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Article

Compressive Strength of Geopolymer Mortar Produced from Fire Brick and Ceramic Waste

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Abstract: The production of Ordinary Portland Cement (OPC) generates about 7% of global CO₂ emissions. Geopolymer concrete could be a successful alternative to the ordinary Portland concrete. Construction and Demolishing Wastes (CDWs) have the potential of containing of alkaline activation binders due to their rich nature of aluminosilicate. This study investigates the possibility of producing geopolymer concrete using Fire brick (FB) and Ceramic Waste (CW) CDWs based materials as binders. This was achieved through evaluating the compressive strength of the produced geopolymer mortar. The main tested parameters were; the type of binders (FB and CW), the activator molarity (10, 12, and 15) M, the activator (Sodium Silicate to Sodium Hydroxide) ratio (1, 1.5, 2, 1 and 2.5), curing conditions (ambient and heating) and curing period (age) (7, 14 and 28) days. It was mainly found that both binders could be a successful potential to produce geopolymer concrete. The FB-based mortar showed higher compressive strength than the counterpart CW-based mortar for all considered parameters. Overall, it seems that the chemical composition of the binder was the main factor affecting the compressive strength of the resulted geopolymer.

Keywords: geopolymer; construction and demolition waste; firebrick; ceramic tiles; compressive strength

1. Introduction

The production of Ordinary Portland Cement (OPC) is independently responsible for about 7% of the total CO₂ emissions worldwide [1]. This encouraged the researchers to looking for alternative solutions to reduce the use of OPC in concrete, consequently eliminating the CO₂ emissions [2]. Generation of Construction and Demolition Waste (CDW) has become prominent worldwide. Being one of the largest sectors contributing to global solid waste production. Twenty-eight member states of the European Union generated a total 830 million Ton (Mt) of CDW in 2012, which accounts for approximately 1.65 Mt per year [3]. In 2015, United States generated a total of 548 Mt of CDW, which corresponds to nearly 1.7 Tons of CDW per capita [4]. Examples show that generation of CDW is a global issue. Unless controlled properly, large portions of CDW continue flow to the clean landfills and threaten the health of individuals and environment. CDWs have the potential of containing alkaline activation due to their aluminosilicate-rich nature. However, studies on the performance of CDWs in geopolymer synthesis are quite limited. It was shown that individual fractions of CDW-based materials such as tiles [5], bricks [6], glass [7], concrete [8], unbound aggregates [9], fire clay brick [10] can successfully be used in geo-polymerization. Geopolymers (alkali-activated materials) are green options that commonly produced by activating aluminosilicate wastes (*i.e.*, precursors) by alkali hydroxides and silicates solutions [11]. Common industrial by-products (*e.g.*, different classes of Fly Ash (FA) and Ground Granulated Blast Furnace Slag (GGBFS) are widely used in producing of geopolymer matrix [12]. Research on geopolymer has been reported for different sources of materials depending on their local availability and the production cost. Studies on the mechanical properties; in specific, compressive strength of geopolymer mortar or concrete made from fly ash or GGBS partially or fully replacement were reported in previous works. The main examined parameters were: activator molarity, activator ratios, curing methods, the temperature of heat curing, and the alkaline solution to binder ratio [13-24]. These studies concluded that the fly ash and GGBS can be successfully used in production of geopolymer matrix. However, no significant work has been reported on the use of other wastes such as; Ceramic Ball (CB), Fire Brick (FB), Ceramic Waste (CW), Glassy Sand (GS), Cement Kiln Dust (CKD), Grinded Aggregate (GA) and Silica Rock (SR). The current study

investigated for the first time the potential of use these materials as binders for producing geopolymer matrix. Firstly, XRF test was conducted to choose the binders that could possibly used for producing geopolymer matrix. Secondly, for the chosen binders, the following parameters were examined: the curing conditions (ambient and heat curing 70 and 100 C°), the molarity of alkaline activator (10, 12, 15) M and the alkaline activator ratios (AR) sodium silicate (Na₂SiO₃) to sodium hydroxide (NaOH) (1:1, 1.5:1, 2:1, and 2.5:1). Furthermore, fly ash replacement for the chosen binders with 15% or 30% for specific specimens were also investigated. Finally, the performance of the produced geopolymer was evaluated in terms of the compressive strength at ages of 7, 14 and 28 days.

2. Materials and Methods

2.1. Binding Materials

Assorted resources of binding materials were adopted in this study according to their availability (naturally or waste-form found), namely; Ceramic Ball (CB), Fire Brick (FB), Ceramic Waste (CW), Glassy Sand (GS), Cement Kiln Dust (CKD), Grinded Aggregate (GA) and Silica Rock (SR) (See Figure 1). XRF test was conducted to determine their chemical composition and ensure they met the requirement needed for producing geopolymer matrix [25], Table 1, presents the results of the XRF tests.



Figure 1. Different resources of binding materials.

