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Rabab Al Hassani , [Tarig Ali](#) , [Md Maruf Mortula](#) \* , [Rahul Gawai](#)

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## Article

# An Integrated Approach to Leak Detection in Water Distribution Networks (WDNs) Using GIS and Remote Sensing

Rabab Al Hassani, Tarig Ali, Md Maruf Mortula \* and Rahul Gawai

Department of Civil Engineering, American University of Sharjah, Sharjah P.O. Box 26666, United Arab Emirates; g00079369@alumni.aus.edu (R.A.); atarig@aus.edu (T.A.); rgawai@aus.edu (R.G.)

\* Correspondence: mmortula@aus.edu (M.M.M.)

**Abstract:** Leakages in the water distribution networks (WDNs) are real problems for utilities and other governmental agencies. Timely leak detection and location identification has been a challenge. In this paper, an integrated approach to geospatial and infrared image processing method was used for robust leak detection. The method combines drops in flow, pressure, and chlorine residuals to determine potential water leakage locations in the WDN using Geographic Information System (GIS) techniques. GIS layers were created from the hourly values of these three parameters for the city of Sharjah provided by Sharjah Electricity, Water and Gas Authority (SEWA). These layers are then analyzed for locations with dropped values of each of the parameters and are overlaid with each other. In the case where there were no overlaying locations between flow and pressure, further water quality analysis was avoided, assuming no potential leak. In the case where there are locations with drops in flow and pressure layers, these overlaying locations are then examined for drops in chlorine values. If overlaying locations are found, then these regions are considered potential leak locations. Once potential leak locations are identified, a specialized remote sensing technique can be used for precise leak location. This study also demonstrated the suitability of using an infrared camera for leak detection in a laboratory-based setup. This paper concludes that the following methodology can help water utility companies in the timely detection of leaks, saving money, time, and effort.

**Keywords:** water distribution network (WDN); leak detection; GIS; remote sensing; infrared (IR)

## 1. Introduction

Water distribution networks (WDNs) are complex systems that are prone to significant water loss, and this loss is mainly due to pipe leakage [1,2]. It is estimated that leaks can contribute up to 70% of water losses in transmission systems [3]. Leakages in these WDNs are mainly caused by pipe damage or by the network's inability to control pressure due to uncertain demand and operating conditions [4]. Leaks in pipelines are issues of increasing concern in WDNs as they have negative environmental, economic, and social impacts. Pipe leakages have detrimental effects on natural water resources, nearby infrastructure, and the environment, as it causes pipes to burst and it enables the entry of harmful contaminants into the network [4,5]. The loss of a substantial volume of water that has undergone costly treatment is for one is a serious economic issue [6]. Moreover, leaky pipes cause an increase in pumping energy and system rehabilitation costs, which compromises the water quality by enabling the entry of contaminated groundwater, pathogens, and soil constituents [7]. Leaks also have the potential to erode soil and recharge aquifers beneath urban areas, which puts building foundations at risk [8]. Not only will mitigating leakages reduce operating costs and increase revenues, but it will also improve water efficiency, minimize infrastructure damage, and prevent adverse effects on human health [9].

Detecting these leakages pose a great challenge to engineers since the pipes are usually buried underground. For this reason, constant monitoring is required to identify and prevent potential water

leakages in pipes. Traditional leak detection methods, which are disruptive techniques, would change the structure of the WDN, disrupting the serviceability of the network [10]. However, research on novel non-destructive methods have shown the potential for leak detection without altering the chemical composition or geometry of the materials being investigated.

The advent of technology has led to developments in non-destructive leak detection techniques for WDNs. For instance, innovative non-destructive techniques (NDT) like infrared (IR) cameras, spectrometers, and Ground Penetration Radar (GPR) have been used to identify leaks in different types of pipes at different moisture contents [10,11]. The study deduced that all the three NDTs could identify leaks for PE, PPR, and PVC pipes but the detection effectiveness decreased as moisture content of soil increased. A recent study has shown the use of GPR and IR cameras simultaneously to effectively determine water leaks in both cold and hot weather conditions [12]. Since GPR is a geophysical imaging technique used for subsurface monitoring, it can accurately identify the pipe location underground [12]. The IR camera is then able to determine leakage locations and estimate the leakage area [12]. Thermal imaging has also been successful in leak detection [13]. In addition, acoustic emission methods are another NDT suitable for detecting leaks as it collects sound signals generated by the cavitation and the turbulence that occurs in a leak [14,15]. It is important to note that signal processing and classification methods are required to verify the noises formed are due to leaks [14]. So, integrated approaches are proven to be the most effective for accurate and efficient real-time monitoring. Therefore, using a combination of NDTs has proven to enhance the effectiveness of leak detection methods to help mitigate leakages.

