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Traffic Calming Measures and Their Slowing Effect on the Pedestrian Refuge Approach Sections (Case Study—on Urban Streets in Poland)

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Abstract: The ever-increasing use of motor vehicles causes a number of traffic safety and community issues, which are particularly severe in cities, accompanied with scarcity of parking spaces and challenges encountered in the road layout alteration projects. The commonly applied solutions include designation of through streets and implementation of on-street parking on residential streets and retrofitted traffic calming measures (TCMs). This article presents the results of the study conducted on a two-way street where Metered Parking System MPS was implemented together with diagonal and parallel parking spaces, refuge islands, horizontal deflection and lane narrowing by a single-sided chicane. The aim of this study was to identify those TCMs that effectively helped to reduce the island approach speed. Heuristic method was applied to assess the effect of the respective TCMs on reducing the island approach speed and the key speed reduction determinants were defined using cause and effect diagram and Pareto chart. Comparative analyses were carried out to rate the respective TCMs as effective, moderately effective or ineffective. Section 1 of this article presents the output of the literature review on urban parking analyses and TCM efficacy. Section 2 presents the study site and the applied heuristic method. The study results are presented in Section 3. Section 3 defines the determinants on the cause and effect diagram and analyses the determinants using the Pareto chart. The final conclusions and comments are given in Section 5. Although the study was limited to a single street in Poland, the findings may hold true in other countries where similar TCMs are used.

Keywords: pedestrian refuges; refuge islands; speed variation; reduce speed; horizontal deflection; free view; Pareto chart; cause and effect diagram

1. Introduction

The ever increasing use of motor vehicles causes more and more severe traffic issues, in urban areas in particular. Various traffic management measures are applied to address these issues, including designation of urban transit routes, implementation of traffic calming schemes, parking planning, etc. A well-planned metered parking system requires smooth coincidence of traffic calming plans with the planned parking spaces and carefully planned pedestrian mobility improvements. The design aspects of different traffic calming measures (TCMs) are laid out in the basic design guidelines [1–5]. TCMs include raised intersections, speed tables, narrowing the carriageway by chokers or pinchpoints, various speed humps and speed bumps. Horizontal deflections are also applied in planning of parking spaces depending on the parking configuration.

Elvik [6] suggests to use meta-analysis approach in designating urban transit routes or traffic calming zones to address the relevant traffic safety issues. These should lead to defining a hierarchical road system and moving through traffic out of the residential streets, thus improving traffic safety in these residential areas. Different approaches to urban traffic safety and traffic and parking resources management scenarios also in metered parking settings are presented, for example, in [7–10]. It should be noted though, that the issues tackled in these articles concern mainly parking in urban

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areas. A different TCMs study approach taking into account their effect on the traffic performance, traffic safety, natural environment, public health and the economy was presented in articles [11–16] showing that traffic calming has some undesirable effects as well. The group of TCMs that were found to have undesirable environmental effects included speed cushions, speed bumps, speed humps and stop signs.

Review of studies on the speed-reducing effect of horizontal deflections located on the refuge island approach sections

The efficacy of various TCMs used on city streets, i.e., their slowing effect has been studied by many researchers. In most cases, these studies analyse TCMs in relation to traffic safety improvement [17–20]. Article [12] is different in this respect, in that it considers also the environmental and public health impacts of the analysed TCMs. The study involved in situ tests conducted using a special test vehicle. Le et al. [12] used a comparative analysis technique to demonstrate the superiority of chicanes among the analysed TCMs, except in terms of vehicle emissions. That said, most studies are limited to analysing the efficacy of speed humps, speed tables and chicanes in terms of speed reduction on the approach to pedestrian crossings. Some authors took into account landscape features and visibility of the pedestrian crossing and the road ahead, relating the obtained speed reduction not only to the TCMs but also to various factors townscape surrounding refuge island [21,22]. Improvements to the community life owing the implemented traffic calming scheme were analysed, for example, in [22]. Other researchers noted the slowing effect of repeating the speed humps or speed tables and the length of slowed driving [23–27].

Other researchers analysed the efficacy of various TCMs. For example, Gonzalo-Orden et al. described in [28] comparative analyses of speed reductions obtained with the applied raised crossings, lane narrowing or chokers speed cameras and speed camera signs. These analyses led to the conclusion that the obtained speed reductions depended on the TCM type, its geometric features and emplacement in the street. Distefano & Leonardii [29,30] arrived at similar conclusions on the efficacy of chicanes and horizontal deflections in city streets. They compared speed profiles (85th percentile and average values) on local streets before and after installation of speed tables and up to 1 m wide chicanes on a one-way street and road narrowing treatment accompanied by a horizontal deflection on a two-way street. The before-and-after study results presented in the article [29] show the highest percentage reduction of operating speed for a single lane chicane installed on a narrow one-way street with on-street parallel parking configuration. The lowest percentage reduction was, in turn, noted on a two-way street with a carriageway narrowing treatment on one side, accompanied by a horizontal deflection (with parallel on-street and pavement parking). In this case, very good visibility of the road past the narrowing treatment was ensured. Kruszyna & Matczuk-Pisarek [31] arrived at different conclusions in their study on speed reduction obtained with a refuge island, speed table on the approach section or a raised pedestrian crossing. The comparative analyses showed that raised pedestrian crossings offered the highest speed reductions. Sołowczuk [32] studied speed reductions obtained with raised pedestrian crossings in a downtown located Tempo 30 zone, relating the obtained values not only to the TCM geometry and the townscape surrounding street, but also to the specific traffic volume in a given street.

Akgol et al. [33] and Aydin et al. [34] conducted a driving simulator study to investigate the effect of chicanes installed near pedestrian crossings. The factors they considered in their study included the effective lane width, the shapes of islands and vehicle trajectories. In conclusions of [33] it is stated that effective speed reduction may be obtained with a set of three chicanes located at the refuge island on streets with 3 m effective lane width or with a more economical option of two chicanes on streets with 2.7 m effective lane width. Hussain et al. also used a driving simulator, yet with a different approach, as presented in their article [35] investigating the effect of roadway narrowing, horizontal deflection and various road markings and upright signs. These studies confirmed the highest efficacy of road narrowing used in combination with horizontal deflection and carriageway narrowing obtained by zigzag markings or variable message signs.

The first study that related the speed reduction to the travel path deflection by a median island or chicanes was conducted in the UK by Sayer and Parry [36,37]. In the test track trials the test vehicles navigated through artificially simulated horizontal deflection and chicanes. Experienced drivers were employed for these trials. The output of the study confirmed that the primary speed reduction factors were the stagger length, free view through the chicane, deflected path angle and the visual obstruction type (Figure 1). In this study, the free view width "*a*" had a positive value if the median island between opposing lanes allowed the driver to see the travel lane behind it at the road surface level. If, on the contrary, the driver approaching the island could not see the whole lane width at the road surface level past the island, "*a*" acquired a negative value. The wider was the median island, and thus the less of the travel lane at the road surface level was visible to the driver, the greater was the obtained speed reduction. These findings were confirmed by Zhang ed. al. [38], who, in addition, investigated reductions of noise and vehicle emissions.

(a)

(b)

(c)

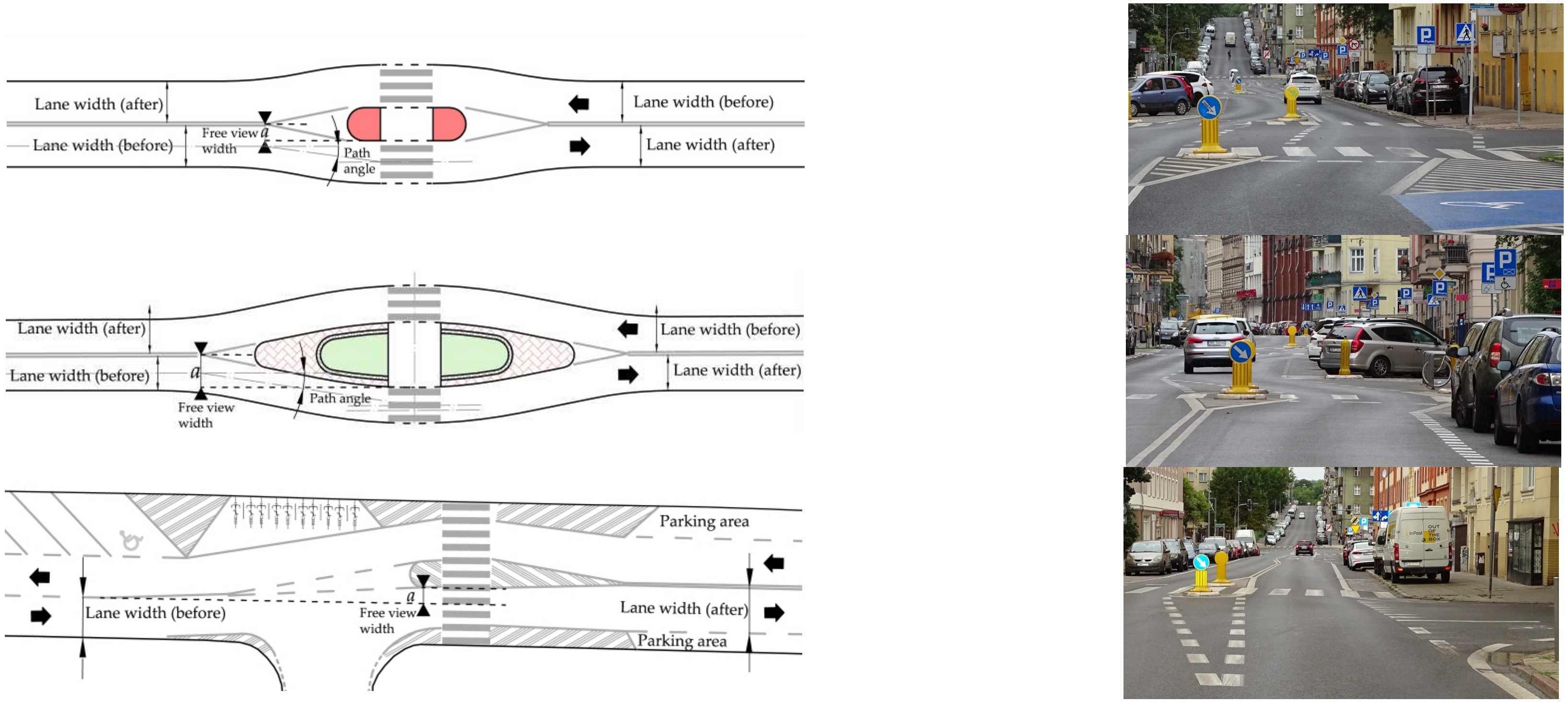

Figure 1. Free view and path angle illustration: (**a**) "*a*" - small; (**b**) "*a*" - larger; (**c**) "a" +. Source: own work.