Table 1. Chemical composition of assorted binding materials based of XRF test.

| Type of Binding Material | Chemical Composition % | | | | | | | | | |
|--------------------------|------------------------|--------------------------------|-------|--------------------------------|-------|-------------------|------------------|-------|------------------|-------------------------------|
| | SiO ₂ | Al ₂ O ₃ | CaO | Fe ₂ O ₃ | MgO | Na ₂ O | K ₂ O | MnO | TiO ₂ | P ₂ O ₅ |
| Ceramic Ball | 68.48 | 22.19 | 1.04 | 0.03 | 0.28 | 1.2 | 0.95 | 0.24 | 0.02 | 0.27 |
| Fire Brick* | 54.11 | 22.4 | 4.432 | 2.94 | 0.606 | 0.158 | 0.953 | 0.03 | 1 | 0.071 |
| Aggregates | 71.4 | 2.54 | 11.2 | 0.37 | 1.6 | 12.25 | 0.36 | 0.10 | 0.16 | 0.2 |
| Glassy sand | 92.16 | 6.69 | 1.16 | 0.20 | 0.61 | 0.02 | 0.1 | 0.02 | 0.20 | 0.12 |
| Silica Rocks | 97.01 | 0.306 | 0.052 | 0.775 | 0.063 | 0.041 | 0.057 | 0.02 | 0.32 | 0.16 |
| Ceramic Waste* | 49.5 | 20.303 | 2.43 | 6.62 | 4.43 | 1.9 | 1.596 | 0.043 | 0.678 | 0.074 |
| Cement Kiln Dust | 15.6 | 3.72 | 45.36 | 2.6 | 1.9 | 3.16 | 2.74 | 0.45 | 0.25 | 0.17 |

* Selected binders.

2.2. Selected Binders

Fire brick and ceramic wastes were adopted in this study due to their chemical compositions, local availability and its low cost. The firebrick waste was collected from damaged lining of oil furnaces in petroleum refineries, whereas, the ceramic waste was obtained from the locally available deconstruction wastes (Figure 2). The two CDW-based materials were initially crushed and then milled for final grinding. Table 2, lists the chemical compositions of FA, FB, and CW CDW-based precursors as determined by X-ray fluorescence (XRF) analysis following the provisions of ASTM E1621-13 [26]. The results showed that FB and CW are rich in siliceous and aluminous oxides which

are vital chemical compounds for the formation of calcium silicate aluminum hydrate (C-A-S-H) gel in geopolymer materials. These minerals (i.e., siliceous and aluminous oxides) enhanced the mechanical properties of the final products [27]. Table 2 also provides, the Loss on Ignition (L.O.I) (which is an indicator of the quality of the binder) obtained by measuring the content of the organic matter in the binder. It was determined in accordance with the guidelines of ASTM C114-18 [28].



Figure 2. CDW-based materials: (a): Fire brick waste, (b): Ceramic waste.

Table 2. Chemical composition of the selected precursors.

| Chemical Compound | Fly Ash | Fire Brick | Ceramic Waste |
|-------------------|---------|------------|---------------|
| SiO2 | 51.999 | 54.113 | 49.113 |
| Al2O3 | 18.273 | 22.394 | 20.303 |
| Fe2O3 | 7.288 | 2.94 | 6.62 |
| CaO | 3.145 | 4.432 | 2.432 |
| MgO | 1.191 | 0.606 | 4.427 |
| Na2O | 0.683 | 0.158 | 1.897 |
| K2O | 1.296 | 0.953 | 1.596 |
| TiO2 | 0.949 | 1.196 | 0.678 |
| MnO | 0.055 | 0.03 | 0.043 |
| P2O5 | 0.199 | 0.071 | 0.074 |
| LOI | 14.87 | 13.45 | 12.2 |
| SUM | 99.948 | 100.343 | 99.383 |

Figure 3, shows the crystalline structures of FB and CW which was analyzed using X-ray diffraction (XRD) technique. The XRD diffractograms identifies the characteristic peaks at $2\theta = 26.976^\circ$ and $2\theta = 35.475^\circ$, corresponded to SiO_2 and Al_2O_3 oxides respectively. These characteristics are important to assess the pozzolanic potential of any supplementary cementitious material [25]. In the current study, the two binders showed low intensity diffraction peaks at $2\theta = 33.23^\circ$ belonging to Fe_2O_3 oxide, which provides a supporting role in strength gaining and pozzolanic mechanism with silica and alumina. Similar to alumina, it can be observed that the two binders presented diffractions peaks at $2\theta = 50.457^\circ$ related to CaO oxide. The presence of CaO oxide enhances the formation of $\text{Ca}(\text{OH})_2$ which extends the formation of hydration products in proposed binders [29]. The studied binders also revealed diffraction peaks at $2\theta = 39.773^\circ$ for sodium and potassium oxides and $2\theta = 41.023 - 42.725^\circ$ for magnesium oxide, and $2\theta = 33.23^\circ$. Finally, the results from XRD patterns were well agreed with that obtained from XRF data.

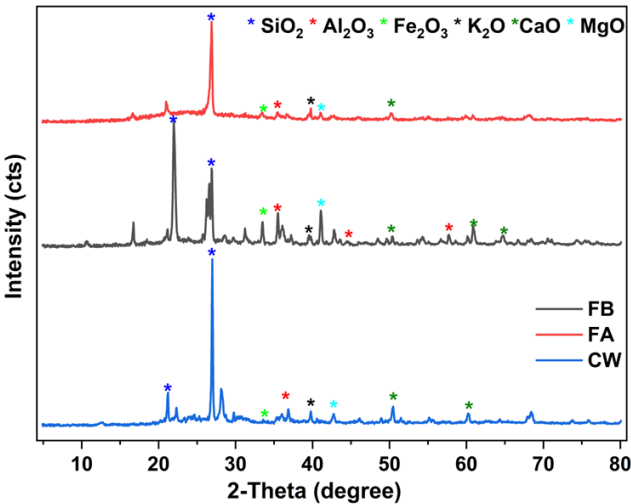


Figure 3. X-ray diffractograms of the FB and CW precursors.

