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Article

Strength Characteristics and Wastage Allowance of Pipe Pile Subjected to Marine Corrosion

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Abstract: The study's objective is to analyse the mechanical properties of steel pipe piles as a part of a trestle bridge subjected to five years of natural marine corrosion degradation. Sixteen tensile specimens were extracted from the steel pipe piles in the splash, tidal, and immersion zones. The experimental tensile test results are used to establish regression equations defining elastic modulus, yield strength, strain hardening index, and strength coefficient for the true stress-strain curves of the three regions. A non-linear time-dependent mathematical model is exploited to identify the corrosion degradation using the data from one single corrosion degradation measurement campaign. The analysis indicated that the splash zone experiences the most severe corrosion degradation, and there are progressive losses in the mechanical properties of each zone as the corrosion degradation progresses. The established relationships of the mechanical properties, as a function of the ratio of corroded plate thickness to the as-built one, can be used as a fast-engineering approach to identify the mechanical properties of severely corroded piles. The corrosion degradation allowance is also defined using the First Order Reliability Method, accounting for existing uncertainties covered by the partial safety factors. By examining the impact of marine corrosion on the mechanical properties of marine structures and developing predictive models to assess the corrosion's effect on material strength and corrosion allowance, the study aims to improve offshore structures' safety, design, and maintenance.

Keywords: marine environments; corroded steel pipe pile; tensile test; tri-linear constitutive model; time-dependent corrosion degradation

1. Introduction

The integrity assessment of offshore structures during their service life considers the impact of corrosion degradation. In marine environments, steel pipe piles of temporary structures such as steel trestle bridges may undergo severe corrosion, leading to a pronounced deterioration in their mechanical properties [1]. Corroded steel pipe piles can diminish structural reliability and safety and may even lead to accidents [2].

Therefore, investigating the degradation of mechanical properties of corroded materials in different regions of marine structures is essential; for that, different approaches and corrosion models have been developed. Appuhamy et al. [3] employed the concept of effective thickness, conducting tensile tests on corroded steel plates to calculate the residual yield and tensile strength. Garbatov et al. [4] conducted monotonic tensile tests for corroded, cleaned and fatigue strength in [5, 6] on steel specimens cut from ship hull box girder corroded in seawater, revealing a non-linear decrease in yield strength due to localised corrosion and stress concentration.

Many studies investigated the degradation of material properties through artificial salt spray corrosion tests [7, 8]. Sheng & Xia [9] and Zhang et al. [10], among others, investigated the tensile performance of pitted steel plates using mechanical milling methods, discovering that pitting reduced yield strength and ductility.

Jia et al. [11] studied 180 specimens of Q345 steel subjected to artificial corrosion through salt spray tests. Tensile tests revealed that cross-sectional loss was the leading cause of decreasing the elastic modulus, yield stress, and ultimate strength. At the same time, pitting was the primary factor causing a reduction in ultimate elongation. Gathimba et al. [12] presented the surface characteristic evaluation results of corrosion-exposed steel piles under different exposure water levels in a marine environment over 19.5 years. Zhang et al. [13] conducted tensile tests on Q235 weathering steel specimens after accelerated corrosion tests, revealing that the stress-strain curves of the corroded specimens were lower than those of the non-corroded specimens. Yield strength, tensile strength, and the yield-to-tensile ratio decreased linearly with increasing corrosion time.

Some studies have explored the in-situ corrosion characteristics of steel materials. Guo et al. [14] conducted microscopic and tensile tests on Q235 steel specimens subjected to corrosion degradation in the atmosphere for 33 years. They found that corrosion pits significantly reduced the yield strength, ultimate strength, elastic modulus, and ductility, with elongation decreasing by 24.51%. Xia et al. [15] studied the morphology and remaining thickness of marine corroded steel pipe piles in the splash, tidal, and fully submerged zones. They observed that the steel pipe piles were subjected to double-sided corrosion degradation in the splash and tidal zones. At the same time, in the fully immersed zone, only the outer surface suffered more severe corrosion.

Models describing the variation of the corroded plate thickness over time for steel marine structures have been employed for structural integrity, reliability, and risk-based analysis of ageing structures. Some recent and advanced approaches in this field have been presented in [16-22].

The present study evaluates the mechanical properties of steel piles on a steel trestle bridge subjected to severe natural marine corrosion for five years. Sixteen specimens are collected from the splash, tidal, and fully submerged zones, with five specimens from each region. One intact specimen with a smooth polished surface is also analysed and used as a reference. The study establishes regression equations for the corresponding stress-strain curves for the three preselected zones of different corrosion environments. The long-term non-linear corrosion model is set, and based on it, the corrosion degradation allowance is defined using the First Order Reliability Method accounting for existing uncertainties.

2. Specimen Collection and Processing

2.1. Specimen Acquisition

The steel piles used in this experiment are sourced from a temporary structure, a steel trestle bridge, constructed for constructing the *Pingtian Cross-sea Bridge*, as shown in Figure 1. The steel trestle bridge is in the East China Sea, where the environmental conditions pose severe corrosion.