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The above literature review allowed to compile in Figure 2 and compare the calculated 85th percentile and mean speeds noted just before the pedestrian crossing or chicane. Figure 2 shows a high degree of inconsistency of the data obtained by different researchers due to different locations (test tracks, transition zone, village centre, suburban two-lane single carriageway streets) and data selection. As regards the data selection, the researchers chose either to analyse free traffic flow only or use the steady traffic flow data with varying hourly volumes and separately the free traffic flow data.

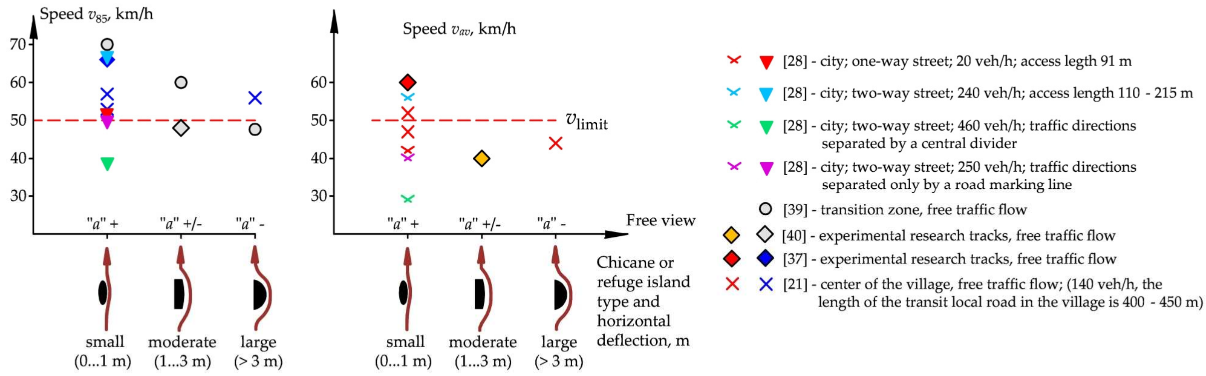

Figure 2. Comparison of v_{85} and v_{av} values ahead of a refuge island or median island in different locations. Source: own work.

Review of previous before-and-after speed studies with the use of heuristic method

Heuristic methods are used in management analyses when dealing with complex situations and lots of information. They allow to assess the efficacy of the analysed parameters based on the established determinants. The principles of this method were described by different scholars, including Juran (first edition in 1951) [41–43] and Deming [44] (first edition in 1982) and were elaborated by Kaoru Ihikawa who proposed seven basic quality tools for the Total Quality Management (TQM) system [45–47]. Quality management principles may be used successfully for assessments of other issues, including road maintenance [48,49], road operating speed management [50–54] or very specific applications, such as analysing fluid velocity variations in medical equipment [55]. The seven tools developed by Kaoru Ishikawa [45–47], i.e.,:

a. flow chart presenting the steps of the analysis,
b. check sheet, specifically statistical tests to check speed consistency among the consecutive survey sites deployed on the street under analysis,
c. normal distribution histograms,
d. scatter diagram showing relationships,
e. control chart showing speed changes along the analysed street,
f. cause and effect diagram (diagram fishbone or diagram Ishikawa's) for defining the primary and secondary factors,
g. Pareto chart to define the finally identified speed reduction determinants.

allow determination of factors that contributed to attaining the final effect in consideration. In traffic speed studies, the heuristic method allows to estimate the influence of the different determinants on the final operating speed reduced by various treatments, including TCMs. The above-mentioned seven tools of the heuristic method were used in this study for assessing the efficacy of different TCMs implemented in the analysed downtown street section.

The above literature review revealed that the research publications and various existing design guidelines have so far not covered the issue of efficacy of repeated and varied TCMs before refuge islands on two-way city streets. The purpose of this study was to find the most effective TCM configuration before refuge islands located on two-way streets in urban areas. TCM effectiveness is understood as reduction in the operating speeds to improve traffic safety as a result. Section 1 of this

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article presents the literature review on TCMs application near refuge islands and a general description of the heuristic method principles. In Section 2 the reader will find:

– information on the study site (a two-way city street with 50 km/h speed limit) and details of the respective study sections with different parking and TCM arrangements,
– traffic safety analyses before and after changes to the traffic organization plan,
– description of the heuristic method used in the study.

Section 3 presents the results of speed change analyses for the studied street sections. Section 4 discusses the obtained results and analyses the pre-defined determinants that, in combination with hourly traffic volumes, may cause operating speed reduction ahead of the refuge island. These analyses were made with the use of cause and effect diagram and Pareto charts. Section 5 presents conclusions that may be used by traffic engineers designing traffic calming for two-way city streets. The sequence of analyses as they appear in the article is presented in Figure 3.

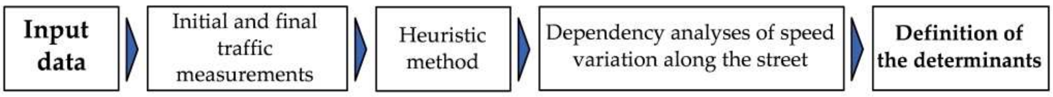

Figure 3. Sequence of analyses conducted in this study. Source: own work.

2. Materials and Methods

2.1. Study area

A two-way street in downtown Szczecin, Poland was chosen as the study site. In 2015 urban blocks alteration scheme was started in Szczecin in order to improve the transport network and in relation with the planned Metered Parking System MPS implementation. This required demarcation of metered parking spaces. Tempo 30 zones were introduced on some streets and various TCMs were implemented elsewhere in the area, as part of this road system alteration scheme [56]. This study deals specifically with a two-way street including demarcated parking spaces, refuge islands and horizontal deflection of the travel path, imposed by road markings and refuge islands (Figures 4 and 5). The street had 50 km/h speed limit. In Poland, 10 km/h allowance is applied in routine speed checks by means of speed cameras, as guided by relevant codes [57]. This allowance is deemed to account for the measurement and driver's errors, as the case may be.

Figure 4. Visualization of the analysed two-way street before the 2014 alteration showing traffic directions. Source: own work on a satellite image background Google Earth [58]).

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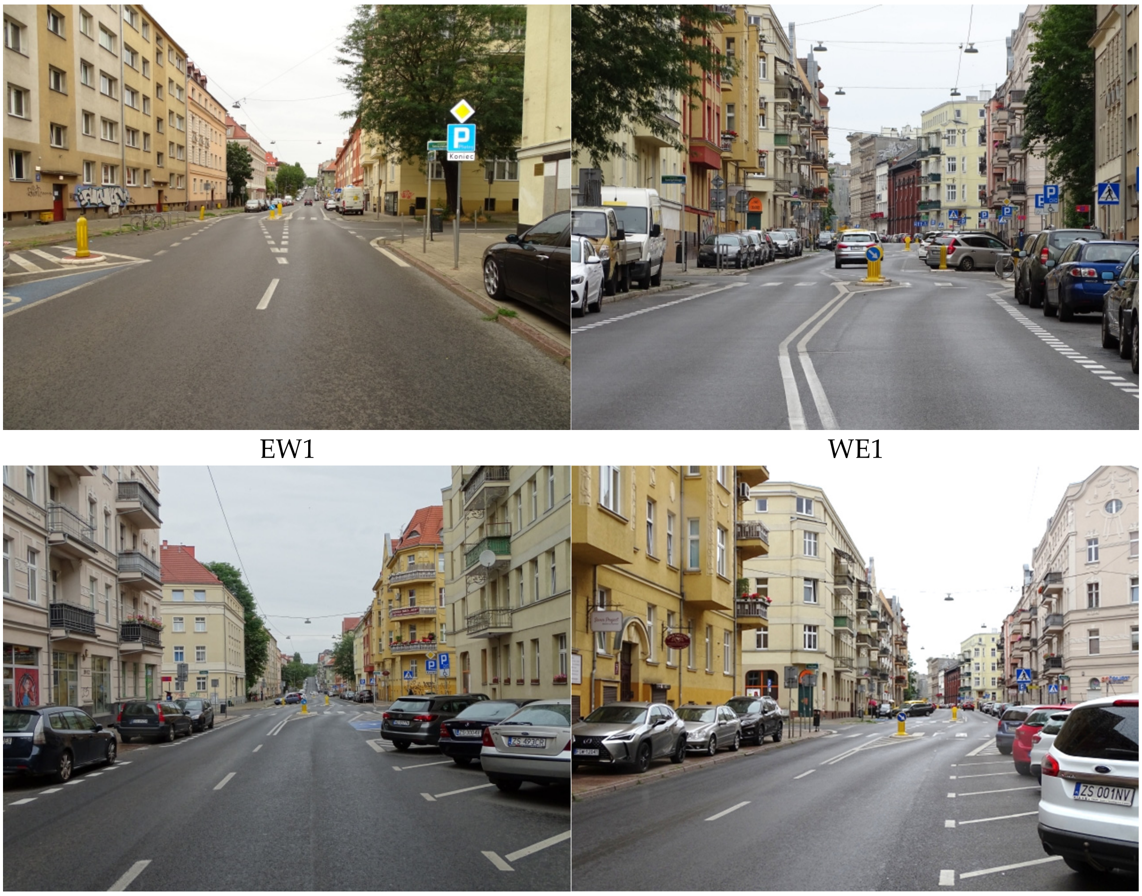
Figure 5. Visualization—four study sections located between three junctions were selected for the analyses of this study, on which the total number of sixteen survey stations were set up. Source: own work on a satellite image background Google Earth [58].

Considering the two-way traffic arrangement on the analysed street, the study sections were identified with geographical symbols and numbers (Figures 5 and 6). Thus the in the direction W→E sections between the signal-controlled junction and the roundabout were designated WE1, WE2 and WE3. Accordingly, the in the direction E→W sections located in the same area were designated EW1, EW2, EW3. All the study sections are shown in Figure 5 below. Geometrical features of the respective study sections are given in Figure A1 in Appendix A.

(**a**)



EW1 WE1

(**b**)

EW2 WE2

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(c)

EW3 WE3

Figure 6. Study sections: (**a**) WE1 & EW1; (**b**) WE2 & EW2; (**c**) WE3 & EW3. Photo A. Sołowczuk.

2.2. Traffic safety and volume count data for the analysed street

The effect of the changed traffic management arrangements on the analysed street was assessed through a road incident statistical analysis carried out using the Accident and Collision Registration System SEWIK [59] software program output data. These input and output data are given in Table 1 below.

Table 1. Input data and statistical analysis output for the analysed street. Source: own research based on data presented in [59].

