mixtures containing BOF slag to lose heat, and thus require more time for the pavement to open to traffic.
7. The Initial Temperature of the SMA and PA mixtures with BOF slag was higher than that of the dense mixture. The SMA specimen coarse aggregate particles form a stone to stone contact skeleton that transfers thermal energy and increases all the temperature zones. Asphalt mixtures have higher porosity and contain higher amounts of coarse aggregate, decreasing the thermal conductivity and resulting in higher temperature.

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