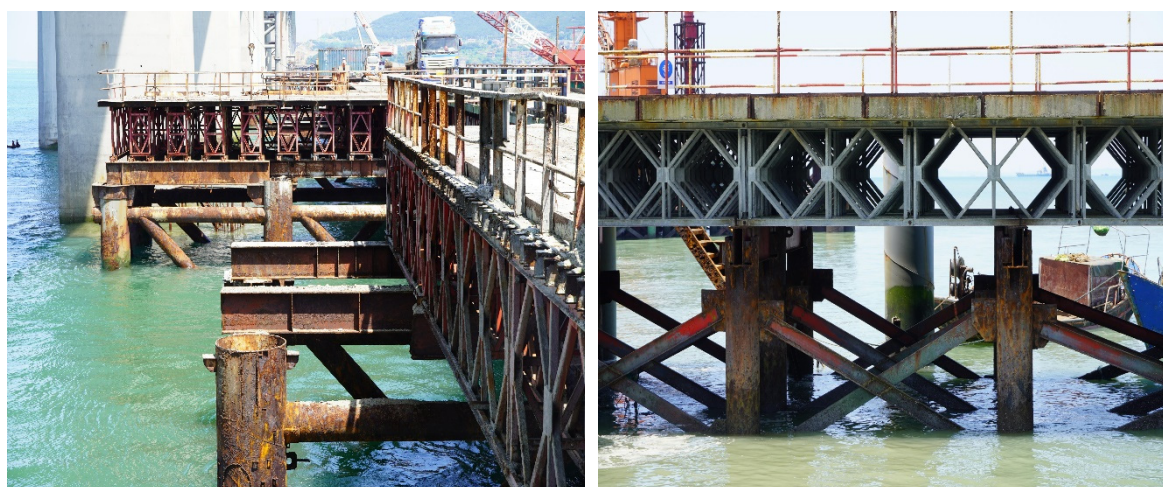


Figure 1. Steel frame of Pingtian Cross-sea Bridge.

The marine environmental characteristics in the region are presented in Table 1. The steel piles are Q235B steel and bear axial compressive loads from the upper structure and lateral loads from wind, water flow, and waves.

Table 1. Marine environment characteristics in the Pingtan area.

Characteristic	Value
Mean tidal range, m	4.3
Water temperature, °C	19.4
Maximum water velocity, m/s	2.23
Average water velocity, m/s	1.03
Dissolved oxygen saturation, %	95-100
PH	8.1-8.3
Salinity, %	3.0-3.2%
Average wind velocity, m/s	6.9
Tidal cycle, times/day	2
The most enormous wave height in history, m	16

The nominal outer diameter of the steel piles is 1,020 mm, with a nominal wall thickness of 12 mm and a total length of approximately 21 m, as shown in Figure 2. The splash, tidal, and fully submerged zones are differentiated based on the average high and low tide lines [23]. Steel plates with dimensions of 270 mm×270 mm are cut from each of the three zones, as shown in.



Figure 2. Steel pipe pile and specimen locations.

Following the specifications (GB/T 16545-2015) [24], the samples are cleaned using an acid-washing solution composed of hydrochloric acid, distilled water, and hexamethylenetetramine, as shown in Figure 3, where the hexamethylenetetramine is an inhibitor that prevents the dissolution of the base steel material. The steel composition is obtained in the testing laboratory, and specific values are presented in Table 2, meeting the requirements of the “Carbon Structural Steels” (GB/T 700-2006) [25].

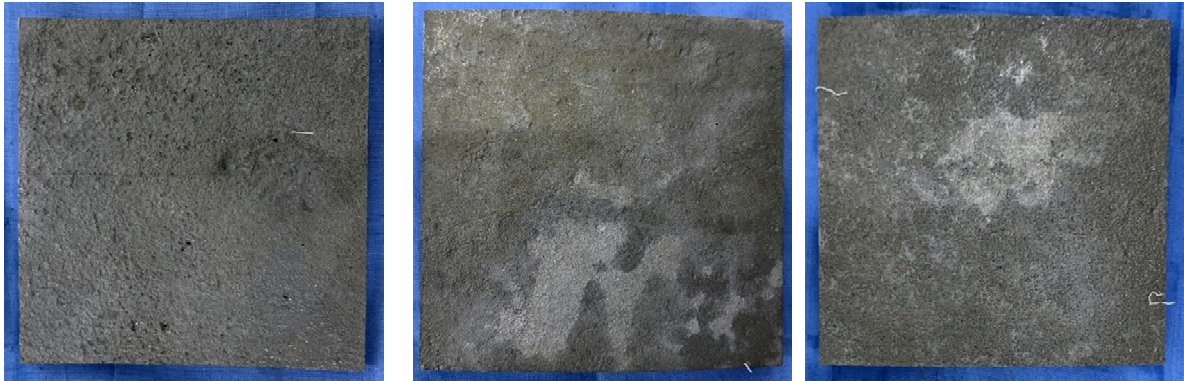


Figure 3. Corrosion product cleaning, splash zone (left), tidal zone (central) and immersion (right) zones.

Table 2. Elemental composition of Q235B steel.

Composition	C	Si	Mn	P	S
Content/%	0.176	0.198	0.5	0.023	0.019
GB/T 700-2006	≤0.20	≤0.35	≤1.40	≤0.045	≤0.045

2.2. Tensile Test Setup

2.2.1. Tensile Specimen Production

According to the Metal Tensile Test Standard (GB/T 228.1-2010) [26], samples are taken from the steel plates in each of the three zones. These specimens have the same thickness as the original steel piles, and the dimensions of the tensile specimens are shown in Figure 4. Control specimens undertake the same processing methods and sizes, with their upper and lower surfaces polished to a smooth finish to resemble surfaces that have never corroded.

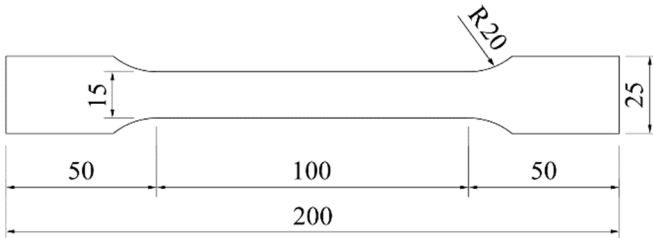


Figure 4. Test specimen configuration.

The specimen numbers are listed in Table 3, with each group consisting of 5 parallel corroded specimens, and P-1 represents the control specimen without corrosion. Six points are uniformly distributed from each specimen, measured, and the average value is taken as a thickness d_0 . The average thickness remaining ratio D represents the corrosion loss; the specific data can be found in Table 3. D is estimated as follows:

$$D = \frac{d_0}{d} \times 100, \% \tag{1}$$

where d represents the as-built thickness of 12 mm.