Years	Traffic accidents in general	Pedestrian accidents
Before data: 2000.01.01–2015.12.31	27	6
After data: 2016.01.01–2023.05.31	6	1
The Chi-square significance test χ^2 was used to confirm or refute the efficacy of a given TCM and the resulting traffic safety improvement. Null hypothesis H_0: $\chi^2 = (n_1\, t_2 - n_2\, t_1)^2/(t_1\, t_2\, (n_1 + n_2))^1 \leq \chi_\alpha{}^2$; (no statistically significant difference exists). Alternative hypothesis H_1: $\chi^2 > \chi_\alpha{}^2$; (a statistically significant difference does exist). The following inequation should be satisfied at the same time: $n_1/t_1 > n_2/t_2$. Critical value $\chi_\alpha{}^2 = 3.84$ at the significance level $\alpha = 0.05$.		
$\chi^2 =$	$3.0 < 3.84$	$1.0 < 3.84$
$n_1/t_1 > n_2/t_2$	$1.7 > 0.8$	$0.38 > 0.13$

Legend: n_1—before-project road incidents/accidents, n_2—after-project road incidents/accidents, t_1—years before, t_2—years after.

The statistical test results compiled in Table 1 have not definitely confirmed the efficacy the changed traffic arrangements, i.e., less road incidents and vehicle/pedestrian collisions. However, the substantial growth of traffic on the analysed street in the time-span of the study must be taken into account at this point, as it could have some bearing on the number of road incidents. The traffic volume output data are given in Figure A2 in Appendix B. The cause of a higher traffic volume in one direction of travel is the local traffic arrangement with a two-lane one-way in the direction E→W street to the north and a two-way street to the south, the latter including a two-way tram line (Figure 4). This arrangement results in non-uniform traffic loading of the two travel directions with almost two times higher in the direction W→E traffic volume (Figure A2 in Appendix B).

2.2. Measurement and analysis method

In order to assess the slowing effect of the implemented modification of the traffic arrangements round-the-clock traffic count and speed measurement surveys were carried out on site for two days, i.e., for the total time of 48 hours on each of the sixteen survey stations. SR4 [60] synchronised traffic

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detection devices were used, mounted on the existing sign posts. The locations of the sixteen survey stations and deployment positions of the SR devices are shown in Figure 5.

Traffic counts and speed measurement surveys started on Friday morning and ended on Sunday. These weekend surveys lasted through May and June. The weather was dry during that time, ensuring uniform driving conditions. Four survey stations were deployed on each one-block section, positioned as follows: at the section (block) entry, just before the refuge island, within and past the junction. This deployment allowed observation of speed variation along a given portion of the street under analysis. In total, 16,000–18,000 travel speed readings were logged at each survey station.

Considering traffic volume daily variations (ranging from a few to dozen plus veh/h overnight to about 500 veh/h during the day), the data were subjected to a statistical analysis using the Two-sample Kolmogorov-Smirnov test and Median test in order to determine whether the hourly traffic volumes may be analysed as one group or must be treated individually. The authors conducted two-day preliminary traffic count and speed measurement surveys at two survey stations on the analysed street before and after metered parking and TCM scheme implementation. For a majority of the results, standard deviation was a variable statistic and negative results were obtained in both tests. Therefore, it was required for the statistical analysis purposes to split the speed data set into sub-sets corresponding to traffic volume intervals of 50 veh/h. The statistical test for four of these sub-sets for different traffic flow directions (including two "before" and two "after" sub-sets) are given in Table 2 below.

Table 2. Results of statistical tests to check whether speed data may be analysed as a single set. Source: own work.

No.	Traffic volume, veh/h	Traffic flow directions							
		Before measurement data				After measurement data			
		W→E	E→W	W→E	E→W	W→E	E→W	W→E	E→W
		Test K-S[1]		Median test [2]		Test K–S[1]		Median test[2]	
1	$N \leq 50$ & $50 < N \leq 100$	9.8	12.4	120.3	1392.4	14.3	15.6	2469.4	1544.9
2	$50 < N \leq 100$ & $100 < N \leq 150$	12.1	13.5	1737.1	308.2	17.9	17.4	2344.7	3113.2
3	$100 < N \leq 150$ & $150 < N \leq 200$	20.4	11.9	16893.3	189.6	16.9	20.9	1744.0	7370.2
4	$150 < N \leq 200$ & $200 < N \leq 250$	24.6	–	7490.0	–	17.3	25.4	2735.2	16035.3
5	$200 < N \leq 250$ & $250 < N \leq 300$	–	–	–	–	19.6	23.8	4905.4	5532.8
6	$250 < N \leq 300$ & $300 < N \leq 350$	–	–	–	–	24.2	19.3	26238.0	2549.3
7	$300 < N \leq 350$ & $350 < N \leq 400$	–	–	–	–	33.8	–	56117.9	–
8	$350 < N \leq 400$ & $400 < N \leq 450$	–	–	–	–	32.2	–	21759.6	–

[1] Two-sample Kolmogorov–Smirnov test λ: H_0: $F(v^{Ni}) = F(v^{Ni+1})$ and H_1: $F(v^{Ni}) \neq F(v^{Ni+1})$, $\lambda_\alpha = 1{,}36$, $\alpha = 0.05$.
[2] Median test: H_0: $F_1(x) = F_2(x)$ and H_1: $F_1(x) \neq F_2(x)$, $\chi_\alpha^2 = 3.84$, $\alpha = 0.05$.

Note also that since the lowest hourly traffic of up to 50 veh/h was recorded only during a few hours overnight, with the actual number of only dozen plus vehicles per hour collective analysis of these data with the daytime speeds measured at two- or even three times greater traffic volumes would not be in line with the design of experiments (DOE) principles. Yet another issue supporting the sub-division of the speed data set into hourly volume sub-sets was only sporadic crossing of the street or walking across or driving out of the parking spaces during night time.

The main objective of this study was to assess the efficacy of the applied TCMs used understood as speed reduction before the refuge island. To this end, the authors conducted preliminary measurements of initial velocity at which drivers applied brakes before the pedestrian crossing and the drive in and drive out speeds. These speeds were measured with SR4 synchronised traffic detector combined with a video camera during preliminary 1-hour long observations on two one-block sections with diagonal on-street parking. SR4 [60] logging chart example is shown in Figure A3 in Appendix C. These preliminary results were analysed and then the readings below 10 km/h were left out, as they were most likely associated with braking before the pedestrian crossing or driving in or out of the parking. The number of occurrences of these speeds in the dataset varied from just one to a several depending on the time of the day (boxed in blue in Figure A3 in Appendix C). Generally

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there were not more than 2-4% of such speeds in each hourly data set. An increased frequency of their occurrence (from a few to a dozen plus records) coincided with higher traffic volumes, i.e., 250–450 veh/h in the morning between 7 a.m. and 8 a.m. and in the afternoon between 4 p.m. and 6 p.m. The final speed analysis results with or without considering the readings below 10 km/ h in each sub-set were:

– 85th percentile speeds varied by up to 0.1-0.2 km/h,
– average speeds ranged 0.5-1.0 km/h.

The data recorded by the SR4 traffic detectors (boxed in green in Figure A3 in Appendix C): vehicle speed in km/h, headway in meters, time intervals, measurement date and time to one second accuracy and all the statistical data (values boxed in brown and red in Figure A3 in Appendix C). These data allowed carrying out other, supplementary analyses, for example to determine the effect of braking on the speed of the following vehicle. The results showed lower following vehicle speeds for up to 4 sec. time intervals between consecutive readings (boxed in blue in Figure A3 in Appendix 3), which depended on the headway to the decelerating vehicle. For time intervals greater than 4 sec., the following vehicle speed readings that depended on the headway to the decelerating or parking vehicles did not depart from the relevant mean speeds of other vehicles in the street. For the sake of consistency of the data used in the speed variation analysis, the readings below 10 km/h were left out in all analyses, this in line with the design of experiments (DOE) principles [61–63].

2.3. Research methods

The analysed parameters were 85th percentile speed, mean speed, speed reduction ratio, determined in the data sub-sets defined by hourly traffic volume ranges. As mentioned, heuristic method was chosen for the purposes of this study.

The sequence of the flow chart analyses is shown in Figure 7. Standard statistical analyses are conducted as the first step (Figure 7), including normality test, plotting histograms of the factors under analysis, Two-sample Kolmogorov–Smirnov test and median test. The third tool of the heuristic method used in this study were scatter diagrams relating the vehicle speeds to the hourly traffic volumes (Figure 7). Relationships between v_{85} and v_{av} on the one hand and the hourly traffic volumes on the other were obtained in almost all cases with the correlation coefficient greater than 0.7. However, this relationship was not confirmed for entry to and exit from a signalled junction or roundabout. On all other survey stations, both these speeds were found to depend on the hourly traffic volume. The fourth tool of the heuristic method used in this study were 3D diagrams (Figure 7) representing speed and speed reduction ratio variations between the block entry and the refuge island. The speed and speed reduction ratio distributions turned out useful and were used to define the determinants associated with the refuge island itself and its visibility to the driver.

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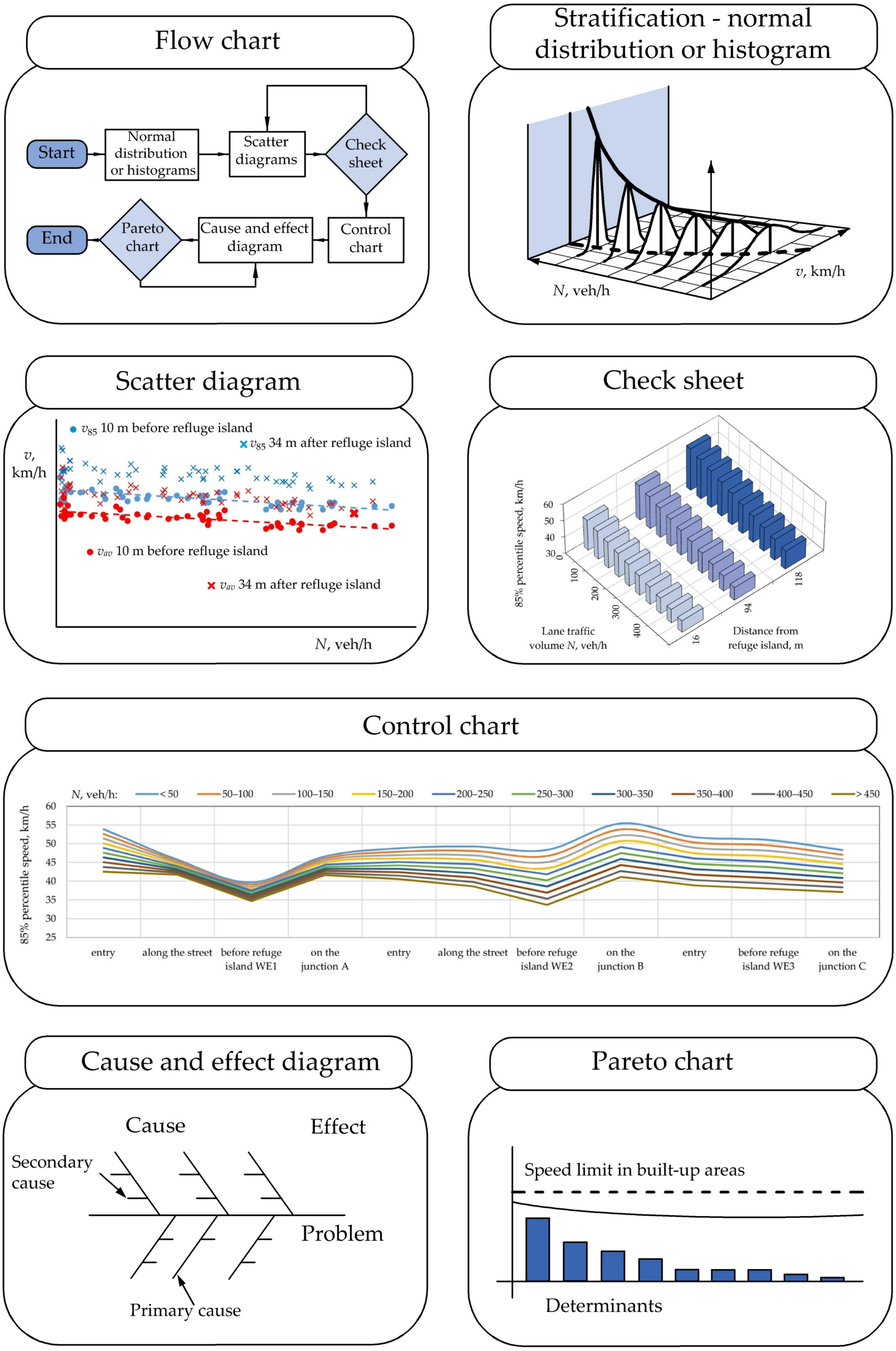