The reviews of existing leak detection methods indicated the problem of accuracy in relation to identifying the location of leakages. So, combining different technologies to improve the accuracy of leak detection has proven to be most effective [16]. This paper aims to use geographic information systems (GIS) and remote sensing with an infrared camera to accurately detect leakage in a pipe network. GIS refers to a computer-based system that stores, analyses, and displays geographically referenced data [17]. Therefore, GIS is essential for the operation of water networks; studies have shown the integration of GIS can assist in real-time leak detection [18, 19]. A recent study [20] utilized the ArcGIS software to assist in the analysis of water losses in a WDN by using four feature classes: pipeline layer, meter layer, elevation map, and field operations layout. ArcGIS displayed the results obtained from the field and the results calibrated, which illustrated the faulty meters and pipes leak locations [20]. Combining the results of the model displayed on GIS with other layers like a topographic layer of the region or the district metered area zones enhanced the analysis of critical zones of WDNs for optimal operation and management of the network [20].

Remote sensing provides better temporal and spatial coverage than ground detection methods [21]. Recent studies have shown that spatial resolution is an essential parameter for the detection and mapping of water leakage regions using remote sensing data. A study [16] identified leakages by recording remote sensing data from ground spectroradiometers and hyperspectral data from a low altitude system. The study deduced that water leakages can be monitored and detected using the appropriate spatial resolution images. The spectral signals of dry and wet soils were recognizable in the visible range 400-700 nm and in the near IR range of 750-900 nm [16]. In comparison to dry soils, wet soils have 20-25% lower reflectance values, and the difference is maximized in the near IR range [16]. The research study concluded that remote sensing is effective for the determination of water pipe location and leakage [16].

The literature on using hyperspectral imaging for water leakage detection is limited, however, it is a growing area in remote sensing [22]. Hyperspectral imaging is a developing technology in remote sensing where an imaging spectrometer collects hundreds of images at different wavelengths for the same spatial area [22]. It is concerned with the measurement, analysis, and interpretation of spectra taken from a given scene or object at a short, medium, or long distance from by an airborne (drone) or satellite sensor [23]. The NASA Jet Propulsion Laboratory's Airborne Visible Infra-Red Imaging Spectrometer (AVIRIS) can record the visible and near infrared spectrum (wavelength range 400-2500 nm) of the reflected light on an area 2-12 km wide and several kilometres long using 224 spectral bands [24]. The result is a stack of images whereby each pixel has a corresponding spectral

signature or 'fingerprint' that distinguishes the underlying objects, and the final data volume comprises several gigabytes per flight [22]. This usually requires hardware accelerators to speed up computations. Hyperspectral sensors are expected to increase their spatial, spectral, and temporal resolutions [22]. The incredible amount of spectral information available from the latest hyperspectral devices has opened doors to real-time processing applications such as monitoring of leakages [25].

An IR camera detects infrared energy reflected or emitted by an object and converts it into a thermal image [10]. Leaks in an underground water network may change the temperature of the surrounding soil as leaked water is typically cooler than soil, which absorbs thermal energy faster than water [10]. In addition, IR cameras can be used during any time of the day, and it can investigate large areas in comparatively less time with lower costs than other NDTs [26]. The paper presented the use of thermography IR camera for the detection of heat changes at pavement surfaces due to water pipe leaks underneath the surface. The results of the study showed that the IR camera successfully detected several leaks as thermal contrast at pavement surfaces occurred in fall and spring seasons. However, it failed in detected leaks during the summer and winter due to high pavement temperature and snow coverage accordingly. A more recent study evaluating the effectiveness of spectrometer, GPR, and IR as NDTs deduced that the IR camera was shown to be the most effective for pipeline leak detection [10].

A study [27] used medium and high-resolution data from different satellites for the detection of water leakages in the "Frenaros – Choirokoitia" water pipe in Cyprus. The study applied two alternative methodologies, the first used a high resolution QuickBird image to identify and verify 'suspicious leaks' in a small area near the water pipes [27]. The second methodology involved using multi-temporal analysis using medium-resolution SPOT images. The analysis focused on regions around the joints of the pipe, using a 10m buffer zone [27]. This method recorded 10 possible leakage points along the 25km long pipeline [27]. The effectiveness of this study could be enhanced if the images were taken of a larger area, displaying an entire WDN and not just a single pipeline. In addition, acquiring images at a high spatial resolution can increase the accuracy of leak detection along the pipeline.

In this paper, an integrated approach of GIS and infrared image processing was used to detect leakages in the WDN of Sharjah Electricity and Water Authority (SEWA). The objective of this paper is to develop a leak detection method using GIS and remote sensing. The aim of using this method is to enhance the efficiency of WDNs by increasing the accuracy of identifying leakages. This study creates a GIS-based customized system to identify potential leakage locations and to deploy an IR camera to identify potential leak locations.

## 2. Materials and Methods

To achieve the objectives, the paper develops an integrated leak detection method using GIS and remote sensing. For this reason, the methodology is deconstructed into two phases. In the first phase, the use of GIS helps identify potential leak locations using different sets of hydraulic and water quality data of a real WDN that may indicate potential leaks. That is, sudden drops in pressure, flow, and water quality can be shown on spatial variability maps generated by GIS. At the end of the first phase, there are either no leaks found in certain locations or there are candidate leak locations identified. Once the candidate leak locations are found, the second phase begins with the use of remote sensing to capture and process images that can be utilized in the candidate leak location.