Table 3. Specimens characteristics.

Zone	Specimen	d_0 , mm	D , %	Mean, mm/ Var, mm ²
Splash	S-1	6.972	58.1	57.24/
	S-2	6.979	58.2	0.0143

	S-3	6.877	57.3	
	S-4	6.691	55.8	
	S-5	6.815	56.8	
	T-1	9.367	78.1	
	T-2	9.496	79.1	
Tidal	T-3	9.094	75.8	77.92/
	T-4	9.356	78.0	0.0233
	T-5	9.428	78.6	
	I-1	9.422	78.5	
	I-2	9.451	78.8	
Immersion	I-3	9.429	78.6	78.64/
	I-4	9.464	78.9	0.0004
	I-5	9.413	78.4	
Intact	P-1	4.003	-	-

Table 3 shows that the splash zone experiences severe corrosion, with an average corroded thickness of 6.87 mm and a variance of 0.0143 mm². In the immersion zone, the average thickness is 9.35 mm, and the variance is 0.0233 mm², while in the tidal zone, the mean value is 9.44, and the variance is 0.0004 mm². The control specimens have an average thickness of approximately 4mm. Ocean waves increase the contact between the steel pile surface and oxygen, leading to the splash zone's most severe corrosion degradation and less residual thickness. While the tidal zone is also affected by ocean waves, the duration of the wave impact is not as long as in the splash zone. The relatively lower oxygen content in the immersion zone creates an oxygen-concentration cell, benefiting from cathodic protection. Additionally, being unaffected by ocean waves, this area exhibits less corrosion, with the specimens having the most significant average thickness.

2.2.2. Test Procedure

Tensile tests were conducted on a Shimadzu AG-X Plus 250 kN universal testing machine. The force sensor and extensometer were employed to record the applied load and deflection within the mid-gauge length section. These measurements were used to calculate the engineering stress-strain curve of the steel. The parallel length of the tensile specimen was 100 mm, and the tensile speed at both ends was set at 6 mm/min, resulting in a nominal strain rate of 0.001 s⁻¹, complying with the specifications of the metallic tensile test standard [26]. The specimens after tensile fracture are shown in Figure 5.



Figure 5. Tensile test ruptured specimen.

3. Strength Assessment

3.1. Stress-Strain Analysis

Figure 6 shows that, after corrosion degradation took place, the stress-strain curves of the steel exhibit lower values than the non-corroded specimens. Specifically, the splash zone shows the lowest values, the tidal zone falls in the middle, and the submerged zone exhibits the highest values.

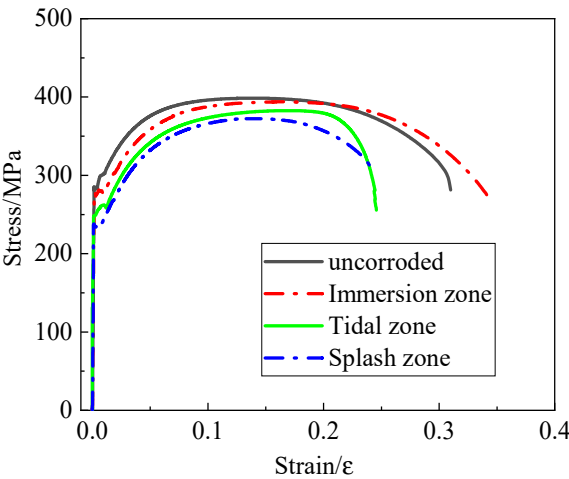


Figure 6. Engineering stress-train curves.

Figure 6 also indicates that corrosion degradation leads to a reduction in both yield strength and tensile strength. Additionally, specimens from the splash zone entered the necking stage earlier in the test, possibly due to numerous corrosion pits on the specimen surface. More bottomless corrosion pits can induce more severe stress concentration, resulting in an earlier initiation of yielding and, consequently, a reduction in yield strength.

3.2. Mechanical Property Analysis

The more detailed parameters of tensile performance are shown in Table 4. After corrosion, the specimens did not exhibit a distinct yield plateau, and the yield strength was typically taken at the lower yield point. As indicated in Table 4, the data distribution of yield strength (f_y) and ultimate tensile strength (f_u) within the same region is relatively concentrated, while the post-fracture elongation (δ) and elastic modulus (E) show higher variability within the same region. The tensile characteristic values in each group are smaller than those in the non-corroded specimen, indicating a severe degradation in the mechanical performance in all sea areas.

Table 4. Parameters of tensile test.

No.	f_y , MPa	Mean, MPa/ Var, MPa ²	f_u , MPa	Mean, MPa/ Var, MPa ²	Δ , %	Mean, %/Var, -	E 10 ⁵ , MPa	Mean, 10 ⁵ MPa/ Var, MPa ²
S-1	248.81		382.01		20.38		1.67	
S-2	236.51		373.06		22.65		1.68	
S-3	233.90	237.40/ 42.005	372.46	371.51/ 52.161	24.10	22.68/ 9.853	1.70	1.67/0.001
S-4	233.95		362.70		19.16		1.63	
S-5	233.81		367.33		27.12		1.65	
T-1	254.43		384.82		25.40		1.71	
T-2	279.48		402.60		35.00		1.93	
T-3	258.37	263.00/ 104.467	382.68	390.16/ 59.854	24.58	29.35/ 23.300	1.71	1.78/0.010
T-4	266.15		390.60		33.90		1.83	
T-5	256.56		390.12		27.86		1.70	

I-1	268.62		392.41		28.5		1.73	
I-2	282.41		398.54		30.99		1.94	
I-3	296.93	285.12/ 229.482	392.89	392.33/ 27.420	24.53	29.68/ 3.367	1.76	1.80/0.009
I-4	273.46		393.77		34.56		1.85	
I-5	304.17		384.04		29.83		1.72	
P-1	310.13		407.18		33.3		2.08	

To better reveal the extent of the reduction in mechanical performance of steel piles subjected to marine corrosion in each region, the corrosion loss rates of f_y , f_u , δ , and E for each specimen are presented in Table 5.