Figure 7. The seven basic tools of the heuristic method used for analysing the efficacy of speed reduction treatments before refuge islands. Source: own work.

The fifth tool were 3D and linear control charts of speed changes and speed reduction ratios along the street under analysis (Figure 7). The analyses of the geometric and qualitative parameters,

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various speed distributions and statistical test results were used to initially define the determinants. These determinants were presented in the Ishikawa's cause and effect diagram, the sixth tool of the heuristic method applied in this study. A division into primary and secondary causes of the analysed slowing effect was made at this point. It was assumed that these determinants may be related to each other or independent. Stratification or concordance matrix are applied when dealing with a large number of determinants, most conveniently represented in the Pareto charts, the seventh tool of the heuristic method. In the Pareto chart the determinants were rated in the order of decreasing effect, i.e., from the lowest to the highest approach speed or from the greatest to the lowest speed difference between the block entry and the refuge island station. The determinants were assessed in two ways: as a series of speed values before the refuge island and speed differences related to a given determinant or using an illustrated, summed up number of determinants confirmed on a given study section. The adopted sequence of the heuristic method analyses allowed to identify the refuge island approached at lowest speeds or featuring the greatest speed difference on the approach section (Figure 7) and, as the final outcome, also identify the relevant determinants. In the summary of the conducted analyses, it will also be possible to identify the most effective among the applied TCMs. The control charts, in turn, allowed determination of treatments having a prolonged slowing effect also past the terminal junction, i.e., in the next section of the street.

3. Results

As mentioned, the speed data set was sub-divided into sub-sets defined by hourly traffic volume ranges. Considering the amount of data from the round-the-clock two day speed survey with about 16,000–18,000 readings per one SR4 detector, it was necessary to decide on the appropriate approach to be taken in the subsequent analyses. The parameters considered in previous studies [28–30,35,36] were the 85th percentile speed and the mean speed while in traffic safety analyses mean speed was considered [12,22,27,65–69] Kruszyna & Matczuk-Pisarek [31], in turn, used a speed reduction ratio, and Distefano et al. [29,30] expressed the speed reductions between the point in front of refuge island and some distance earlier in percent. Jamroz et al. [25] used solely the 85th percentile speed as this parameters is used for the purpose of speed limit analyses and the associated selection of the appropriate speed limit sign. From this wide selection of the available speed parameters, 85th percentile speed, mean speed and speed difference (between the section entry and refuge island) were chosen for the purposes of this study, i.e., TCM efficacy assessment. Having in hand such an extensive database, it was possible to consider in this study also the hourly traffic volume effect. The analysis of 24-hour speed data with the measurement time given to 1 sec. accuracy (Figure A3 in Appendix C) showed that the so far used free-flow speed may be deemed to correspond to the values obtained at hourly traffic volume below 50 veh/h. However, one should bear in mind that these speeds concern mainly the night period when they are not influenced by pedestrian traffic (Appendix B Figure A2).

As per the adopted methodology and the statistical test results (Table 2), the speed data normality and stratification depending on the hourly traffic volume and the survey station location in relation to the analysed refuge island were checked as the first step. The obtained results of speed changes at the refuge island are shown in Figure 8. Figure 8 shows the 85th percentile speed distribution among three survey stations for all the six analysed street sections along the refuge island approach section. The obtained speed changes presented in Figure 8 show a strong relationship with the hourly traffic volume and other factors, including those related to the implemented TCMs. These are most likely the determinants sought in this study.

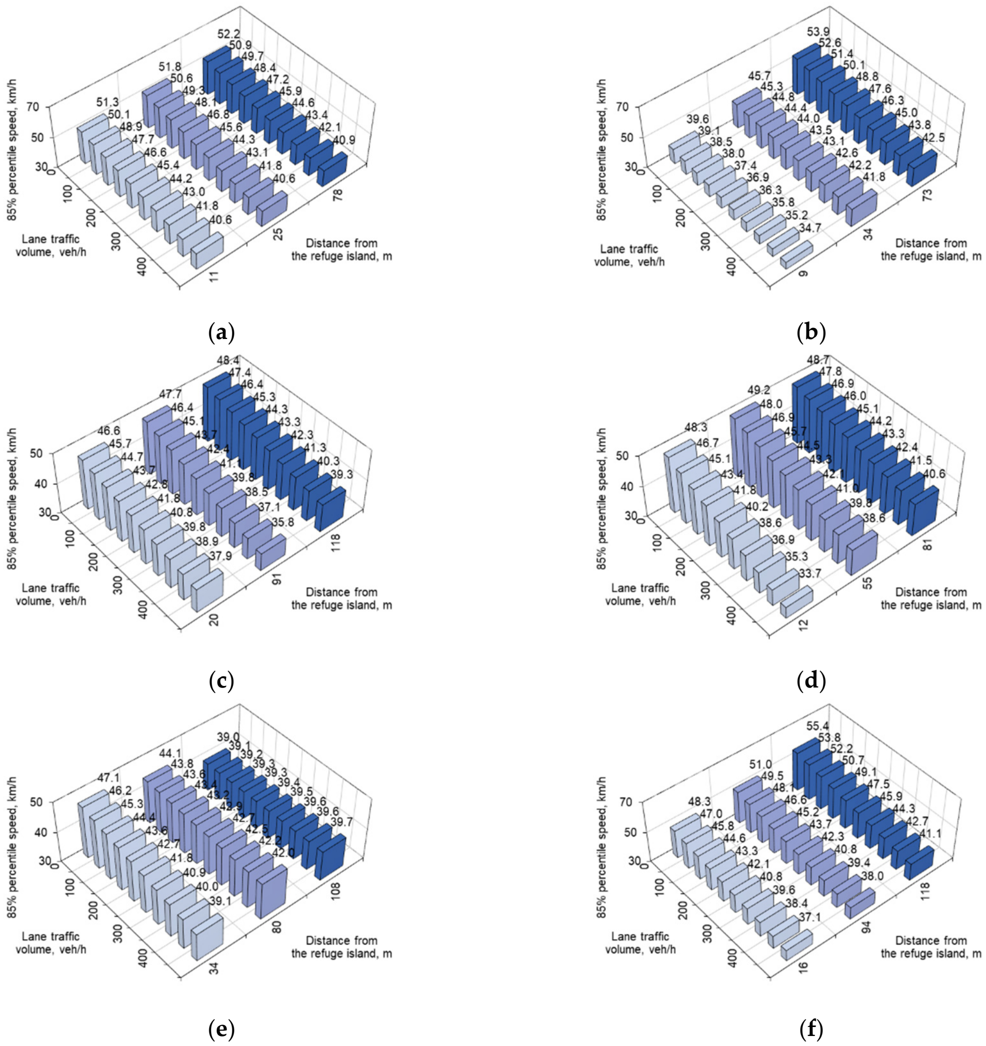

Figure 8. 85th percentile speed distribution example: (**a**) EW1; (**b**) WE1; (**c**) EW2; (**d**) WE2; (**e**) EW3; (**f**) WE3. Source: own work.

The values of v_{85} and v_{av} were calculated for all the survey stations and each survey hour. Next the speed results data set was subdivided into hourly traffic volume sub-set and regression analyses were carried out. Appropriate regression relationships were obtained for all the results. Considering low hourly volumes overnight (not exceeding dozen plus veh/h) larger scatters of v_{85} and v_{av} were obtained only for the up to 50 veh/h range. Illustrative speed vs. hourly traffic volume relationships are given in Figure 9.

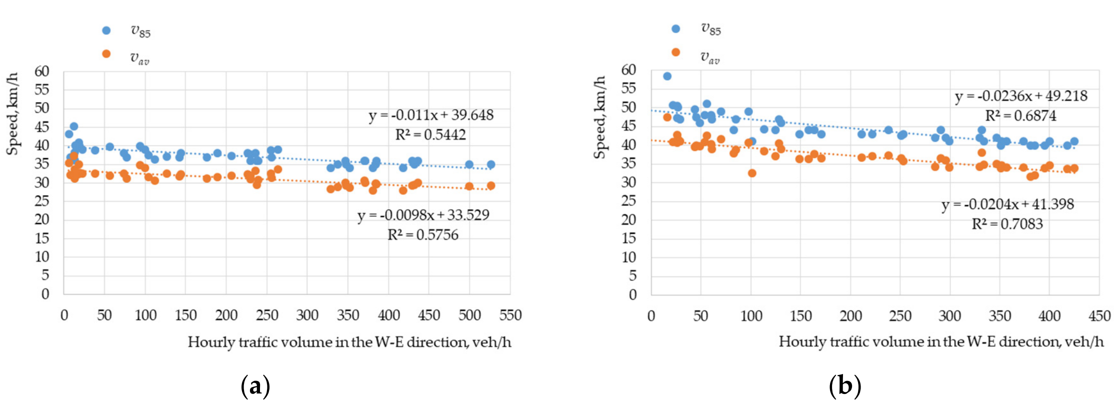

Figure 9. Regression analysis examples for different survey station locations: (**a**) at a refuge island; (**b**) on the refuge island approach section. Source: own work.