### 2.1. Phase 1: GIS Application

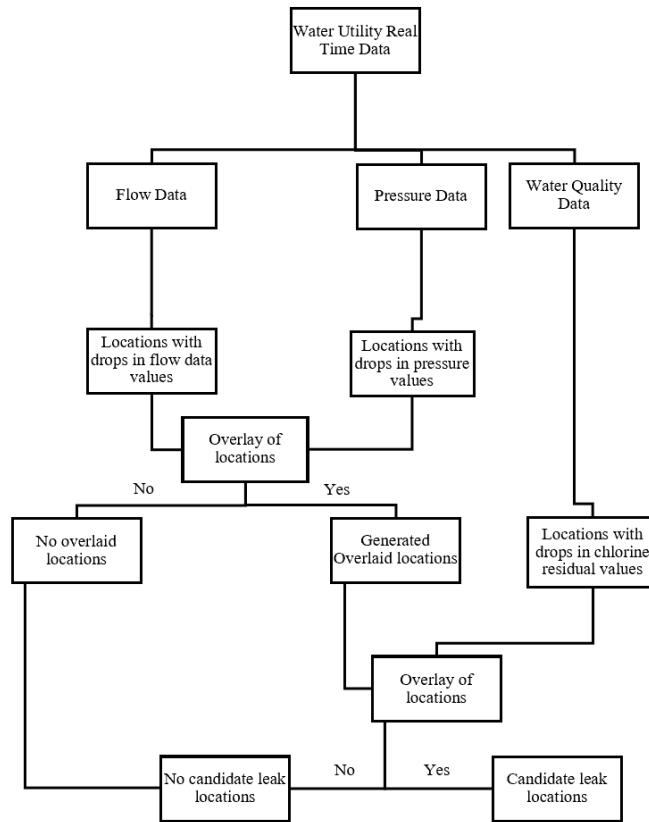
In this study, WDN for the City of Sharjah, UAE was used to demonstrate the applicability of the model. Sharjah Electricity and Water Authority (SEWA) manages the desalination plants and WDN in the City of Sharjah. The WDN consists of more than 4000 km of pipe networks. Figure 1 illustrates a map of SEWA's WDN that was used for the GIS-based model for the leak detection system. Most of the pipes were made up of asbestos cement pipes. The SEWA installed many sensors throughout the large WDN to monitor the hydraulic and water quality parameters to ensure enough water is available with good quality.



**Figure 1.** Water distribution network of Sharjah Electricity and Water Authority.

SEWA installed sensors in the WDN capable of monitoring real-time hydraulic and general water quality parameters. Potential leaks can cause drops in parameters like pressure, flow, and water quality. Also, the larger the number of joints along a pipe length, the more likely it is for there to be a leak. In large WDNs like Sharjah, it is difficult to differentiate leaks to other sources of parameter fluctuations. So, spatial variability maps were generated in GIS for pressure, flow, and water quality parameters such as pH, conductivity, and chlorine residuals. The spatial variability maps were used to identify parts of the WDN where large drops of pressure, flow, and chlorine residuals occur. It is assumed that the locations with high drop in pressure, flow and chlorine residuals may indicate potential leakages. Three conditioning factors were represented as raster layers on the ArcGIS Pro software: pressure, flow, and chlorine residual.

The ArcGIS Pro software was used in this phase. The flow chart illustrated in Figure 2 shows the values of the three parameters, flow, pressure, and water quality of a set of locations during any period. This data was provided by SEWA. The real-time data is continuous, and the software is expected to run analysis as data is provided. Initially, the flow and pressure parameters in different regions undergo overlay analysis. An overlay analysis carries through all the attributes of the features taking part in the overlay, (drops in pressure and flow) and creates a new polygon dataset. The data set provided ten locations (regions) in Sharjah and tabulated thirty readings for pressure and flow data for each location, and the average value was calculated for each of those data sets. Any individual value lower than the average of the data set is under suspicion of containing a leak. Each value indicating a drop in pressure or flow along the pipe was overlaid in the software, and this either created new overlaying regions or no regions at all. In the case where there are no overlaying regions among the two parameters, there is no need to analyse the water quality values and the conclusion is that there are no leaks detected in the given set of locations at a particular time. In the second case where there are one or more overlaying regions formed among pressure and flow drops, these overlaying regions are merged with locations that indicate drops in chlorine residual values. Afterwards, there is no new overlaying region, which indicates that there are no leaks in that location at a certain time or there are new overlaying regions formed. If one or new overlaying regions are generated, then the conclusion is that those locations do have leaks.