Table 5. Loss rate of corrosion mechanical properties.

No.	f_y loss rate, %	Mean, %/Var, -	f_u loss rate, %	Mean, %/ Var, -	δ loss rate, %	Mean %/ Var, -	E loss rate, %	Mean %/ Var, -
S-1	19.77		6.18		38.80		19.71	
S-2	23.74		8.38		31.98		19.23	
S-3	24.58	23.45/ 4.370	8.53	8.76/ 3.145	27.63	31.89/ 88.829	18.27	19.90/ 1.682
S-4	24.56		10.92		42.46		21.63	
S-5	24.61		9.79		18.56		20.67	
T-1	17.96		5.49		23.72		17.79	
T-2	9.88		1.12		-5.11		7.21	
T-3	16.69	15.20/ 10.866	6.02	4.18/ 3.619	26.19	11.87/ 210.164	17.89	14.64/ 23.933
T-4	14.18		4.07		-1.80		12.02	
T-5	17.27		4.19		16.34		18.27	
I-1	13.38		3.63		14.41		16.83	
I-2	8.94		2.12		6.94		6.73	
I-3	4.26	8.06/ 23.838	3.51	3.65/ 1.653	26.34	10.87/ 120.531	15.38	13.46/20.230
I-4	11.82		3.29		-3.78		11.06	
I-5	1.92		5.68		10.42		17.31	

The mechanical performance of P-1 is considered the original mechanical properties of the steel pile material, and the corrosion loss rates of each specimen's material properties are calculated accordingly. As shown in Table 5, the corrosion loss rate in the immersion zone is slightly smaller than that in the tidal zone, while the corrosion loss rate in the splash zone is much greater than that in the immersion and tidal zones. The varying corrosion loss rates in different regions indicate that corrosion in other areas under marine conditions not only results in other cross-sectional losses but also induces varying degrees of stress concentration, leading to various changes in the mechanical properties of the steel material [27].

The average corrosion loss rates of the five groups of mechanical properties in each region from Table 5 are calculated and presented in Table 6, where the average corrosion loss rates in the splash zone are the highest, with f_y , f_u , δ , and E experiencing average losses of 23.45%, 8.76%, 31.89%, and 19.90%, respectively. The tidal zone follows, with average loss rates of 15.02%, 4.18%, 11.87%, and 14.63% for f_y , f_u , δ , and E , respectively. The immersion zone exhibits a milder degree of corrosion, with average loss rates of 8.06%, 3.65%, 10.87%, and 13.46% for f_y , f_u , δ , and E , respectively. The splash zone of the steel piles is subject to long-term wave, current, and wind loads, along with an increased oxygen supply to this area [15], resulting in severe corrosion and high mechanical performance loss rates.

Table 6. Mechanical property.

Zone	Average loss rate of f_y	Average loss rate of f_u	Average loss rate of δ	Average loss rate of E
S-zone	23.45%	8.76%	31.89%	19.90%
T-zone	15.20%	4.18%	11.87%	14.63%
I-zone	8.06%	3.65%	10.87%	13.46%

The correlation between f_y , f_u , and E , and the average thickness residual rate D was calculated separately, and the specific values are presented in Table 7, where most correlation coefficients between f_y , f_u , E , and D are above 0.5.

Table 7. Correlation between average thickness residual rate and mechanical properties.

	$D - f_y$	$D - f_u$	$D - E$
S-zone	0.514	0.722	0.851
T-zone	0.474	0.770	0.578
I-zone	0.592	0.515	0.794

4. Stress-Strain Model

4.1. True Stress-Strain Model

The nominal stress-strain relationship is used widely in engineering. However, when displacement is large, the stress and strain are no longer distributed evenly in the test specimen. Thus, the true constitutive relation in the plastic stage cannot be built [28]. The true stress-strain curve is employed to assess the mechanical properties of materials. The true stress and strain are calculated from the engineering stress and strain data using the Eqn (2) to (3) as described in [28], where the true stress (σ_z) is defined as:

$$\sigma_z = \frac{F}{S_0} \frac{L_e + \Delta L}{L_e} = \frac{F}{S_0} (1 + \varepsilon) \quad (2)$$

where F is the measured load, S_0 is the average cross-sectional area, L_e is the estimated elongation length, ΔL is the elongation displacement. The true strain (ε_z) is estimated as follows:

$$\varepsilon_z = \ln\left(\frac{L_e + \Delta L}{L_e}\right) = \ln(1 + \varepsilon) \quad (3)$$

where ε is the measured strain.

The engineering stress-strain curve before necking is converted into the true stress-strain curve, as shown in Figure 7. Comparing the range of the elastic stage, it can be observed that the immersion zone has the most extended range. In contrast, the splash zone has the shortest, indicating that the steel in the splash zone enters the yielding earliest after corrosion by seawater. Comparing the tensile strength at a strain of 0.15, it is found that the tensile strength is highest in the immersion zone and lowest in the splash zone, indicating that the influence of corrosion degradation on the tensile strength in the splash zone is much more significant than in the tidal zone and immersion zone.

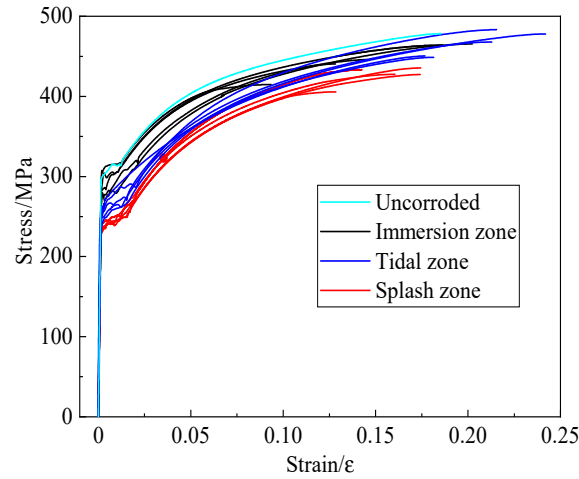


Figure 7. True stress-strain curves of the specimens.