Figure 4 shows the morphology of FB and CW that was determined using Field Emission Scanning Electron Microscope (FESEM) and elemental composition determined by Energy-dispersive X-ray spectroscopy (EDX). The FESEM of fly ash (which is one of the most frequently used source materials in the production of geopolymer concrete) was also included for the sake of comparison (Figure 4a). From this Figure, it can be seen that the particles of the fly ash appeared in spherical shape with various diameter ranging from 500 nm to 10 μ m, due to various elements as shown in XRF results. Referring to Figure 4b, the surface morphology of the CW had uneven particle shape, rough surface texture and sharp edges. While the FESEM observation of FB indicated that the FB particles have a nonhomogeneous and angular-shaped structure morphology as well as a flaky appearance (see Figure 4c). In addition, Figure 5, exhibits the EDX line spectra of the three binders, showed the presence of oxygen (O), silicon (Si) and aluminum (Al) as a main element with highest percentages for FB binder, with a trace of sodium (Na), calcium (Ca), magnesium (Mg) and iron (Fe).

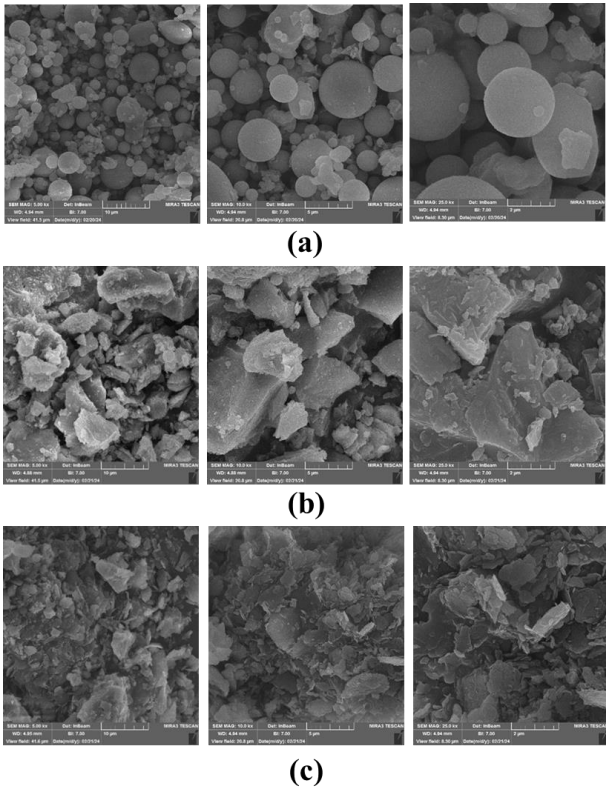


Figure 4. FESEM images of;(a): Fly ash, (b): Fire brick, (c): Ceramic Waste.

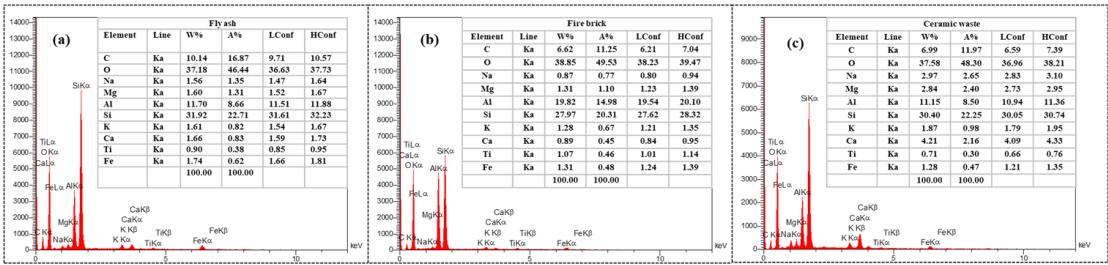


Figure 5. EDX results: (a): Fly ash, (b): Fire brick, (c): Ceramic Waste.

Figure 6 shows the calculated S_{BET} values (Specific surface area calculated by Brunauer-Emmett-Teller BET) method. This value was $2.8688 \text{ m}^2/\text{g}$, $2.2603 \text{ m}^2/\text{g}$ and $5.4581 \text{ m}^2/\text{g}$ for FA, FB and CW, respectively. The value of the total pore volume and mean pore diameter were $0.01250 \text{ cm}^3/\text{g}$ and 5.449 nm for FA; $0.008276 \text{ cm}^3/\text{g}$ and 5.483 nm for FB; and $0.02744 \text{ cm}^3/\text{g}$ and 7.580 nm for CW. In general, higher specific area provides good interaction between the binder and activator, hence improving geopolymerization process and mechanical properties.