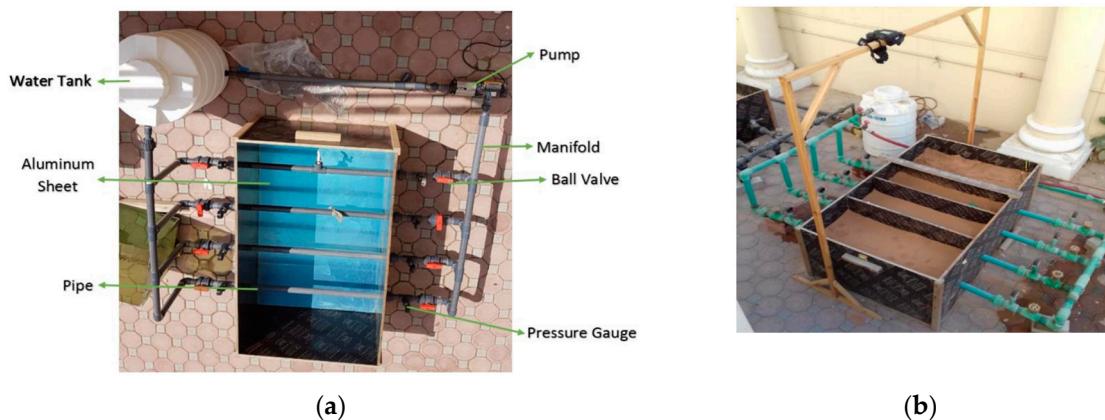
**Figure 2.** Flow Chart of Phase 1 using ArcGIS Pro.

## 2.2. Phase 2: Remote Sensing with Infrared Camera

In the second phase of this integrated approach, thermal images of the model distribution system were captured using the FLIR420 IR camera (Figure 3). The camera has a wavelength range of 7500-14000nm. Since the data provided by SEWA was a few years old at the time of the study, conducting field tests may not be meaningful as maintenance activities might have already been done on the suspected locations. In addition, due to government restrictions related to field experiments on the locations (identified in Phase 1), similar experiments were conducted on the model setup. The thermal images of the model water distribution system, shown in Figure 4a,b, were captured manually. This WDN model is a carefully built setup, which includes a dune sand filled box with four PPR pipes, is designed to simulate pipe leaks in underground conditions. One of the pipes is a regular pipe without any leak and the three other pipes were created with simulated leaks of crack, hole and joint. Experiment was conducted on the pipe with hole. These grey scaled IR images generated during laboratory-based experiments was then processed to display the heat signatures. Since water has a cooling effect compared to the surrounding soil, with the help of time series images, the leakage locations can be identified.



**Figure 3.** FLIR T420 IR Camera.



**Figure 4.** (a) Labelled Diagram of Experimental Setup; (b) Model Water Distribution System.

### 3. Results and Discussion

#### 3.1. GIS Application

##### 3.1.1. Case Scenario 1: No Detection of Leakage Location(s)

To prove that this integrated approach works, this method was applied to values of dates that are known not to have any leakages. The raw data provided by SEWA is shown below and it illustrates the flow and pressure values for each day in one month at 10 locations in the city of Sharjah. Using ArcGIS Pro, the flow and pressure data for each location is evaluated to map the water quality parameters. As mentioned in the previous section, the assumption is made that any flow higher than the average or pressure value lower than the average from the data set (highlighted in green and red) is suspected to have a leak in that location. Another assumption for suspected leakage is customer complaints.

Tables 1 and 2 show the daily flow data (in  $\text{m}^3/\text{day}$ ) at 10 different locations in a specific month and Table 3 shows the average flow for that month. Any value less than the average is highlighted in green.

**Table 1.** Flow data provided by SEWA (first half of the month).

DMA_New	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	D12	D13	D14	D15	D16
AL RAHMANYA - 1	740	760	660	680	710	770	710	670	600	790	580	740	500	600	0	650
AL RAHMANYA - 3	960	1100	1030	970	1200	1140	1210	1120	600	1160	990	1190	1140	670	1250	1210
INDUSTRIAL AREA - 4	4500	5070	5440	6930	5430	5450	5012	5051	104760	4730	4960	5180	5380	5150	5150	5350
INDUSTRIAL AREA - 4	30	20	10	0	40	30	40	40	20	10	0	10	20	40	10	30
BARASHI	410	570	160	630	670	930	1180	2260	2070	1800	1220	1420	640	780	1060	830
MAYSALOON	440	650	800	450	170	100	240	1310	1710	2540	2780	3120	2620	2670	2940	3110
AL FAYAH	980	980	1020	970	1030	1050	1070	1080	1010	1060	1060	1080	1070	1060	960	1020
AL GHUWAIR	3340	4230	8190	3800	4650	5290	2870	3850	830	790	870	860	1200	890	950	1070
BU TINA	6500	6490	6600	7280	7270	7120	7010	7040	6930	7410	7800	7420	7410	7150	7200	7150
AL SABKHA	3740	3730	3690	3670	3770	3760	3680	3730	3700	3870	3810	3720	3840	3970	3780	3830
AL SABKHA	890	880	900	830	800	860	740	880	850	820	800	730	640	690	700	630
AL GHAPHIA	1270	1140	750	220	550	1170	610	830	1060	740	350	690	830	730	850	760
AL GHAPHIA	1160	1070	950	780	860	1070	930	1060	1050	800	680	740	650	640	750	750