The elastic modulus and yield strength for each zone are shown in Figure 8. The elastic modulus significantly varies among different areas, with a relatively small change in the tidal zone and a more scattered distribution in the splash and immersion zones. Due to the limited experimental data, linear regression fits mechanical performance parameters and the average thickness residual rate. The regression equations for the elastic modulus in each region are as follows:

$$E_S = (72.0D + 135.9) \times 10^5, \text{ GPa} \quad (4)$$

$$E_T = (53.2D + 154.8) \times 10^5, \text{ GPa} \quad (5)$$

$$E_I = (40.4D + 167.6) \times 10^5, \text{ GPa} \quad (6)$$

The yield strength for various regions in the true stress-strain curve is depicted in Eqn (7) to (9). The regression curves fitted to the results are as follows for each region:

$$f_{yS} = 125.7D + 184.3, \text{ MPa} \quad (7)$$

$$f_{yT} = 59.4D + 250.2, \text{ MPa} \quad (8)$$

$$f_{yI} = 45.7D + 264.3, \text{ MPa} \quad (9)$$

When the specimen reaches the yield strength, low-alloy steel generally enters the plastic stage, followed by the strengthening stage. The strengthening stage can be regarded as uniform plastic deformation. According to Gathimaba et al. [28], the strengthening stage of the true stress-strain curve can be represented by a simplified power-law equation:

$$\sigma_h = \left[\sigma_r \left(\frac{E}{\alpha \sigma_r} \right)^n \right] (\varepsilon_z - \varepsilon_q)^n = K \varepsilon_h^n \quad (10)$$

where σ_h is the stress in the strengthening section, σ_r is the stress at the reference point, α is the dimensionless constant, n is the strain hardening index, K is the strength factor,

$$K = \sigma_r \left(\frac{E}{\alpha \sigma_r} \right)^n \quad (11)$$

where ε_q is the yield section strain, and ε_h is the strengthening section strain. K and n regression curves in each region are defined as:

$$K_S = 6549.6D - 4505.7 \quad (12)$$

$$K_T = 842.4D - 24.7 \quad (13)$$

$$K_I = 5354.4D - 3616.4 \quad (14)$$

$$n_S = -0.162D + 0.566 \quad (15)$$

$$n_T = 0.312D - 0.050 \quad (16)$$

$$n_I = 5.172D - 3.915 \quad (17)$$

The coefficient of determination R^2 for S-zone, T-zone and I-zone in Eqn (12) to (17) is shown in Table 8, where it can be seen that R^2 for all regression equation is relatively lower and the I-zone represent better firing in the three of the four analysed parameters.

Table 8. Coefficient of determination R^2 .

	Eqn	R^2	Eqn	R^2	Eqn	R^2	Eqn	R^2
S-zone	(4)	0.539	(7)	0.350	(12)	0.164	(15)	0.181
T-zone	(5)	0.189	(8)	0.225	(13)	0.154	(16)	0.195
I-zone	(6)	0.724	(9)	0.197	(14)	0.286	(17)	0.391

Figure 8 compares the data presented in [29, 5, 16] with the regression curves developed in the present study. Wołoszyk et al. [16] and Garbatov et al. [5] employed methods related to immersion in seawater and seawater flushing, respectively, to analyse the effects of seawater corrosion on the mechanical properties of Q235 steel.

Due to the incomplete correlation data for K and n in the literature, a comparison was conducted by calculating elastic modulus loss rates and yield strength with the three developed relationships here. It can be observed that the data from [29] aligns well with the splash zone. However, the data from [5, 16] are mainly below the relationships for the three zones, indicating that, at the same corrosion loss rate, [5, 16] exhibit a higher loss in elastic modulus. The data from [29, 16] are spread on both sides of the regression curves of the present study related to the splash zone, while the data from [16] are closer to the curves of either the tidal zone or the immersion zone in this study. The laboratory's accelerated corrosion data also show a higher linear relationship and less scatter.

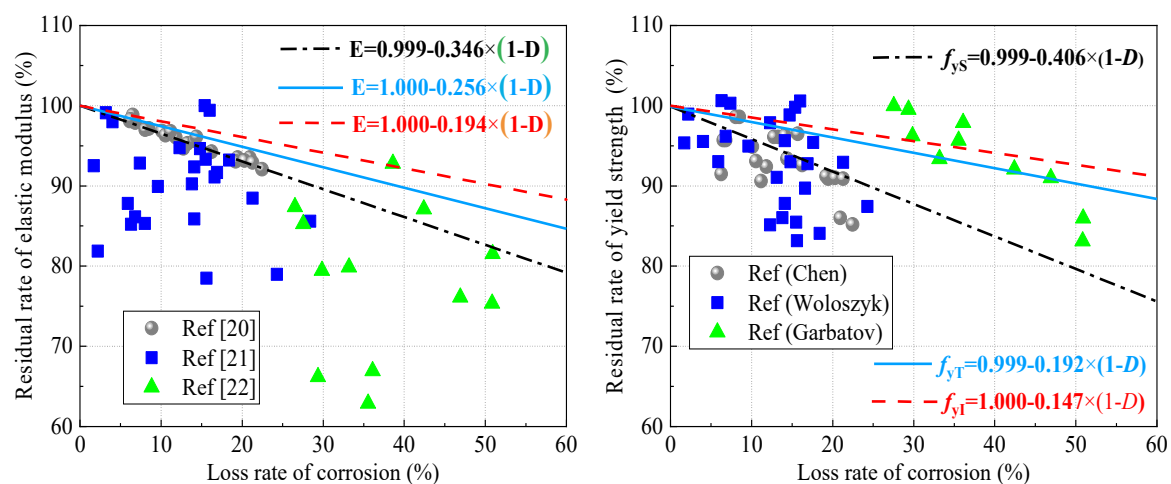


Figure 8. Elastic modulus (left) and yield strength (right).