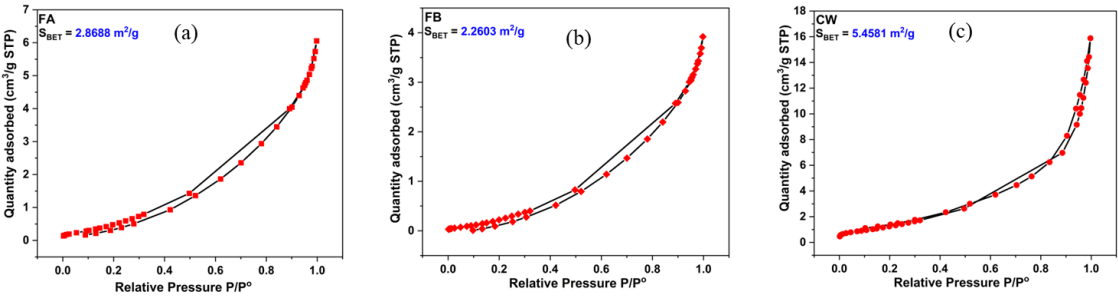


Figure 6. BET analysis of (a): Fly ash, (b): Fire brick, (c): Ceramic Waste.

2.3. Preparation of Alkaline Activator

Sodium Hydroxide (NaOH) and Sodium Silicate (Na_2SiO_3) was used as an alkaline activator. To prepare the alkali activator, firstly, a 10M sodium hydroxide solution was prepared by dissolving 400 g of sodium hydroxide NaOH (98%) flakes in 1L of water tab. Secondly, the solution was allowed to cool down to the room temperature for 24 hr. The 12M and 15M sodium hydroxide solutions were, respectively prepared by dissolving 480 gm and 600 gm following the same procedure for 10M. Finally, the sodium silicate solution (13.1-13.7 % Na_2O , 32-33% SiO_2) was mixed with the previously prepared sodium hydroxide solutions. Previous studies showed that the combination of sodium silicate (SN) and sodium hydroxide (HN) led to enhance the geopolymerization reaction and increase the degree of gelation [29,30]. In addition, studies reported that the activator ratio; Na_2SiO_3 : NaOH of 2.5:1 usually led to the optimum mechanical properties of the produced geopolymer matrix [31,32]. In this study the activator ratios; Na_2SiO_3 : NaOH (1:1, 1.5:1, 2:1, 2.5:1) were considered as a parameter under investigation which is a further step beyond the current state of the art.

2.4. Fine Aggregates

River sand was used as fine aggregate in this study with a nominal maximum grain size of 4.75 mm. The results of the sieve analysis are presented in Figure 7. Gradation of the sand was tested according to the limit specified for the use in masonry mortar as per ASTM C33 / C33M-16 [33].

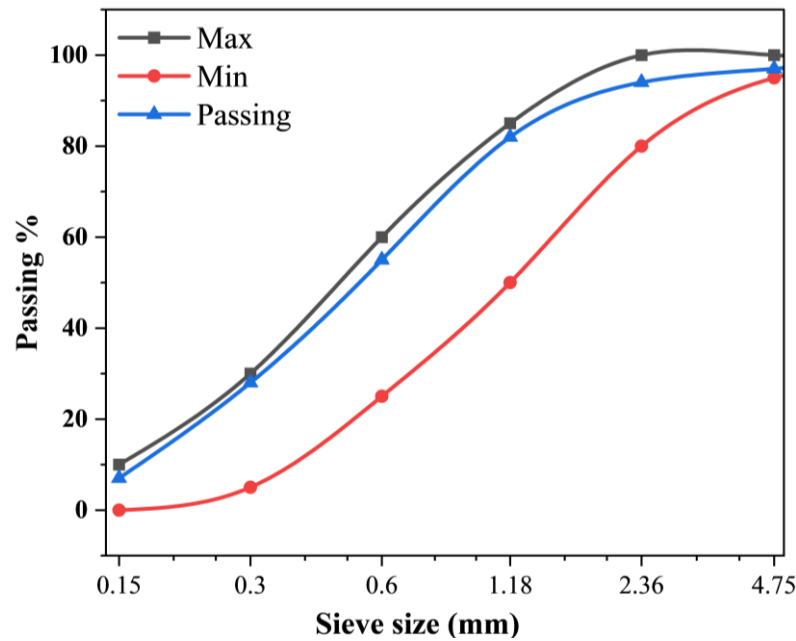


Figure 7. Sieve analysis of fine aggregate.

2.5. Trail Mixes

Several trail mixes were examined to select a mix ingredient that provided acceptable properties in terms of workability and compressive strength. For all mixes, the CDW-based material and sand were poured into a mixer and mixed for 1 minutes. Then, the alkali solution (the Liquid to Binder (L/B) ratio was 0.44) was slowly added to the mix and stirred for 4 minutes [7]. The fresh mixes were then cast into cubic molds measuring (50×50×50) mm (Figure 8). Specimens were divided into three groups. The first group was left to cure at ambient temperature (40±2), the second and third group was treated with heat at temperature of 70 and 100 C° for a period of 24 hrs. and then left to cure at ambient temperature to the day of testing at ages of 7, 14 and 28 days. Finally, compressive tests were conducted to evaluate the strength of the mortar using compression machine with a loading rate of 0.9 kN/s according to ACI Committee 437 [34]. Detailed of trial mixes proportions and resulted compressive strength are provided in Table 3. According to this Table, trial mix No. 3 was adopted based on their workability and compressive strength.