**Table 2.** Flow data provided by SEWA (second half of the month).

DMA_New	D17	D18	D19	D20	D21	D22	D23	D24	D25	D26	D27	D28	D29	D30	D31
AL RAHMANYA - 1	660	780	600	730	720	660	660	670	600	680	640	710	670	560	710
AL RAHMANYA - 3	1180	900	1000	1270	1200	1210	1150	1390	1010	1180	1140	1230	1160	1200	1360
INDUSTRIAL AREA - 4	5310	5180	5060	5340	5090	5220	5330	5250	4830	4950	5380	5160	5350	5380	5000
INDUSTRIAL AREA - 4	30	40	20	0	10	20	20	30	80	110	50	80	60	60	80
BARASHI	660	1260	920	970	1690	1370	1160	820	1220	1580	2070	1580	1710	1480	1270
MAYSALOON	2850	2850	2190	1870	2090	2380	2390	2020	2650	2000	1740	0	1530	1400	1380
AL FAYAH	1050	1050	1130	1010	1050	980	1010	990	1030	1170	1040	1060	1100	1020	1090
AL GHUWAIR	1060	1050	950	950	940	920	980	950	1150	1450	260	210	210	220	200
BU TINA	7040	7690	7380	7070	6950	6860	7050	7070	7320	7380	7200	7220	7470	7220	7150
AL SABKHA	3790	3860	3790	3790	3760	3700	3770	3700	3730	3750	3680	3690	3780	3750	3770
AL SABKHA	620	670	690	710	720	770	750	720	700	640	620	630	630	680	740
AL GHAPHIA	640	160	60	360	730	750	900	780	460	510	550	440	230	180	550
AL GHAPHIA	790	720	720	810	800	800	920	760	910	800	770	830	750	740	820

**Table 3.** Average flow in a month (m<sup>3</sup>/day).

DMA_New	AVG (m <sup>3</sup> /day)
AL RAHMANYA - 1	651.9354839
AL RAHMANYA - 3	1113.548387
INDUSTRIAL AREA - 4	5210.967742
INDUSTRIAL AREA - 4	33.5483871
BARASHI	1173.870968
MAYSALOON	1773.870968
AL FAYAH	1041.290323
AL GHUWAIR	1780
BU TINA	7156.452

AL SABKHA	3761.290323
AL SABKHA	739.6774194
AL GAPHIA	640.3225806
AL GAPHIA	834.8387097

Tables 4 and 5 show the daily pressure data (in bars) at 10 different locations in one month and Table 6 shows the average flow for that month. Any value less than the average is highlighted in red.

**Table 4.** Pressure data provided by SEWA (first half of the month).

DMA_New	D1	D2	D3	D4	D5	D6	D7	D8	D9	D10	D11	D12	D13	D14	D15	D16
AL RAHMANYA - 1	4.07	3.87	2.56	4.2	3.96	3.76	4	3.43	2.33	3.12	3.13	3.23	2.84	1.99	3.65	3.81
AL RAHMANYA - 3	3.7	3.41	2.18	3.7	3.58	3.23	3.39	2.73	2.22	2.31	2.41	2.33	3.03	2.61	2.8	2.98
INDUSTRIAL AREA - 4	1.07	1.01	0.99	0.97	1.05	1.04	1.03	1.05	1.07	1.1	1.13	1.07	1.09	1.07	1.05	1.02
BARASHI	3.31	1.73	1.93	2.64	2.2	1.33	1.89	2.09	2.48	2.04	1.38	1.44	1.09	0.89	1.05	1.17
MAYSALOON	1.07	1.07	1.03	0.95	1.03	1.04	1.02	1.07	1.14	1.21	1.12	1.08	1.15	1.09	1.07	1.08
AL FAYAH	0.89	0.86	0.88	0.89	0.88	0.9	0.87	0.87	0.85	0.93	0.94	0.89	0.93	0.9	0.9	0.87
AL GHUWAIR	0.87	0.87	0.84	0.76	0.83	0.86	0.85	0.88	0.93	0.94	0.85	0.84	0.9	0.86	0.84	0.85
BU TINA	0.69	0.68	0.68	0.63	0.67	0.69	0.68	0.7	0.7	0.72	0.68	0.67	0.71	0.69	0.68	0.67
AL SABKHA	1.05	1.02	1.01	0.97	1.01	1.05	1.01	1.04	1.02	1.04	1	1.02	1.03	0.96	1.01	1.01
AL GHAPHIA	1.15	1.1	1.06	1.01	1.07	1.14	1.07	1.12	1.11	1.1	1.04	1.07	1.08	1	1.07	1.08
AL NASSERYA	0.9	0.8	0.82	0.8	1.07	1.08	0.79	0.84	0.84	0.89	0.88	0.83	0.87	0.83	0.81	0.81
AL QADSIA	0.9	0.87	0.88	0.85	0.89	0.9	0.87	0.9	0.87	0.93	0.91	0.85	0.93	0.89	0.87	0.86
INDUSTRIAL AREA - 6	0.67	0.66	0.5	0.3	0.55	0.64	0.67	0.7	0.79	0.56	0.32	0.5	0.49	0.47	0.48	0.53