The degradation level of the elastic modulus in steel subjected to accelerated corrosion aligns well with the fitting curve for the splash zone in this study. For specimens treated with seawater flushing, the degradation level of yield strength corresponds well with the fitted curves for the tidal and immersion zones in the present study.

4.2. Tri-Linear Constitutive Model

The following four points describe the tri-linear constitutive model as was initially proposed in [6]:

$$\sigma_0 = \varepsilon_0 = 0 \quad (18)$$

$$\sigma_{y1} = f_y, \varepsilon_{y1} = \frac{\sigma_{y1}}{E} \quad (19)$$

$$\sigma_{y2} = f_y, \varepsilon_{y2} = \left(\frac{\sigma_{y2}}{K}\right)^{1/n} \quad (20)$$

$$\sigma_p = K(\varepsilon_p - \varepsilon_{y2})^n, \varepsilon_p > \varepsilon_{y2} \quad (21)$$

where σ_0 is the stress at the starting point, ε_0 is the strain at the starting point, σ_{y1} is the stress at the beginning of the yielding stage, ε_{y1} is the strain at the beginning of the yielding stage, σ_{y2} is the stress at the beginning of the reinforcement stage, ε_{y2} is the strain at the beginning of the reinforcement stage, σ_p is the maximum stress at the reinforcement section, ε_p is the strain at the maximum stress at the reinforcement section. A bilinear material behaviour may be described, as seen in Figure 13.

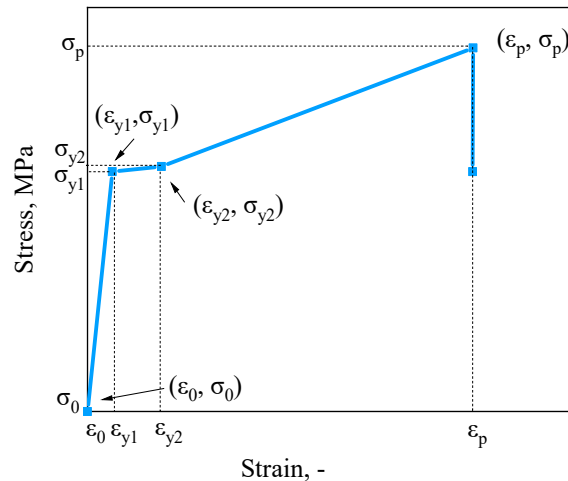


Figure 9. Bilinear elasto-plastic stress-strain relationship.

The average thickness remaining ratio range for specimens in each zone is substituted into Eq. (17) to obtain the fitting regions for the tri-linear constitutive model. A comparison between the areas fitted and the experimental stress-strain curves is shown in Figure 14. For the splash zone, the duration of the elastic stage in the observed data aligns closely with the fitting region, the yield stage is slightly above the fitting region, with a maximum difference of 0.9%, and the strengthening stage matches the fitting region at both ends but shows a slight elevation in the middle part of the experimental data. The overall situation in the tidal zone is like the splash zone, with a maximum difference of 3.3% in the duration of the elastic stage. The immersion zone closely resembles the first two zones in the elastic stage, with a maximum difference of 3.8% in the duration of the elastic stage. The strengthening stage for most of the experimental data is slightly above the fitting region but with minimal differences. Comparing the experimental stress-strain curves and the formula-fitted areas for the three zones, it is observed that the plastic and strengthening stages in the middle part are somewhat conservative. In contrast, the other regions are almost identical, with a maximum difference of 5%.

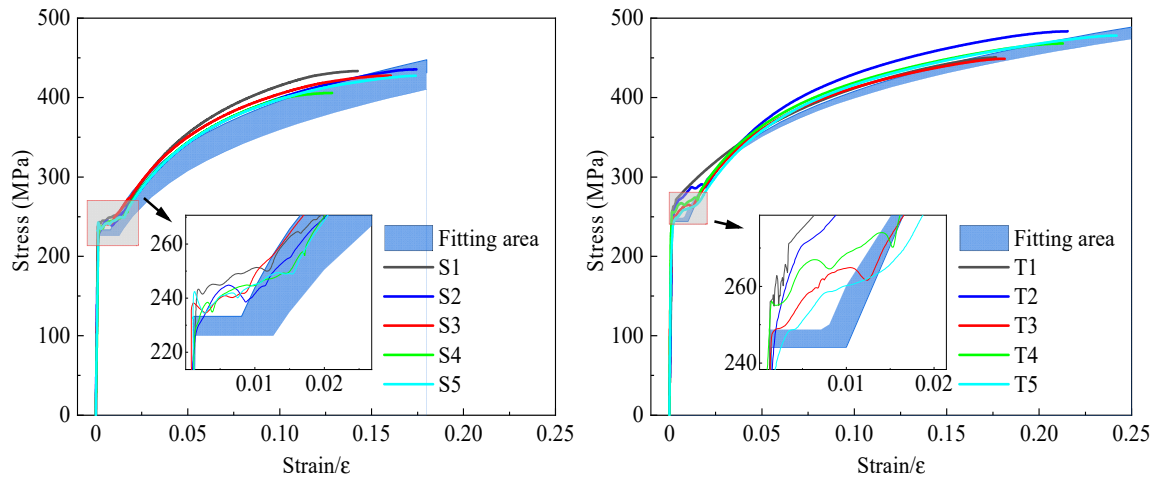


Figure 10. True stress-strain curves, splash (left) and tidal (right) zones.