Table 3. Trail mix Details.

| Mix No. | Mix Details | | | Testing Results | |
|---------|-------------|--------|------|------------------|----------------------------|
| | L/B | Binder | Sand | Workability (mm) | Compressive Strength (MPa) |
| 1 | 0.44 | 1 | 1.33 | 90 | 15 |
| 2 | 0.44 | 1 | 1.25 | 95 | 17 |
| 3 | 0.44 | 1 | 1.00 | 100 | 23 |
| 4 | 0.44 | 1 | 2.25 | 60 | 12 |
| 5 | 0.42 | 1 | 1.70 | 75 | 14 |



Figure 8. Stages of preparation, casting, and testing of the mortar mixes.

3. Results and Discussion

This section presents the results of the compressive strength taking into the account the effect of type of CDW-based precursor (FB and CW), molarity (10, 12, and 15) M, curing methods (ambient and heat curing), and concentration/ratio of alkaline activator (1:1, 1:1.5, 1:2,1nd 1:2.5). The compressive strength was evaluated at ages of 7, 14, and 28 days. The total number of tested specimens was 396 spacemen. The specimen’s name followed Xm_Y_Z where X refers to the type of binder (FB: for fire brick and CW: for ceramic waste), m stands for molarity, Y denotes to the sodium silicate ratio, Z represents the curing method (A: for ambient curing, H: for heat curing (70 and 100 C⁰). The suffix R (FA replacement ratio) was added for those specimens that the fly ash replacement was included.

3.1. Compressive Strength

Table 4 presents the results of the compressive strength (average of three specimens) of geopolymer mortars. They were presented according to the activator concentrations, activator Ratio (AR), curing methods, and curing temperature. In the following sections, the effect of different parameters on the compressive strength development of geopolymer mortars is discussed separately.

Table 4. Test results.

| No | Mix | Compressive Strength (MPa) | | | No | Mix | Compressive Strength (MPa) | | |
|----|--------------|----------------------------|----------|----------|----|--------------|----------------------------|----------|----------|
| | | 7 (day) | 14 (day) | 28 (day) | | | 7 (day) | 14 (day) | 28 (day) |
| 1 | FB10_1_A | 2.5 | 12.2 | 19.5 | 23 | CW10_1_A | 1.8 | 6.1 | 13.3 |
| 2 | FB10_1.5_A | 3.0 | 12.5 | 19.8 | 24 | CW10_1.5_A | 2.0 | 7.0 | 14.5 |
| 3 | FB10_12_A | 3.5 | 13.3 | 26.2 | 25 | CW10_12_A | 2.1 | 8.6 | 18.2 |
| 4 | FB10_2.5_A | 2.8 | 13.8 | 28.3 | 26 | CW10_2.5_A | 2.4 | 9.5 | 19.1 |
| 5 | FB12_1_A | 4.2 | 15.4 | 20.6 | 27 | CW12_S_A | 1.9 | 6.6 | 15.7 |
| 6 | FB12_1.5_A | 5.0 | 16.2 | 23.3 | 28 | CW12_1.5_A | 2.2 | 7.3 | 18.1 |
| 7 | FB12_2_A | 2.1 | 16.0 | 39.6 | 29 | CW12_2_A | 2.6 | 9.6 | 25.2 |
| 8 | FB12_2.5_A | 3.8 | 15.2 | 41.4 | 30 | CW12_2.5_A | 3.8 | 11.0 | 24.1 |
| 9 | FB15_1_A | 5.6 | 10.3 | 12.0 | 31 | CW15_S1_A | 1.7 | 5.2 | 14.5 |
| 10 | FB15_1.5_A | 3.0 | 10.7 | 13.2 | 32 | CW15_S1.5_A | 1.9 | 5.4 | 15.5 |
| 11 | FB15_2_A | 3.2 | 21.2 | 38.0 | 33 | CW15_S2_A | 2.0 | 8.6 | 23.0 |
| 12 | FB15-2.5_A | 2.0 | 22.3 | 39.1 | 34 | CW15_S2.5_A | 2.2 | 9.4 | 24.2 |
| 13 | FB12_2_H70 | 2.1 | 4.9 | 13.8 | 35 | CW12_2_H70 | 4.1 | 9.1 | 14.2 |
| 14 | FB12_2_H100 | 4.8 | 5.6 | 14.2 | 36 | CW12_2_H100 | 4.2 | 9.4 | 15.0 |
| 15 | FB12_2.5_H70 | 5.2 | 13.8 | 17.7 | 37 | CW12_2.5_H70 | 8.7 | 14.1 | 17.5 |

| | | | | | | | | | |
|----|---------------|-----|------|------|----|---------------|-----|------|------|
| 16 | FB12_2.5_H100 | 6.5 | 16.6 | 20.9 | 38 | CW12_2.5_H100 | 9.4 | 14.6 | 17.2 |
| 17 | FB15_2_H70 | 4.4 | 7.1 | 13.2 | 39 | CW15_2_H70 | 3.2 | 4.7 | 8.1 |
| 18 | FB15_2_H100 | 4.2 | 8.2 | 13.4 | 40 | CW15_2_H100 | 3.4 | 5.1 | 8.5 |
| 19 | FB15_2.5_H70 | 5.2 | 12.7 | 18.5 | 41 | CW15_2.5_H70 | 5.2 | 10.3 | 15.4 |
| 20 | FB15_2.5_H100 | 8.8 | 13.9 | 15.9 | 42 | CW15_2.5_H100 | 6.2 | 10.8 | 16.1 |
| 21 | FB12_2_A_R15 | 8.7 | 16.6 | 39.4 | 43 | CW12_2_A_R15 | 5.4 | 7.7 | 22.2 |
| 22 | FB12_2_A_R30 | 9.8 | 20.1 | 43.7 | 44 | CW12_2_A_R30 | 6.2 | 10.3 | 27.1 |