**Table 5.** Pressure data provided by SEWA (second half of the month).

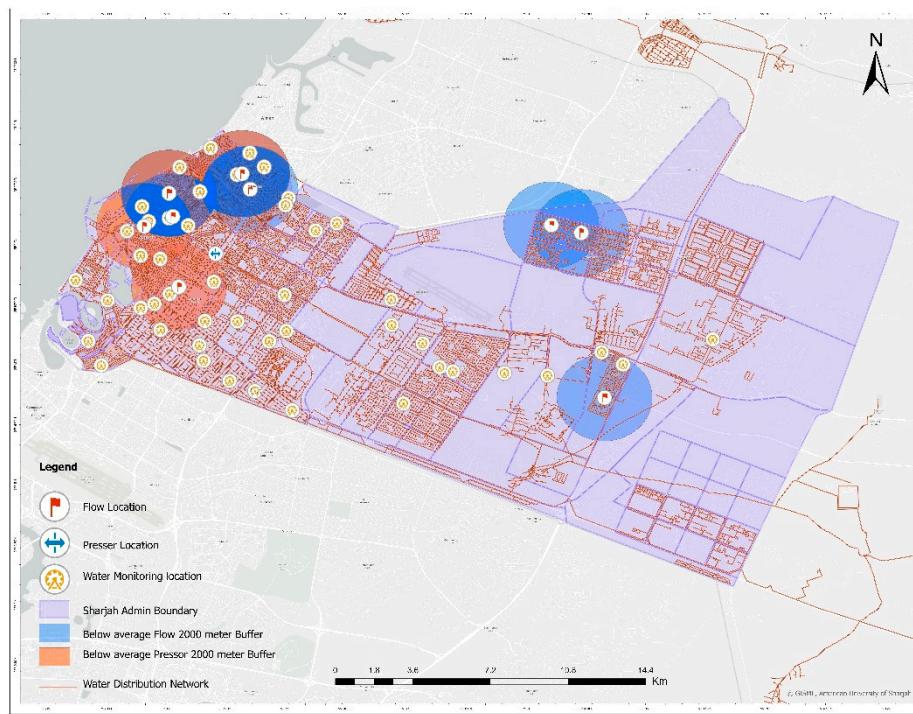
DMA_New	D17	D18	D19	D20	D21	D22	D23	D24	D25	D26	D27	D28	D29	D30	D31
AL RAHMANYA - 1	3.85	3.72	2.61	3.97	3.97	3.93	3.78	3.63	3.73	3.59	3.69	3.82	3.91	2.94	4.1
AL RAHMANYA - 3	3.1	3.12	2.18	3.15	3.22	3.33	3.17	2.94	3.05	2.98	2.73	3.34	2.87	3.21	3.36
INDUSTRIAL AREA - 4	0.98	0.99	0.92	0.96	0.98	0.91	0.86	0.88	0.84	0.85	0.89	0.89	0.98	0.99	1
BARASHI	0.93	1.12	1.25	1.32	2	1.6	1.68	1.26	1.29	1.57	1.76	1.95	1.77	1.64	1.42
MAYSALOON	1.05	1.01	1.03	1.04	1.04	1.01	1.02	1.01	0.97	0.98	0.97	0.95	0.98	0.96	0.95
AL FAYAH	0.83	0.84	0.84	0.87	0.85	0.82	0.83	0.86	0.81	0.83	0.8	0.82	0.86	0.88	0.87
AL GHUWAIR	0.82	0.76	0.79	0.83	0.84	0.8	0.8	0.79	0.72	0.75	0.75	0.77	0.8	0.79	0.77
BU TINA	0.64	0.6	0.61	0.67	0.67	0.63	0.63	0.64	0.58	0.59	0.61	0.62	0.65	0.66	0.65
AL SABKHA	0.99	0.95	0.97	1	1	0.96	1	1.01	0.96	0.98	0.99	0.99	1.01	1.01	1.01
AL GHAPHIA	1.06	1.02	1.04	1.08	1.07	1.02	1.09	1.09	1.06	1.07	1.05	1.07	1.09	1.08	1.08
AL NASSERYA	0.77	0.75	0.76	0.8	0.79	0.75	0.76	0.76	0.71	0.74	0.75	0.74	0.78	0.77	0.77
AL QADSIA	0.82	0.82	0.83	0.87	0.86	0.81	0.82	0.84	0.78	0.81	0.81	0.81	0.85	0.85	0.85
INDUSTRIAL AREA - 6	3.85	3.72	2.61	3.97	3.97	3.93	3.78	3.63	3.73	3.59	3.69	3.82	3.91	2.94	4.1

**Table 6.** Average pressure in a month (bars).

DMA_New	AVG (bars)
AL RAHMANYA - 1	3.522258065
AL RAHMANYA - 3	2.979354839
INDUSTRIAL AREA - 4	0.991290323
BARASHI	1.652258065
MAYSALOON	1.038387097
AL FAYAH	0.866451613