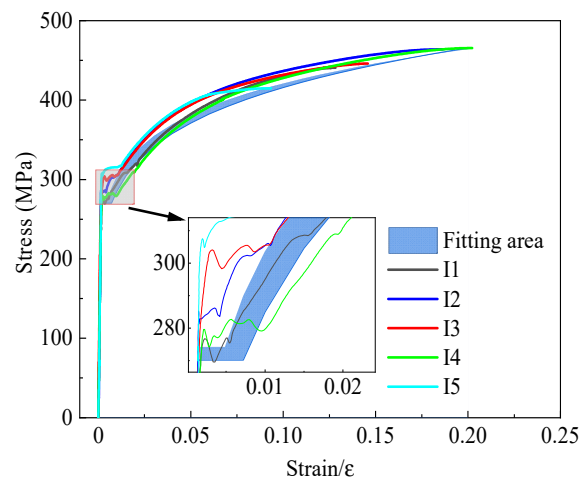


Figure 11. True stress-strain curves, immersion zone.

5. Corrosion degradation allowance

Corrosion degradation is a time-dependent, non-linear process, and its development involves different stages, resulting in various forms of degradation. Corrosion degradation may be global uniform, local pitting, crevice, galvanic, and fatigue corrosion [30, 31]. The corrosion rate and depth can be predicted by employing corrosion degradation models, which can be used for the structural reliability of steel structures during the service life. The currently used corrosion models may be classified as linear, exponential, power-law, and Weibull models corrosion [32-40].

Guedes Soares & Garbatov [41] developed a non-linear model for describing corrosion growth, where the entire corrosion degradation process is divided into three stages, as shown in Figure 15. In the first stage, $t \in [0, A]$, the structure does not experience corrosion degradation due to the corrosion protection system. The second stage, $t \in [A, B]$, begins after the corrosion protection system fails. The third stage, $t \in [B, \infty]$, occurs as corrosion products adhere to the structure's surface, preventing further corrosion development. The corrosion process gradually stops, and the corrosion rate approaches zero. However, corrosion degradation will restart if the corrosion layer detaches from the metal's surface due to scouring or other human activities.

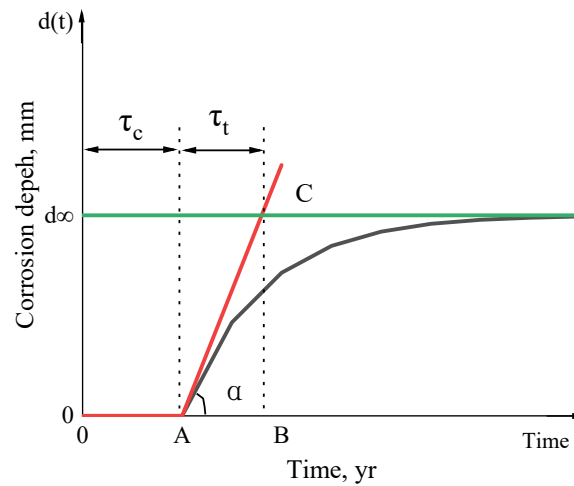


Figure 12. Corrosion depth as a function of time [41].

Corrosion measurement activities were conducted around the 5th year of the service life, and the steel pipe piles had already undergone severe corrosion degradation, resulting in a thickness reduction. It is assumed that once corrosion forms a relatively deep corrosion layer, preventing oxygen from reaching the fresh material, the corrosion degradation rate will decrease or even stop until the corroded layer is penetrated or cleaned. Given the comprehensive corrosion protection design and corrosion prevention measures, the coating life is assumed $\tau_c=1$ year. The transition time, τ_t in the corrosion thickness model [4, 41] is defined based on the experimental result, the long-term corrosion thickness achieved by $t_\infty=5$ years is assumed as $d_\infty=6.00$, mm. The mean value corrosion depth based on the two models used here are (see Figure 16) presented as:

Exponential (non-linear) model [41, 43]:

$$E[d(t)]_{nl} = \begin{cases} 0, t < \tau_c \\ d_\infty \left[1 - e^{-\frac{t-\tau_c}{\tau_t}} \right], \tau_c \leq t \end{cases} \quad (22)$$

Linear model:

$$E[d(t)]_l = \begin{cases} 0, t < \tau_c \\ \frac{d_\infty}{t_n - \tau_c} (t - \tau_c), \tau_c \leq t \end{cases} \quad (23)$$

where $t_n = 5$ years is the time of the corrosion depth measurement campaign. The exponential model requires regular measurement campaigns during the service life. The second model is based on an individual measurement campaign, which can be done at any time once the corrosion propagates. The two corrosion degradation models analysing the analysed zones are shown in Figure 13 and Figure 14. The transition time is estimated based on the corrosion depth measurements accounting for the coating life and long-term corrosion depth. Table 9 shows the transition times for different zones.

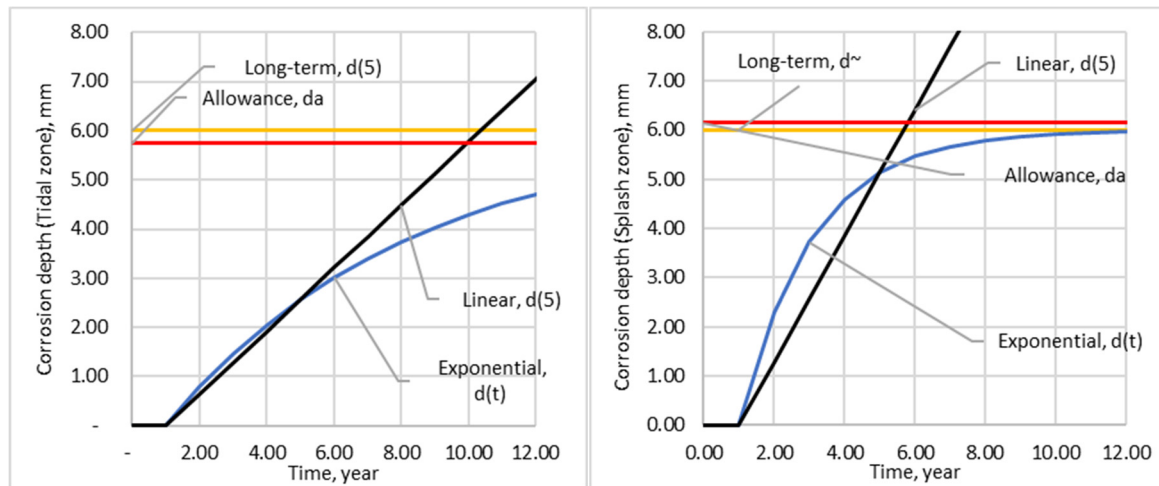


Figure 13. Corrosion degradation in tidal (left) and splash zones.