3.2. Type of Binder

Figure 9a-c presents the results of the compressive strength at different ages and at ambient curing condition. In general, the highest compressive strength recorded was for geopolymer mortars produced from fire brick for all activator concentrations, activator ratio, curing methods, and heat curing temperature. In specific, the higher compressive strength achieved was 39.6, 41.4, 38.0 and 39.1 MPa for specimens FB12_2_A, FB12_2.5_A, FB15_2_A and FB15_2.5_A respectively (Table 4). Whereas, the maximum corresponding compressive strength of geopolymer mortars produced from ceramic waste was 25.2, 24.1, 23.0 and 24.2 MPa, for specimens CW12_2_A, CW12_2.5_A, CW15_S2_A and CW15_S2.5_A respectively.

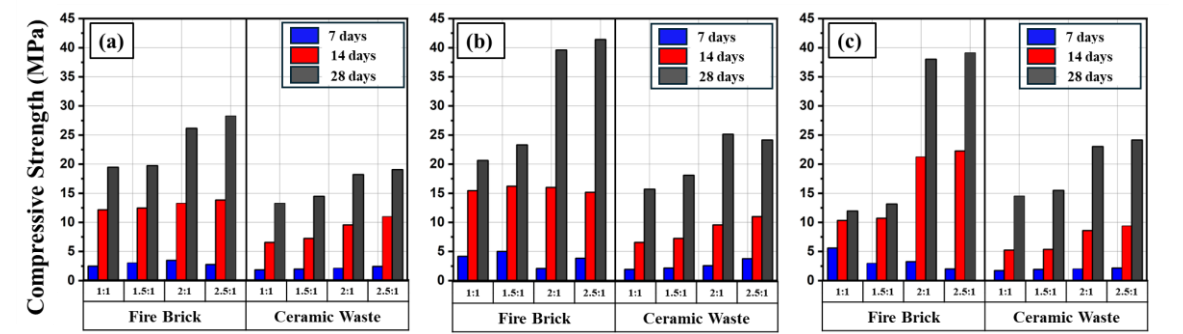


Figure 9. Effect of the type of the binder and activator ratio on the compressive strength at ambient curing condition and different ages for different molarity; (a) 10 Molarity, (b) 12 Molarity and (c) 15 Molarity for ambient curing method.

The performance of fire brick over ceramic waste can be explained by its chemical composition. This material has high SiO₂ (54.113%) and Al₂O₃ (22.394%) content, thus more Si and Al ions are solubilized during alkaline activation and participate in geopolymeric reactions for the formation of Si–O–Al bonds [35]. In contrast, the low compressive strength of CW could be attributed to the following reasons: (i) the existence of sodium and potassium oxides in CW binder materials may cause fast dehydration and thus reduced the efficiency of geopolymerization reaction; (ii) the presence of magnesium oxide in CW binder slightly lowered the mechanical strength of the produced geopolymer mortars [36]; (iii) the high content of CaO could also be the reason behind low compressive strength of CW geopolymer mortar since it consumes NaOH [37].

Figure (9a-c) show also the effect of the age of geopolymer mortar of both binders on the compressive strength. In general, a considerable enhancement in the strength from age 7 to 28 days was observed which agrees with the behavior of normal cement mortar.

3.3. Effect of Alkaline Activator

Three types of molarity (10, 12 and 15) M and four AR (1, 1.5, 2 and 2.5) were adopted. Figure 9a-c presents the results of compressive strength of FB and CW based mortar with that molarity and activator ratio. For both two types of geopolymer, it can be seen that increasing the molarity of alkaline activator from 10 to 12 M resulted in noticeable enhancement in the compressive strength. However, after further increase of molarity to 15 M, a slight reduction in the compressive strength compared to the 12 M was observed. Nevertheless, the 15M specimens showed higher compressive strength than the counterpart specimens of 10 M. It was reported that the low concentrations of NaOH did not provide sufficient alkalinity in the matrix and formation of calcium silicate aluminum

hydrate (C-A-S-H) gel in geopolymer matrix, consequently, the compressive strength weakened. Similarly, the high concentrations of NaOH may result in a residual alkali concentration that remains unreacted and thus reduced the strength [38]. This weakness in the strength could also be attributed to coagulation of silica and faster setting which does not allow for a homogenous mixing resulted in a poor and incipient polymerization [39].

In terms of activator ratio, it was noted that as Na_2SiO_3 increased, the compressive strength was also increased. The reason behind that was the formation of the geopolymer network depends on the Na_2SiO_3 concentration which is responsible for the dissolution of the binder particles of the based geopolymer. However, above 2:1 AR, the increase in the compressive strength was margin (see Figure 9), which might be due to the high viscosity of the Na_2SiO_3 that inhibited the dissociation of the Al and Si into the network of the geopolymer [40].