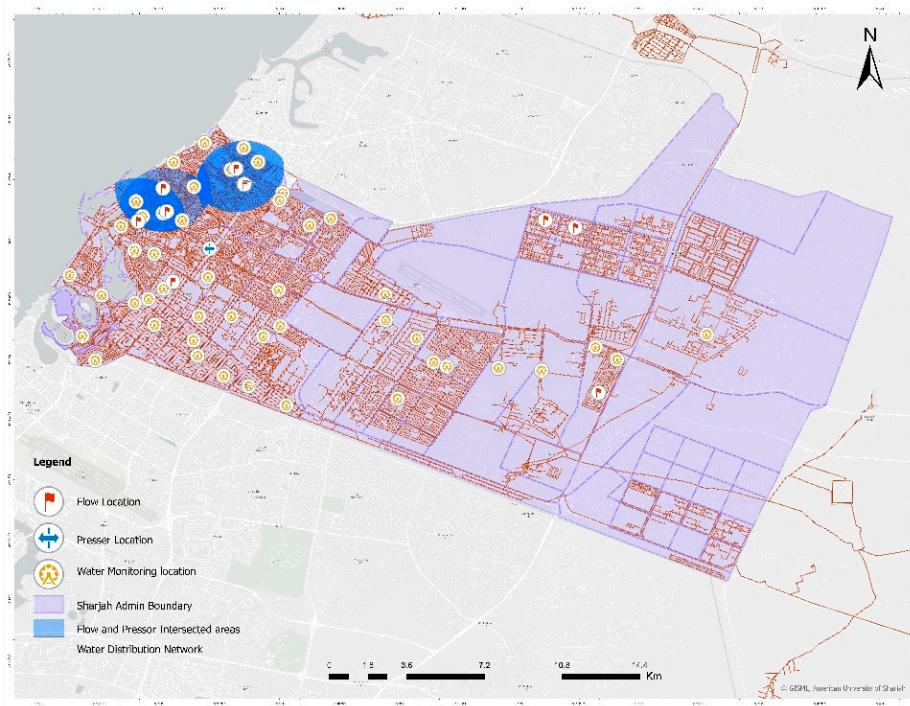
AL GHUWAIR	0.824193548
BU TINA	0.657741935
AL SABKHA	1.0025806
AL GHAPHIA	1.072258065
AL NASSERYA	0.81483871
AL QADSDIA	0.858064516
INDUSTRIAL AREA - 6	0.58806452

The model is initiated by adding the given pressure and flow values into the ArcGIS Pro software. This is followed by the addition of the flow query values, which excludes any flow values above the average value and identifies the value with the lowest flow below average from each location. Then the pressure query values are found by taking the average of the average pressure values from each location. Once the query values have been obtained in the model, a buffer analysis is conducted within a radius of 2000m around the suspected leakage areas. That is, the buffer analysis tool on ArcGIS Pro traverses the suspected regions and the creates buffer polygon offsets. The blue polygons in Figure 5 show the intersections of the buffer offsets generated from the suspected areas (with drops in pressure and flow) within a 2000m radius.



**Figure 5.** Buffering 2000m from location of data.

Once buffering is complete, the software generates a new output feature class. In terms of the water quality parameter, there are 42 chlorine residual locations within the polygon generated. The low chlorine values were determined by finding any value lower than the average of the daily mean chlorine values. The locations of low chlorine values were determined using the 'near' function. Figure 6 focuses on the intersections of the model that generates a new output feature class. The output results on the locations without any leakage event show that there are no regions with leakages. Therefore, this proves that this method works for any pressure or flow value, indicating that it can be applied in any situation.



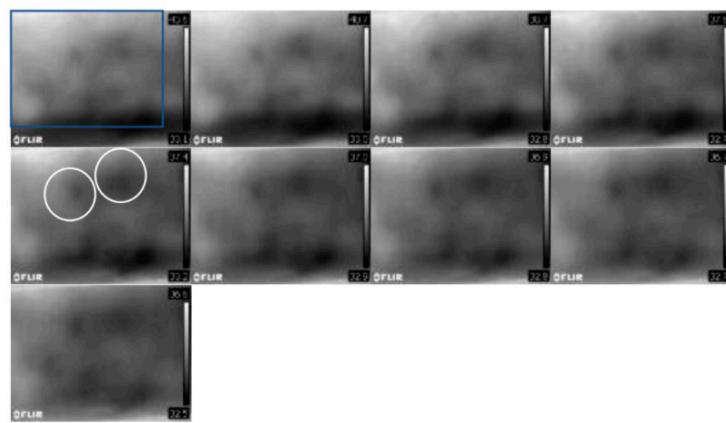
**Figure 6.** Intersection Map.