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Linear control charts were the next heuristic method tool used in this study. Specifically, these were linear speed change diagrams along the analysed street sections (Figure 10). From the graph in Figure 10 it can be figured out that not all the refuge island related TCMs should be considered effective. The greatest speed variations were noted on the direction W→E sections WE1, WE3 and WE2 (Figure 10a,b). In the direction E→W the speed changes were only minute. Figure 10c,d show also an increase of speed past the refuge island EW1 associated with widening of the carriageway to two travel lanes ahead of a signalled junction and signal phases rather than the applied TCMs.

(a)

(b)

(c)

(d)

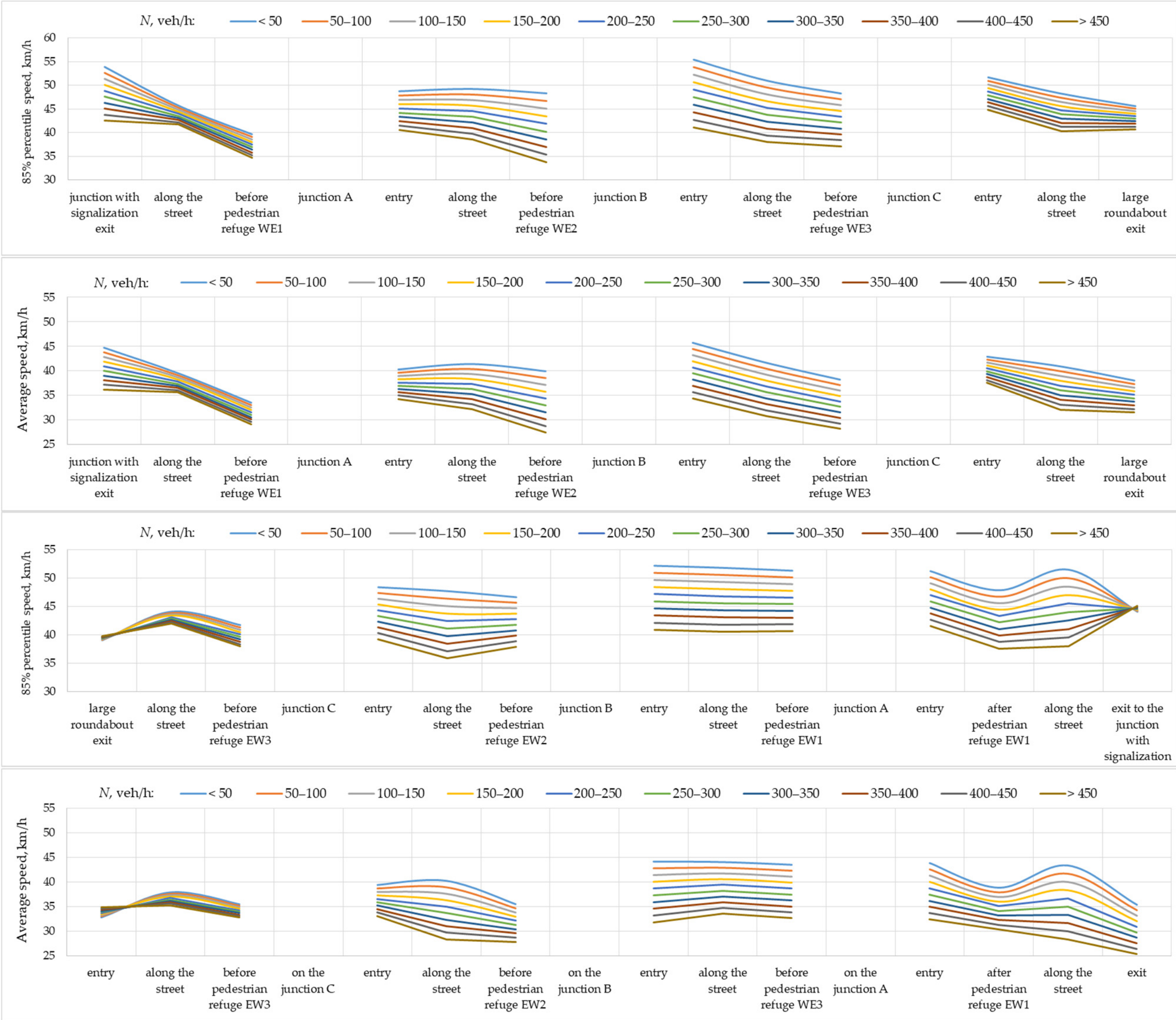

Figure 10. Speed distribution along the analysed street: (**a**) 85th percentile speed in the direction W→E; (**b**) average speed in the direction W→E; (**c**) 85th percentile in the direction E→W; (**d**) average speed in the direction E→W. Source: own work.

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4. Discussion

4.1. Primary and secondary determinants—cause and effect diagram

Figures 11a,c show the driver's central vision area at different speeds. In order to reflect the 50 km/h speed limit on the analysed street (i.e., the statutory built-up area speed limit in Poland), the driver's central vision area at this speed was represented by a colour area on the images of the respective refuge islands, turned to grey scale in the fringe vision area. The latter is also a focal vision area, yet it requires the driver to move head and direct eyes sideways while driving. In line with the heuristic method principles and guidelines to use the cause and effect diagram to identify the determinants, the probable determinants noted on the analysed refuge island are presented in Figure 11b as the next step of the mentioned analyses. The probable determinants in Figure 11b were identified initially based on the TCMs located at the refuge islands shown in Figure 11a,c.

The parameters recorded on the analysed refuge islands showed relevance of free view width "*a*" in line with the already published findings [34–36]. The studies described in [34–36] investigated the effect of horizontal deflection treatment located before a median island on speed reduction past the island. It is a different case in this article, where we assess the slowing effect of TCMs and refuge island on the approach section to the latter. A double horizontal deflection treatment with 1.3 m offset to the right, followed by a 3.3 m offset to the left was found only in the study section WE1 (Figure 11c). The free view width "*a*" was large there, encompassing the whole lane with past the refuge island. The greatest speed differences were noted in this case, in all hourly traffic ranges. A 1 m offset (equal to half the refuge island width) to the right was noted at WE3 (Figure 11c). The study sections WE2 and EW2 (Figure 11a,c) featured a horizontal deflection to the right, clearly visible to the approaching driver, located before the junction with half the refuge island width offset, and a horizontal deflection to the right with the same offset on the section past the junction. The sections EW1 and EW3 had no deflections, giving small if any speed differences noted there (Figure 11a). Horizontal deflection and free view are intrinsically linked to visibility, as shown in Figures 11 and 12. Figure 12a,c also show the driver's central vision area against the clear sight width at the road surface level before the first refuge island (green area) and past the second refuge island (blue area). The issue of visibility at refuge islands was dealt with by several researchers [25,70–76]. The vision field depends on the driving speed and stopping distance (braking distance plus reaction distance) [72,74,76–80]. In the above-mentioned articles vision fields varied, as besides the driving speed they depended on different in various countries standard reaction times and decelerations and also the road surface condition and the longitudinal profile of the carriageway.

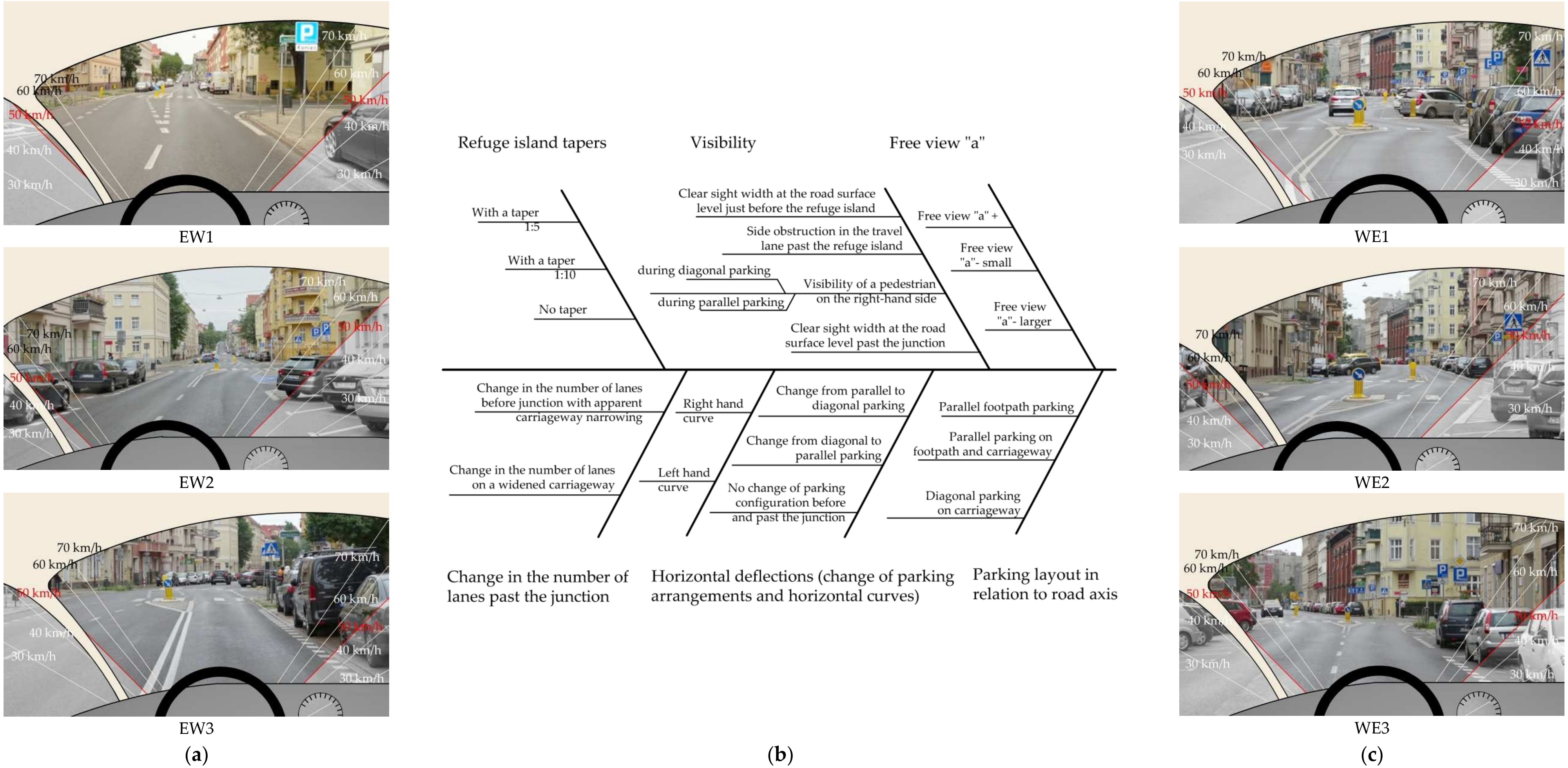

Figure 11. Cause and effect diagram and identification of determinants: (**a**) windscreen view in the direction E→W; (**b**) identification of primary and secondary determinants; (c) windscreen view in the direction W→E. Source: own work.