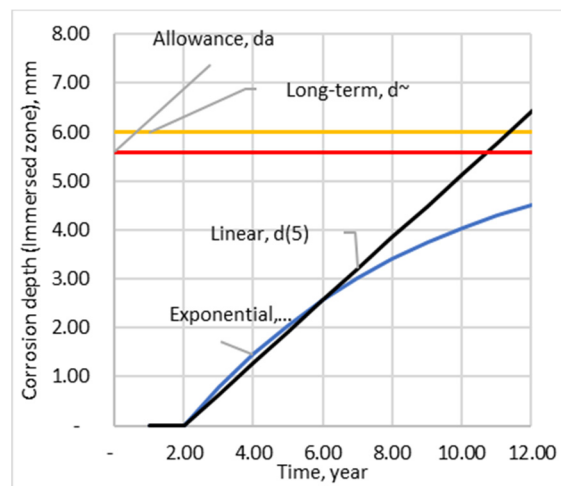


Figure 14. Corrosion degradation in immersed zone.

Table 9. Transition time.

	$d(5),$ mm	$\tau_t, \text{ years}$
S-zone	5.13	2.07
T-zone	2.65	7.17
I-zone	2.56	7.17

It can be seen from Table 9 that the transition time of the splash zone is much shorter, and the corrosion degradation is much more profound. It has already been proven [39] that the logarithms relationship can be used to describe the standard deviation as a function of the time as:

$$StDev[d(t)]_{nl} = \begin{cases} 0, & t < \tau_c \\ a \ln(t - \tau_c), & \tau_c \leq t \end{cases} \quad (24)$$

where a is 0.12, 0.15 and 0.02 for S, T and I-zones respectively.

The corrosion wastage allowances can be defined using the beta structural reliability index at the end of the service life using FORM [42, 43]. When estimating corrosion wastage allowances, all significant inherent (aleatory) variability is accounted for in the long-term analysis in predicting the corrosion depth progress during the service life. Epistemic (lack of knowledge) type of uncertainties will, in most cases, be assumed to be covered by the partial safety factor.

The limit state function, for estimating the corrosion wastage allowances at the end of the service life of 20 years, is defined as:

$$Z = d_a - d(20) \quad (25)$$

where d_a is the wastage allowance, $d(20)$ is the corrosion depth estimated at the 20th year and the design equation is given as:

$$(1/\gamma_{d_a})\mu_{d_a} \geq \gamma_{d(20)}\mu_{d(20)} \quad (26)$$

The beta reliability index at the end of the service life is defined by FORM as $\beta = 2$. Using statistical descriptors of d_a and the prediction of the corrosion degradation $d(20)$ at the 20th year of the service life, Eqns (22) and (24), the design points of d_a^* and $d_{d(20)}^*$ in the normal design space of FORM can be defined, and once the convergence is achieved, the partial factors are calculated as:

$$1/\gamma_{d_a} = \frac{d_{d_a}^*}{\mu_{d_a}} \quad (27)$$

$$\gamma_{d(20)} = \frac{d_{d(20)}^*}{\mu_{d(20)}} \quad (28)$$

Finally, the mean value of the corrosion wastage allowances accounting for the beta reliability index at the end of the service life is defined as:

$$\mu_{d_a} = \gamma_{d_a}\gamma_{d(20)}\mu_{d(20)} \quad (29)$$

Table 10 shows the partial factors and corrosion wastage allowances.

Table 10. Corrosion wastage allowances.

	μ_{d_a}, mm	γ_{d_a}	$\gamma_{d(20)}$
S-zone	6.1487	1.0165	1.0082
T-zone	5.7386	1.0139	1.0151
I-zone	5.7207	1.0157	1.0102

The corrosion wastage allowances should be added to the pile net thickness in the different corrosion environment zones during the design process to guarantee that the structural behaviour of the corroded pile at the year 20 will be with the predicted reliability level. The wastage allowances are based on the corrosion degradation measurements conditional on the environmental conditions and beta structural reliability [44].

6. Conclusions

This study investigated the mechanical properties of steel pipe piles subjected to severe marine corrosion environments explored during the last five years. The study analyses the loss rates of steel material mechanical properties in the splash, tidal, and fully submerged zones, comparing them with specimens subjected to alternative corrosion methods to assess the degree of mechanical degradation. Experimental results reveal that the steel piles in the splash zone undergo the most severe corrosion, with average loss rates of 23.5% for yield strength, 8.8% for ultimate tensile strength, 31.9% for elongation at break, and 19.9% for elastic modulus. Comparing the degradation of elastic modulus and yield strength for different steel types in various marine corrosion environments, it was observed that the differences in mechanical performance degradation among other steels are minimal. Elastic modulus degradation is more consistent across different environments and is particularly pronounced in the splash zone. The developed regression equations are suitable for predicting the mechanical performance of steel pile materials in areas with more severe corrosion. The probabilistic design solutions developed for the corrosion thickness allowance for different corrosion environments referred to a specific service period and reliability level. The developed approaches are

flexible in estimating the mechanical properties and corrosion allowance, where any further update in collecting more data may result in a different trend in the present study's estimated mechanical properties and corrosion allowances.

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