### 3.1.2. Case Scenario 2: Detection of Leakage Location(s)

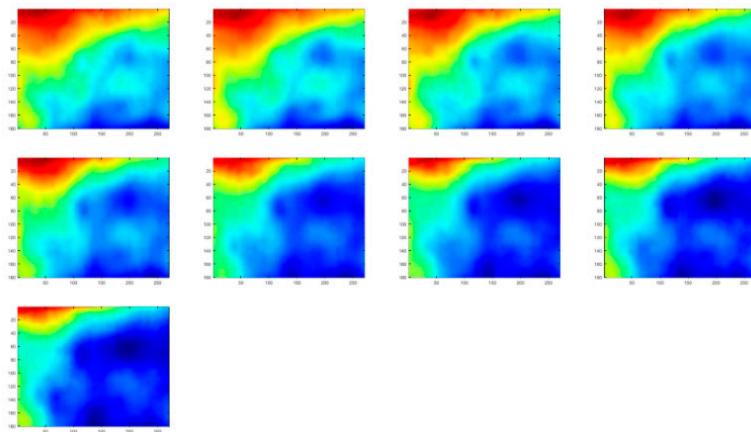
The same procedure applied in Case Scenario 1 is undertaken. The output identified the three locations suspected of leakages area Al Ghaphia, Al Ghuwair, and Maysaloon. Once these locations were identified, a customized interface was run on ArcGIS Pro. The identified leak locations are determined based on the intersections of buffer zones from the low pressure and flow data, as well as areas with low chlorine levels, detected using the 'near' function on the software. The GIS approach used in this study is different to other studies for leak detection. For this reason, direct comparison may not be meaningful. However, other studies using GIS for leak detection found it feasible as a tool for leak detection (28, 29).

### 3.2. Remote Sensing & IR Camera

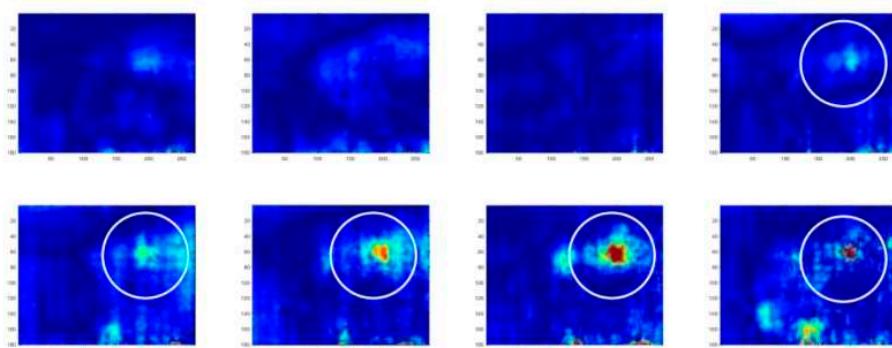
The experiment was designed such that one IR video could be captured over 20 minutes over a pipeline. The experiment was run for three hours, so nine images were acquired on the IR camera. The images shown in Figures 7–9 are the IR images of the model WDN shown in Figure 4(b). Figure 7 shows the grey-scaled image which is illustrated for perception purposes only. The images are then processed, as shown in Figure 8.



**Figure 7.** Grey-scaled IR images.



**Figure 8.** Processed IR Images.



**Figure 9.** Analysis of IR images.

Figure 9 has the best illustration of leaks captured on the IR camera. The circles shown on the images indicate the water leakage locations. The analysis of the leaks was done using the following log ratio shown in Equation (1):

$$\log R = \log \left( \frac{T(IR(t+1))}{T.IR.t} \right) \quad (1)$$

The log-ratio resulted in the best images for visual detection, as the equation focuses on only one source of leakage. Therefore, the log-ratio method is best at detecting the temporal variation of the temperature due to the leakage. Hot spot analysis of the IR images has been employed to detect the leaks. As observed in Figure 9, there is a clear formation of light blue spots over time in the IR images. The light blue spots indicate the locations where the temperature was reduced, due to increased moisture in the soil from the leak. IR images with higher resolutions may be able to capture more distinct temperature contrasts between the soil and water leak. It is important to note that the general moisture of the soil can drastically affect the readings of the IR camera. That is, it can be more difficult to detect leaks if the soil has an initially high percentage moisture (4). Previous studies conducting water leak and oil spill detection also observed it to be a suitable technology (4, 30). The use of GIS can provide the general location of the leak with the infrared technology to be used for accurate leak location. The integrated hybrid approach can be very suitable large water distribution networks.

#### 4. Conclusions

The study used a novel integrated approach combining the use of GIS and remote sensing for effective water leak detection. The study developed a novel GIS based approach for identifying general leakage location. The infrared based remote sensing technology on the other hand was used for identificaiton of precise leak location. Laboratory based experiments were conducted for remote sensing experiments.

Overall, this integrated approach presented in this study has demonstrated promising results in the detection of leakage locations within WDNs. While previous literature has shown the use of GIS and remote sensing independently for water pipeline leak detection [17, 23], the studies have covered smaller regions and did not use an integrated approach. As such, this method goes beyond the identification of leakages and confirms data indicating the absence of leaks in specific regions. By combining GIS and remoting sensing technologies, along with IR image analysis, this approach provides an effective means of monitoring and detecting pipe leaks. The successful application of this integrated approach suggests that further research and experimentation should be conducted. Further applications of hyperspectral remote sensing can offer the potential for more detailed and accurate detection and mapping of leakages.

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