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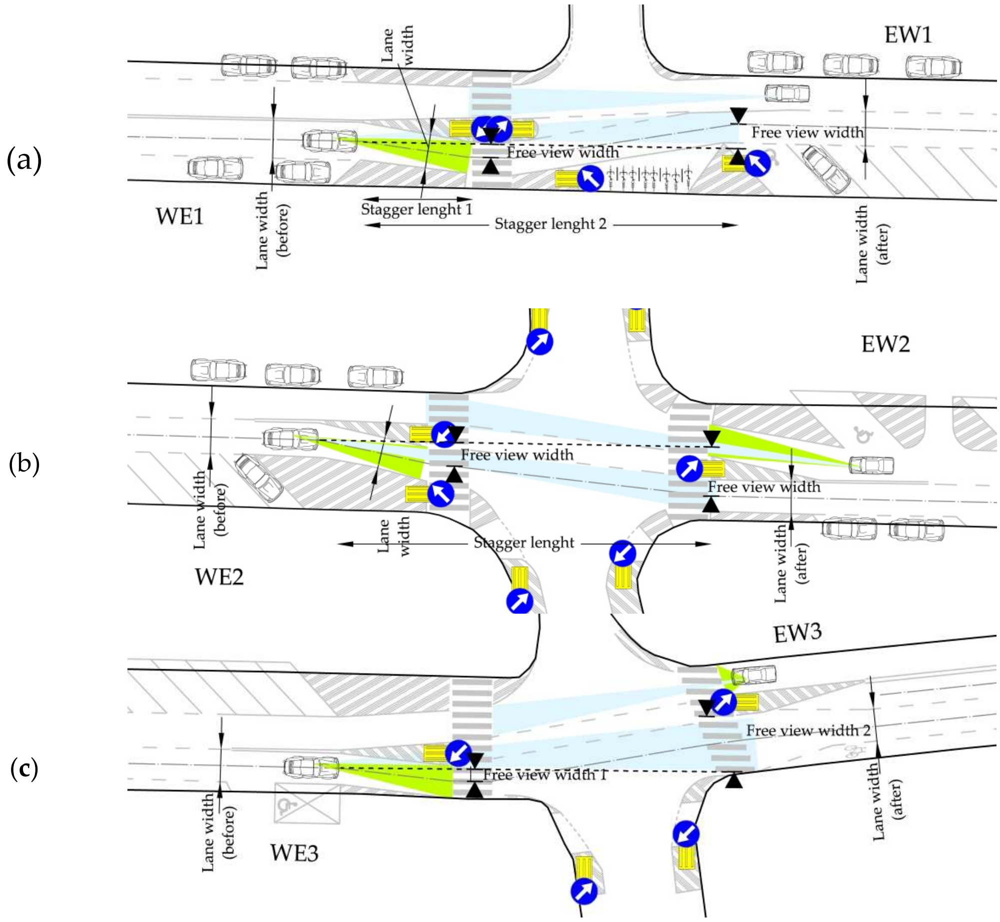

Figure 12. Free view width and clear sight width ahead of and past the refuge island: (**a**) WE1 and EW1; (**b**) WE2 and EW2; (**c**) WE3 and EW3. Source: own work.

For the purposes of this study 13 m clear sight width was adopted as per the guidance of [77]. The field of vision analysis presented in Figure 12 showed that in horizontal deflection layouts the fields of vision add up and a driver approaching the refuge island for pedestrians, sees a vehicle parked past the island as a side obstacle with a clear sight width past the island obstructed by the traffic signs located on the island (Figure 11a). The combined effect of these determinants may be considered the most likely cause of the large speed reduction on the approach to WE1. The layout of traffic signs located on the refugee islands and the horizontal and vertical curves in the C junction also partly restricts the clear sight width past the WE3 island and past the junction (Figure 12c). Horizontal deflection was also found there. However, the geometrical features, two lanes past the junction, a cycle lane and cars parked on the footpath, were found to have less effect on the speed reduction before WE3 [3]. On the next section WE2, the configuration of these geometrical features was found to have much less effect on the speed reduction obtained before the refuge island (Figure 12b).

On the sections located in the opposite traffic direction the geometrical features and the TCMs were not found to have any significant slowing effect. EW1 may be an exception to that, where the view of cars parked partly on the carriageway on the travel lane right-hand side and restricted view past the island due to traffic signs positioned on the refuge island could possibly have some effect on the small anyway speed reduction (Figures 6a and 12a).

The next of the identified determinants concerns the applied painted taper before the refuge island. It is similar to the path angle issue addressed in [35]. However, the TRL trial data [35] cannot be compared with the results of this study, as the former (speed data from over a dozen TRL track rides by experienced drivers) lack the traffic volume information. Nevertheless, the TRL trial data may still be roughly compared with the results obtained in this study at 50 veh/h traffic volume level. Note that the data given in this article relate to the actual traffic conditions in an existing street, taking

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into account a number of other determinants. The taper design varied among the analysed refuge islands, with 1:5 tapers on WE1, WE2 and EW2 and 1:10 taper on WE3. EW1 and EW3 islands had no tapers at all (Figure 12). The issue of tappers in front of refuge islands was tackled in [25]. It was concluded there that 1:5 taper should be used in city traffic conditions, possibly with one or more painted tapers before. However, it should not be used at entrances to sections that include TCMs.

A determinant that may affect the desired slowing before refuge islands could also be a variation in the number of lanes heading in one direction before and past the junction. Such variation in the number of lanes occurs in the terminal sections WE3 and EW1 shown in Figure 11 above and in Figure A1 in Appendix A. The next three determinants identified for the purposes of this study are related to the travel lane geometry (arrangement of straight sections and curves on the approach to and within the junction) and parking orientation on both sides of the street. Change of parking configuration from parallel to diagonal, or vice-versa, imposes a horizontal deflection, use of horizontal curves and the associated road markings (Figures 4, 11 and 12).

4.2. Analysis of determinants based on Pareto chart

The identified determinants, as defined in section 4.1 above, were assessed using logical tautologies. Thus, if a determinant was confirmed on a given section on the approach to the refuge island it got quantification measure score of 1 as per the binary system. Otherwise it got 0 score. In some cases, 0.5 score was given as an intermediate value. This includes free view "*a*" - (small) situations, in line with the conclusions of [37] (WE2 and EW2 in Table 3). Similarly, an intermediate score was given for 1:10 painted taper, this in line with the recommendations of [25] (Table 3 - WE3). Intermediate scores were given also where left hand curve was found in the junction (WE3 in Table 3), as the horizontal curve configuration had a direct bearing on the visibility of the road section past the junction. A possibility of apparent carriageway narrowing past the junction was confirmed in two cases, possibly due to compromised visibility of the road surface past the junction (WE3 and EW1 in Table 3). The determinants used on the analysed sections and their quantification scores are summarised in Table 3. Table 3 includes only the determinants that were found on the sections under analysis.

Table 3. Determinants found on the analysed sections and quantification measure scores. Source: own work.

Determinants	Scores given to the study sections					
	WE1	**WE2**	**WE3**	**EW1**	**EW2**	**EW3**
Free view	1	0.5	1	0	0.5	0
Side obstruction in the travel lane past the refuge island	1	0	0.5	0	0	0
Lack of visibility of a pedestrian on the right hand side of the island	1	0	0	0	1	1
Lack of visibility of the road surface past the junction	0	0	0.5	0	0	0.5
Painted taper applied on the section	1	1	0.5	0	1	0
Left hand curve in the junction	0	0	0.5	0	0	0
Parking configuration changed from diagonal to parallel	0	1	0	0	1	0
Parking configuration changed from parallel to diagonal	1	0	0	0	0	1
Apparent carriageway narrowing past the junction	0	0	0.5	0.5	0	0
Total quantification measure scores:	**5**	**2.5**	**3.5**	**0.5**	**3.5**	**2.5**

The classified quantification measures assigned to logical tautologies were separately and jointly. The Pareto chart (Figure 13a) shows the 85th percentile speeds before the refuge island and their total scores. The upper part of the Pareto chart shows stratification of the 85th percentile speeds recorded before the refuge island at different hourly traffic volumes and the bottom part shows the confirmed determinants and total scores of the quantification measure applied to them. Having a closer look at the data presented in Figure 13a we see that speed variation at the refuge island depends on the combined effect of all the above determinants and the traffic volume rather than any one of them on their own. The 85th percentile speed variations on the respective sections showed their strong relationship with free view and visibility parameters. That said, a strongest relationship was found for combination of the confirmed logical tautologies and the hourly traffic volume,

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especially for the volumes greater than 150–200 veh/h (Figure 13). Similar analyses are presented for the mean speed variations before refuge islands (Figure 13b). The mean speed analysis showed that it depended even stronger on the hourly traffic volume above the 100–150 veh/h range, and on the combined effect of the following determinants: free view, visibility, change of parking configuration and painted taper. However, this dependency is very complex. The joint analysis of the mean speed variation, traffic volumes, and the determinants showed that the more determinants are involved (including TCMs) the greater their overall effect on the speed of vehicles approaching a refuge island. For example, without painted tapers, with travel lane visibility before the island, without visibility of pedestrians approaching the island from the right hand side and poor visibility of the road surface past the junction on EW3 or no confirmed determinants as was the case on EW1 we got 85th percentile speed of about 50 km/h (Figure 13a), i.e., the statutory built-up area speed limit in Poland and 40–45 km/h mean speed (Figure 13b). Now, based on the above analyses, we can rate the analysed TCMs and other factors noted at the refuge islands as:

(a) effective (WE1)—with a change of on-street parking configuration from parallel to diagonal or vice-versa requiring the driver to change the travel path, 1:5 taper or road and island geometry designed to get free view "*a*" - larger so that a vehicle parked in the travel lane is visible as a side obstacle and the travel lane at the road surface level past the island is not visible by the driver approaching the island, altogether resulting in lower island approach speeds;

(b) moderately effective (WE2, WE3 and EW2) with narrower free view width of "*a*" - small, 1:5 or 1:10 painted taper and change in parking configuration and different ways of targeting parking spaces, which in combination produce different geometry and visibility configurations offering the driver a reliable assessment of the road situation during approaching and passing the island and resulting in moderate speed reduction;

(c) ineffective (EW2 and EW3) with "*a*" + free view, no change of parking configuration, no painted tapers, no horizontal deflection and no sight restrictions for the driver approaching and passing the island, discouraging speed reduction.