Overall, it can be recommended that the optimum conditions that provide reasonable compressive strength for both types of geopolymer mortar in terms of molarity and activator ratio was 12M and 2:1, respectively.

3.4. Effect of Curing Temperature

Figures 10 and 11 show the effect of heat curing on the compressive strength in comparison with counterpart specimens cured at ambient conditions. The heat curing at high temperatures was compared with those specimens with molarity of 12M and 15M and AR of 2:1 and 2.5:1 as these molarity and AR showed the highest compressive strength. It can be seen that irrespective of the alkaline solution concentration and the type of binder, the highest compressive strength was achieved for those specimens that cured at ambient temperature. After further increase in the temperature to 70 °C, a considerable decrease in the strength was noted. However, at temperature of 100 °C, a slight increase in the strength in some of the specimens was observed compared to 70 °C. These findings agreed with results reported by Reig, et al. [41]. This would raise an important question on the curing temperature above 100 °C and curing time.

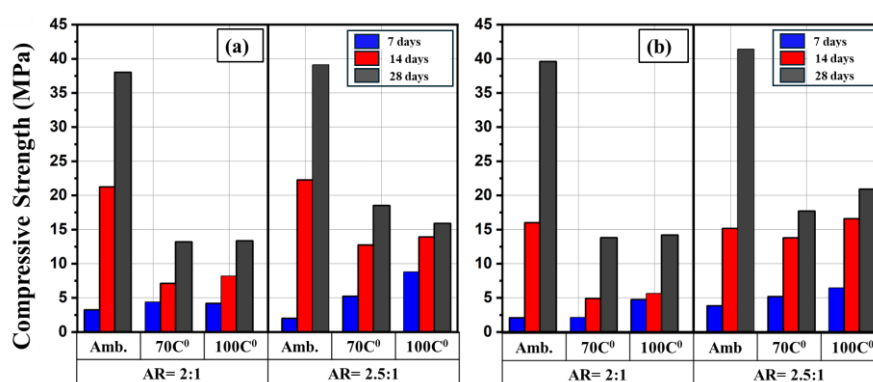


Figure 10. Comparison of compressive strength of FB-based geopolymer mortars at different curing conditions and ages; (a) 12 M and (b) 15 M.

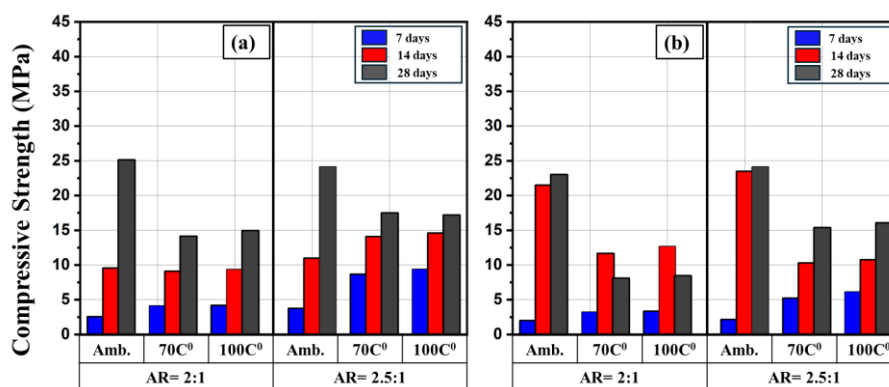


Figure 11. Comparison of compressive strength of CW-based geopolymer mortars at different curing conditions and ages; (a) 12 M and (b) 15 M.

As mentioned, the highest compressive strength achieved was for those specimens that cured at ambient temperature. It was reported that when geopolymer mortars cured at low temperatures, the reaction products would have enough time to slowly fill the pores in the geopolymeric structure. This led to increase the density of the mortar [42], consequently, the compressive strength enhanced. It was also found that the compressive strength decreases with increasing the curing temperature. This probably due to presence of cracks at microscale that caused by excessive shrinkage and dehydration. In addition, curing at elevated temperatures could cause rapid geopolymerization reactions, giving a less ordered and more porous structures, which may reduce the compressive strength [43]. Another reason regarding low compressive strength noted at high curing temperatures can be related to the quality of reaction products that formed after geopolymerization. This may due to the fact that part of solution may evaporate at this temperature and thus did not contribute in geopolymerization reactions [44].

Overall, among all above parameters, it seems that the chemical composition of the source materials was the most effective factor influencing the compressive strength. This explains why the highest compressive strength was achieved for the FB based geopolymers despite the high crystallinity implying that although physical properties are highly important, chemical composition may be the main criterion that modify the mechanical response of geopolymers.

3.5. Fly Ash Replacement

From Table 4, it can be seen that the early strength (at 7 days) of all produced mortars was quite low. According to previous studies [45] using fly ash enhance the early strength of geopolymer; therefore, two percentages (15 and 30%) of the binding material was replaced by fly ash for those specimens of 12M and AR= 2. Figure 12 demonstrates the effect of FA replacement on the compressive strength of geopolymer mortars. For FB-based mortar (Figure 12a), a significant increase in the early strength (at 7 days) was observed, However, this was not the case for the CW-based mortar; as can be seen from Figure 12b, the enhancement of the early strength was limited.