As mentioned, the speed reduction analyses conducted in this study considered, besides the 85th percentile speed, also the relative speed reduction in percent, calculated as a ratio between the 85th percentile approach speed and the block entry speed [29] and various other speed reduction indicators [31]. However, these parameters were assessed in relation to the free flow speed. Having in hand the two-day 24-hour survey data it was also possible could also analyse the effect of the geometrical features and TCMs in combination with hourly traffic volumes on the calculated approach speed parameters v_{85} and v_{av}, estimated before refuge island. It was found that v_{85} and v_{av} variations depended the hourly traffic ranges from free-flow conditions to the maximum of 500 veh/h at 50 veh/h intervals. The underlying cause of the two-day continuous survey was to support determination which of the analysed six pedestrian refuge arrangements turns to be the most effective in real traffic conditions (with 0–500 veh/h hourly traffic volumes) rather than in free-flow situation.

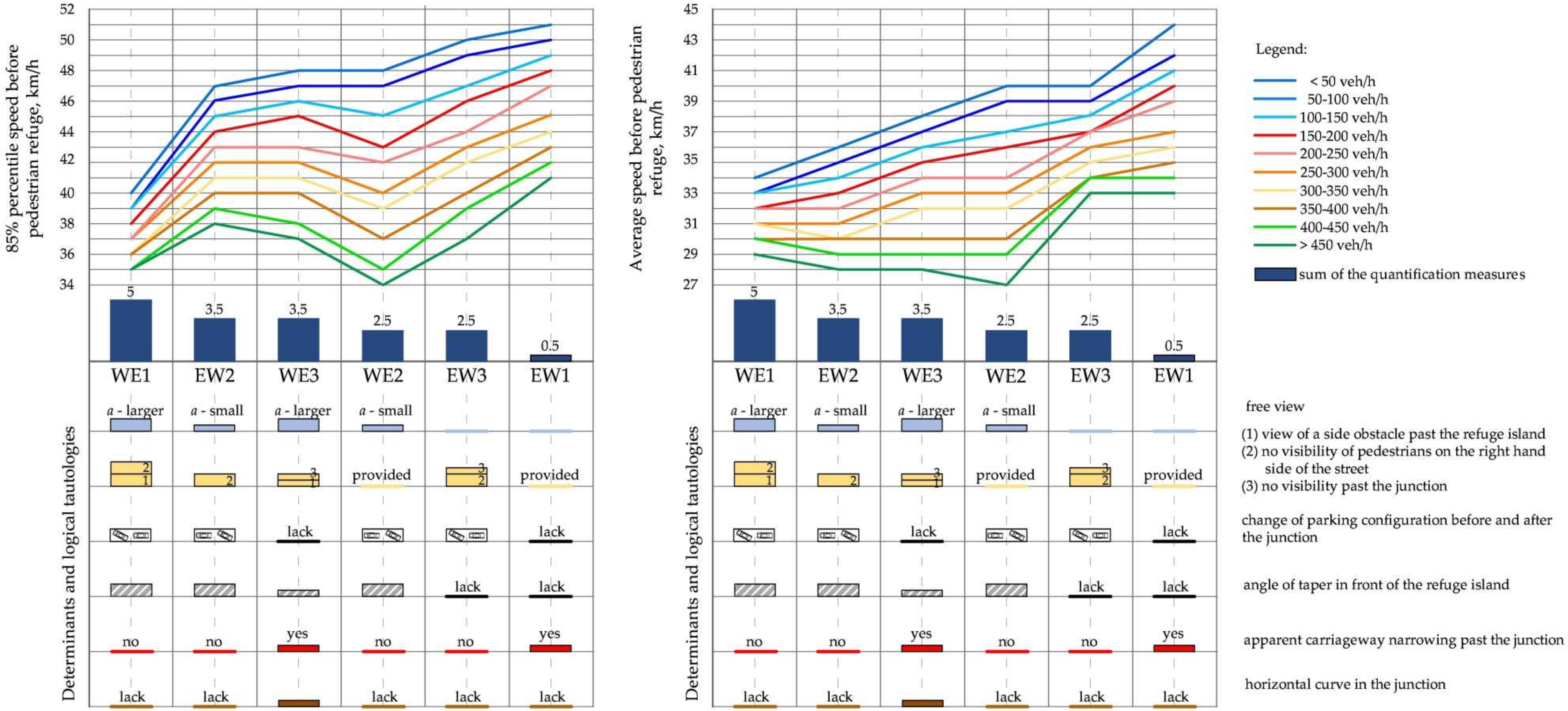

Figure 13. Pareto chart showing the variation of speed distribution parameters (v_{85} and v_{av}) and the determinants adopted for the analysed sections: the sections are listed in the growing order of v_{85} just before the refuge island; and the test sections are listed in the order v_{av} just before the refuge island. Source: own work.

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Next, heaving in mind the speed change effect on noise, safety and vehicle emissions, i.e., the factors intrinsically related to sustainable road construction, we compared the values of Δv_{85} and Δv_{av} noted before the refuge islands with the block entry values of v_{85} and v_{av} (Figure 14). A detailed analysis of the obtained hourly traffic volumes together with the adopted determinants on Δv_{85} and Δv_{av} the island approach speed difference depended on the identified determinants and the hourly traffic volume on a majority of the analysed sections. Noteworthy, this relationship was found to vary depending on the island geometry and the specific TCMs and traffic volume compilation. Highly relevant in this respect were the free view width and visibility of side obstacles, pedestrians and the road surface past the junction. Where the various determinants were confirmed (regarding the free view, visibility, painted taper angle and change of parking configuration), as the hourly traffic intensity increased, Δv_{85} and Δv_{av} were found to decrease on the approach to the island. This may be due to lower block entry speeds or higher speeds just before the refuge island. The lowest speed difference was obtained for EW1 where only free view of "*a*" - small and apparent carriageway narrowing past the junction were noted. Without a horizontal deflection or changed parking configuration the section offers very good visibility on the approach to and past the pedestrian island while the view of two travel lanes heading in the same direction past the junction and of the cantilevered traffic lights was found to have no slowing effect. WE2 is one exception in this analysis, in that it offered good view on constant geometry travel lane and on the cars parked on the footpath parallel to the road past the island and past the junction, despite the free view "*a*" - small, 1:5 painted taper and a change of parking configuration. This being so, the obtained values of Δv_{85} and Δv_{av} most probably depended on the hourly traffic volume only. In the case WE2 and EW3 the growing hourly traffic volume gradually increased the difference of these two speed parameters. On EW3, the travel lane does not change its geometry before the island and the change in the parking configuration past the island has no significant effect on the analysed speed differences before the island.

The above findings are apparently consistent with the findings of the simulator study by Akgol et al. and Aydin et al. [33,34]. The difference between these studies was in single lane narrowing of the two lane carriageway at the pedestrian crossing which is not the case in this study where there are two travel lanes running in opposite directions in all cases. For economic reasons, the authors of [33,34] recommended a one-sided splitter island on the right hand side of the travel lane before the refuge island, as is the case in WE2 and EW2 sections analysed in this study. Comparing their recommendations and the results presented in Figures 13 and 14, it can be concluded that this arrangement would not be effective for refuge islands located on junctions. The second, more expensive option recommended in [33,34] are two one-sided islands on either side of the refuge island, as in our study section WE1. The above findings are apparently consistent by recommending for refuge islands located in junctions horizontal deflection by three islands, resulting in flattened U deflected path of travel instead of flattened "S" shape and one-sided island on the right hand side of the approach to the pedestrian refuge.

The findings of this study are also highly consistent with the findings presented in [28] despite different study areas and different countries with different tempers and driving behaviours found there. In both cases lane narrowing was found to be the most effective treatment at refuge islands, accompanied by horizontal deflection, additional one-sided splitter islands on both ends of the island and painted tapers, that is the use of a few TCMs deployed within a short distance.

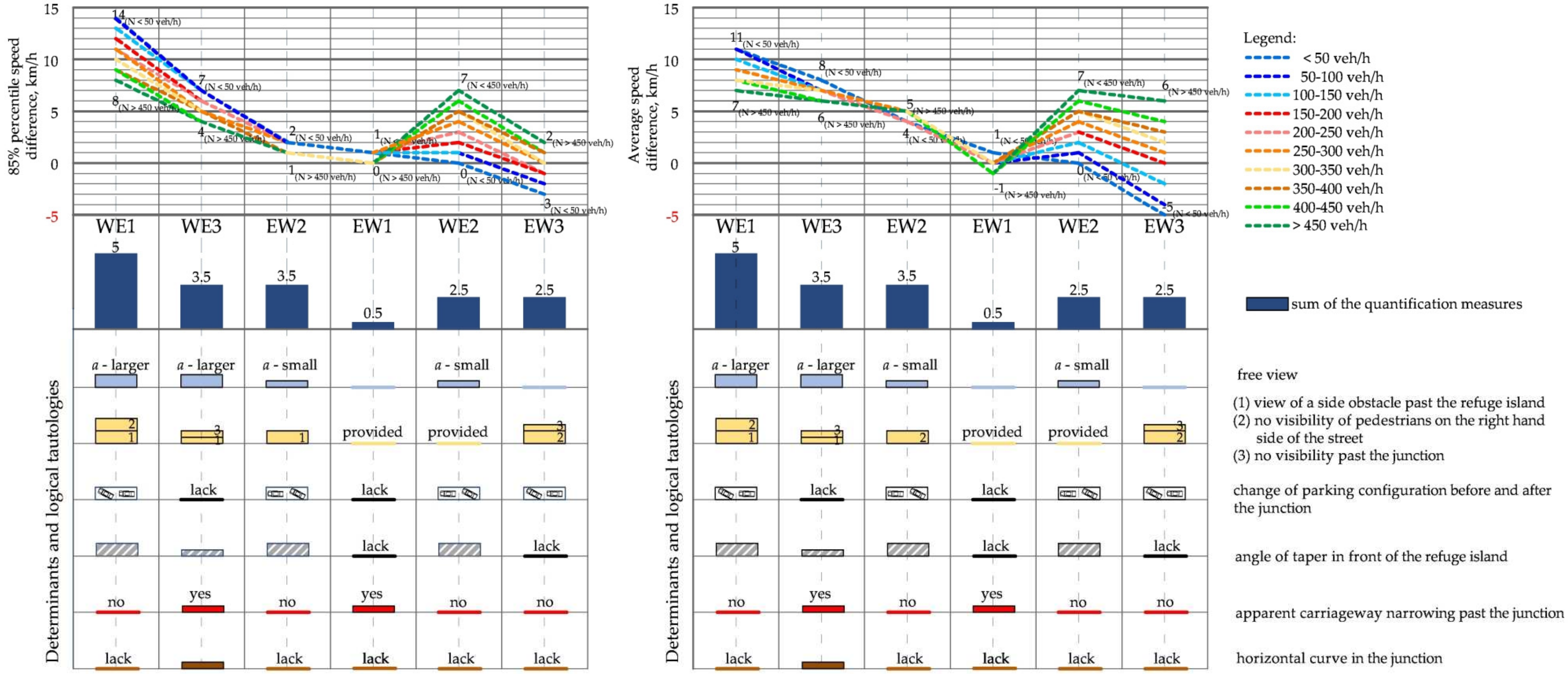

Figure 14. Pareto Chart 2—showing the values of Δv_{85} and Δv_{av} and the determinants adopted on the respective sections: sections listed in the order of Δv_{85} values calculated as $\Delta v_{85} = v_{85}$ (entry) − v_{85} (before the refuge island); and sections listed in the order of Δv_{av} values calculated as $\Delta v_{av} = v_{av}$ (entry) − v_{av} (before the refuge island). Source: own work.