In terms of late strength (i.e., at 28 days), no clear conclusion can be drawn, however, it seems that using fly ash as a replacing did not affect the compressive strength. It could be attributed to the intrinsic properties of based binders in giving high strength at the late age.

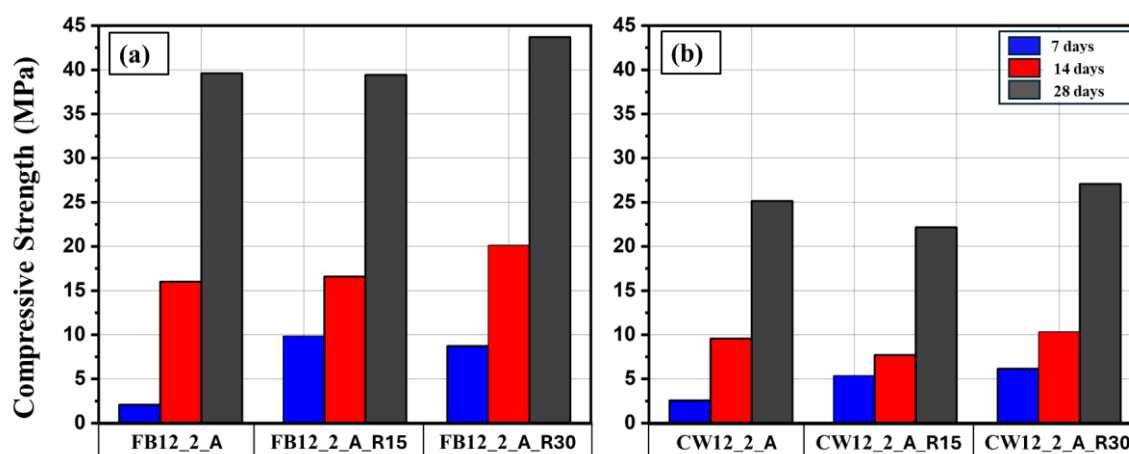


Figure 12. Effect of 15 and 30% replacing of FB and CW-based binder by fly ash on the compressive strength at different ages; (a) FB, (b) CW

4. Conclusions

This study investigates for the first time the potential of producing geopolymer mortar using Fire Brick (FB) and Ceramic Waste (CW) -based materials as a binder. The main tested parameters were; type of binder (FB and CW), the activator molarity (M) (10, 12, and 15M), the activator ratio; Na_2SiO_3 : NaOH (1:1, 1.5:1, 2:1, 2.5:1), curing conditions (ambient and heating to 70 and 100°C) and replacement ratio of the binder by 15 and 30% of fly ash. The compressive strength was evaluated at age of 7, 14 and 28 days. According to the obtained results, the following conclusions can be drawn:

1. Both binders (FB and CW) can be a successful potential for production of geopolymer concrete.

2. Regardless the molarity, activator ratio and curing conditions, the FB- based mortar showed higher compressive strength than the counterpart CW-based mortar for all testing ages. This is due to the fact that the chemical composition (i.e. the content of SiO₂, Al₂O₃, CaO, Fe₂O₃, and MgO) of the selected binder play important roles in the mechanical properties of the produced geopolymer concrete.
3. Different molarity of alkali activator (10, 12 and 15M) resulted in different compressive strength. In general, for both binders, increasing the molarity from 10 to 12 M resulted in a significant increase in the strength. However, beyond 12 M, a noticeable reduction in the compressive strength was observed compared to the 12M but still higher than the counterpart 10M specimens for all other parameters under investigations.
4. For both binder-based geopolymer mortar, the highest compressive strength achieved was for those specimens of molarity 12 M and Na₂SiO₃: NaOH 2:1 and 2.5:1.
5. The results of the compressive strength of the two types of binders showed that the curing at ambient temperature was the more effective than the heat curing at 70 and 100 C°.
6. Replacing 15 and 30% of FB and CW binders by fly ash significantly enhance the early strength (at 7 days). However, the corresponding increase at 28 days was almost negligible which confirm that the based binder was responsible for such strength.
7. Overall, the general conclusion, it appears that the chemical composition of the selected binder was the main factor that play important role in enhancing the compressive strength of the resulted geopolymer.

Abbreviations

The following abbreviations are used in this manuscript:

| | |
|-------|---|
| OPC | Ordinary Portland Cement |
| CDWs | Construction and Demolishing Wastes |
| FB | Fire brick |
| CW | Ceramic Waste |
| FA | Fly Ash |
| GGBFS | Ground Granulated Blast Furnace Slag |
| AR | activator ratios |
| CB | Ceramic Ball |
| SR | Silica Rock |
| GS | Glassy Sand |
| CKD | Cement Kiln Dust |
| GA | Grinded Aggregate |
| XRF | X-ray fluorescence |
| XRD | X-ray diffraction |
| L.O.I | Loss on Ignition |
| EDX | Energy-dispersive X-ray spectroscopy |
| FESEM | Field Emission Scanning Electron Microscope |
| BET | Brunauer-Emmett-Teller |
| M | Molarity |
| GPC | Geopolymer concrete |

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