5. Conclusions

As the literature review showed, studies on refuge island treatments date back to the late 1990s. Nevertheless, the international experiences show that the issue has not be studied completely as yet, and we still do not have definite design principles at hand. The speed reducing effect of pedestrian refuges and, more importantly, improved safety of pedestrians have been demonstrated by the study results of many researchers. The available design guidelines focus on the island width and angle of painted taper applied in front of the island. Less attention is paid to the conditions relevant to the visibility of pedestrians on the way to the refuge island, visibility of the nearby junction at the road surface level, visibility of side obstacles, specifically cars parked at a small distance past the island and also to the effect of hourly traffic volume on the degree of slowing on the way to the island. The relevance of these determinants has been demonstrated in this article. The speed analyses conducted as part of this study in the refuge site locations confirmed high relevance of free view and visibility of pedestrian and the refuge island surroundings. The change in the parking configuration on the way to and past the island and angle of the applied taper were found to have a lesser effect. Also highly relevant was the compilation of the above determinants, considered in combination with hourly traffic volumes.

Comparing the results of this study with the former study results we also found that the various speed parameters and ratios were so far compared in free-flow traffic conditions, thus ignoring the effect of the traffic volume. The relevance of traffic volume was demonstrated in this study, since the drivers tend to react differently when exposed to a higher number of stimuli present on a two-way road, as compared to the test track or simulator based situations.

The determinants whose relevance has been proven in this article show a need for further analyses and studies to be conducted by other researchers to consider different drivers' tempers, engineering experiences and cutting edge technologies applied in various island, lighting and structural details. A complete set of design guidelines would also be desired, that would consider, besides the island width and the island taper angle also the design parameters of the junction ahead, visibility parameters and townscape surrounding. Only when considered altogether, these recommendations will ensure safe design of pedestrian refuges and the associated speed management in downtown streets. Steadier speeds being a probable consequence, the desired reduction of noise and vehicle emissions around refuge islands may well be expected.

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Conflicts of Interest: The authors declare no conflict of interest.

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Appendix A

Study sections parameters

(a)

(b)

(c)

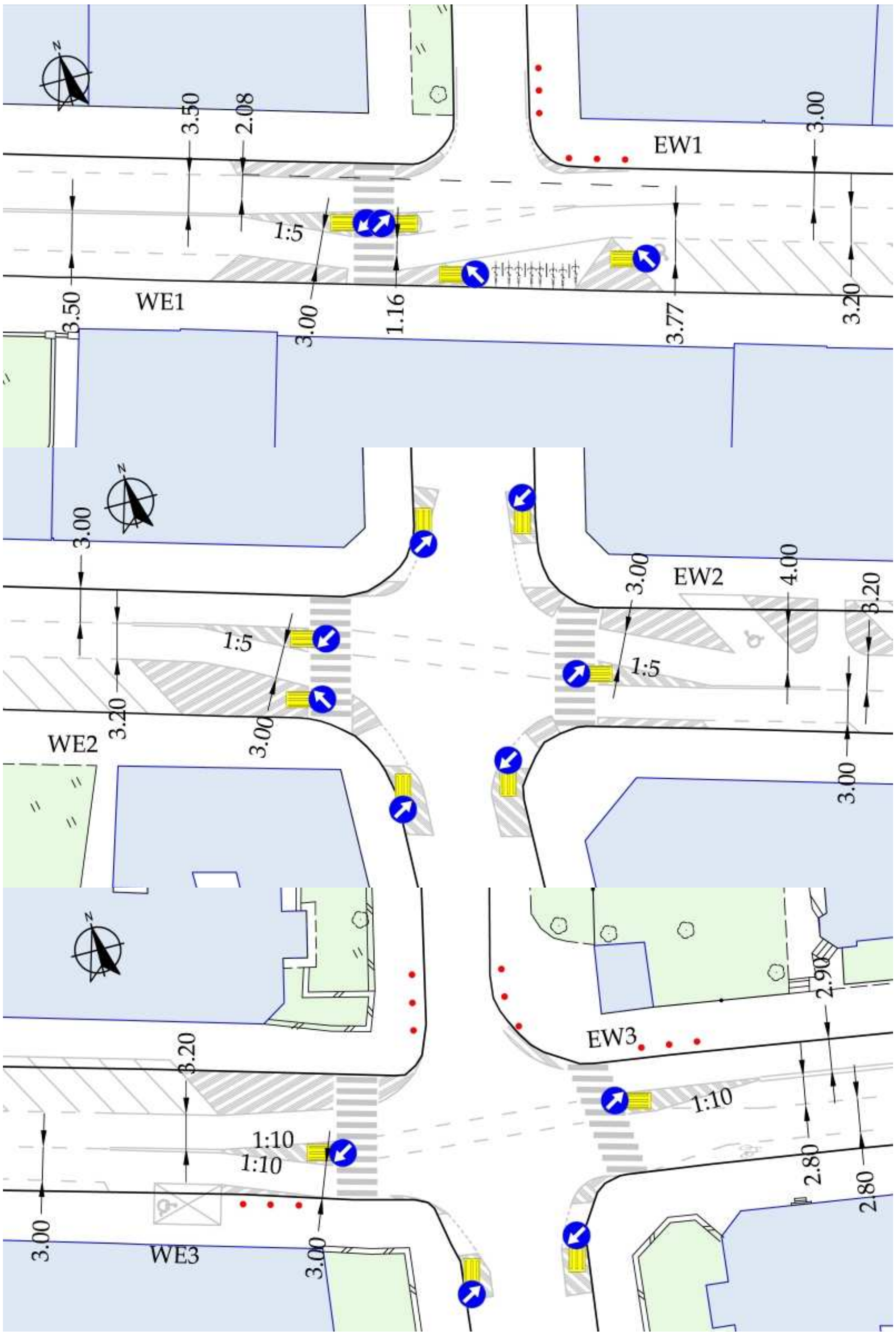

Figure A1. Geometrical features of the study sections: (**a**) WE1 and EW1; (**b**) WE2 and EW2; (**c**) WE3 and EW3. (all distances are in metres). Source: own work.

Appendix B

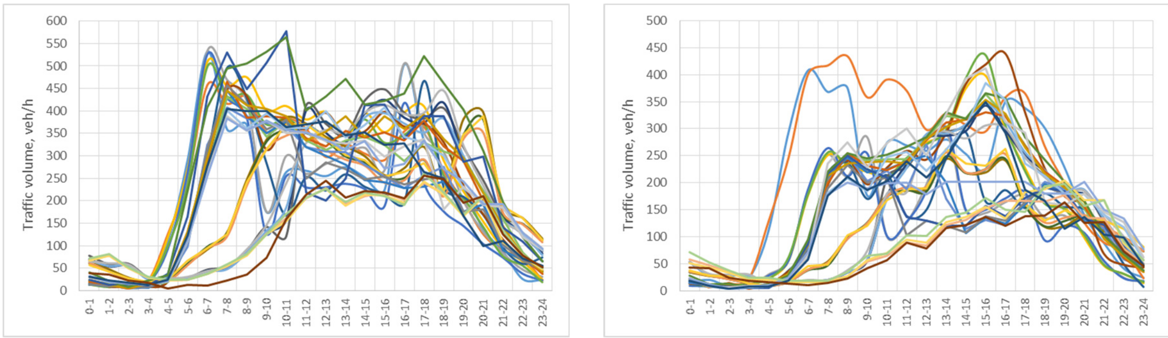

Figure A2. Hourly traffic volumes: (**a**) in the direction W→E; (**b**) in the direction E→W. Source: own work.

Appendix C

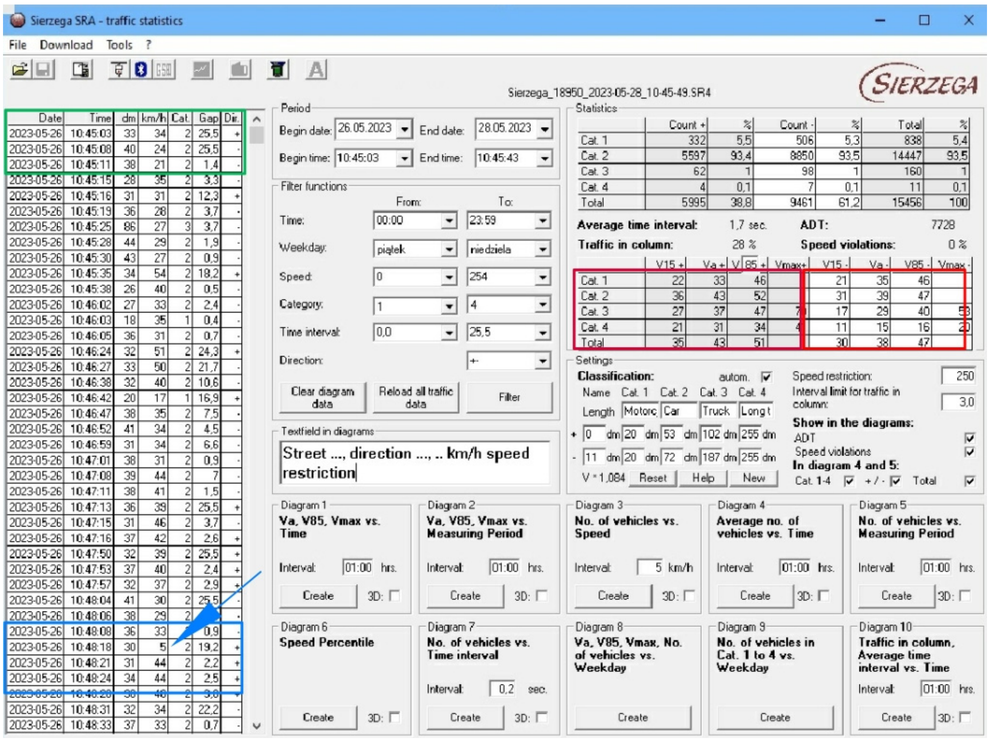

Figure A3. Example of SR4 traffic detector log (of the date: 2023-05-26). Source: own work.

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